



Single and Multiobjective Optimal Control of the Wastewater Treatment Process

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Abstract

This paper involves the use of state-of-the-art optimization techniques to conduct the wastewater treatment process in the most beneficial manner possible. Rigorous single- and multi-objective optimal control tasks are performed on the reduced BSM1 (benchmark simulation model 1) for the wastewater treatment process problem. The multiobjective optimization does not involve weight functions or additional constraints. The state-of-the-art global optimization solver BARON is used. The concentrations of the Nitrite/Nitrate, ammonium and biodegradable organic contaminants are minimized. While the single and multiobjective optimal control profiles for the Nitrite/Nitrate and the biodegradable substance concentrations were qualitatively similar, there was considerable difference between the two profiles when the ammonium concentration profile was minimized. However, when the time for the multiobjective optimal control was increased, resulting profiles for the ammonium concentrations became similar. This demonstrated that increasing the time enabled the multiobjective optimal control to be as effective as the single objective optimal control while controlling all the variables.

Keywords Wastewater · Treatment · Multiobjective · Optimization

Introduction

Purifying wastewater from the contaminants is very essential to safeguard public health and wastewater treatment plants are needed to purify the water from contaminants.

The contaminants commonly occurring in the wastewater that are hazardous to human health include nitrate compounds, nitrite compounds, ammonium compounds, and biodegradable substances. The main aim of wastewater treatment plants is to reduce the concentration of these substances before discharge into the surroundings. While there can be a process to minimize biodegradable residue within the wastewater treatment itself, it is important to develop strategies to minimize this as much as possible. Wastewater treatment plants have to deal with continuous and unpredictable changes in the composition of the incoming untreated impure water. This paper demonstrates the use of a rigorous multiobjective optimal control strategy to ensure that the wastewater treatment plant is operated in a manner such that

the amounts of all the problem-causing contaminants in the effluent water are minimized. This multiobjective optimal control strategy does not involve the use of arbitrary weight functions or additional constraints and the state-of-the-art global optimization solver BARON is used.

Background

The nonlinearity of the wastewater treatment process has led to a lot of modeling work, (Martin 2000; Meijer 2001; Singh et al. 2002; Hulsbeek, et al. 2002) and more recently by (Daughton 2018; Chen et al. 2019; Barceló 2020; Bijlsma et al. 2021; Adhikari and Halden 2022) The need to obtain optimal parameters in order to ensure the efficient operation of the process was investigated by Olsson et al. (2005) and Santin et al. (2016). The irregular variation of the influent along with the complex biochemical processes cause the optimization and control of the process to be challenging. Since the influent composition and the amount of contaminants changes irregularly, it is necessary to use a dynamic optimization to be able to ensure an effective continuous operation of the process. Amand et al. (2013) investigated in detail the effect of controlling the aeration in the wastewater

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treatment process. The use of advanced control strategies on the wastewater treatment plant process was also studied by (O'Brien et al. 2011; Amand et al. 2013; and Santin et al. 2016). The wastewater treatment is an aerobic process and since aeration demands a considerable amount of energy (McCarty et al. 2011; Daverey et al. 2019) the process must be operated in an effective manner. Real-time optimization and model predictive control of waste water treatment plants were studied by Vega et al. (2014); Santin et al. (2016); Piotrowski et al. 2008; Brdys et al. 2008; Tatjewski 2008 and Darby et al. 2011).

Two of the common process configurations for biological wastewater treatment process are the continuous-flow activated sludge process and the sequencing batch reactor process (Henze et al., 2008). Sun et al. compared the nitrogen and ammonia removal from waste water for both these processes and reported almost complete ammonia removal in both processes but observed higher total nitrogen removal in the continuous-flow activated sludge process than in the sequencing batch reactor process. A complete ammonia removal in the sequencing batch reactor process was confirmed in a number of experimental studies (Yalmaz and Öztürk 2001; Andreottola et al. 2001). A comparison of the two processes for the treatment of industrial wastewater was performed by Papadimitriou et al. (2009), who found a higher removal of contaminants by the sequencing batch reactor process. Dionisi et al. 2016, and Dionisi 2017 developed a strategy to calculate the periodic steady state of sequencing batch reactors for biological wastewater treatment.

The reduced BSM1 (benchmark simulation model 1) is commonly used for the waste water treatment process (Julien et al. 1999; Gómez-Quintero and Spérandio 2004; Steffens et al. 1997; Zhao et al. 1994; Silvana et al. 2017). Details of this model can be found in all these articles. This model considers the anoxic and aerobic zones in the wastewater treatment plant. The process details of these zones are given in Silvana et al. 2017.

The values of the parameters, constants and the names of the variables for the equations in this model are shown in Tables 1, 2 and 3. The equations that constitute the model are

$$\rho_{11} = \mu_H \left(\frac{S_{s1}}{K_s + S_{s1}} \right) \left(\frac{S_{O1}}{K_{OH} + S_{O1}} \right) X_{BH} \quad (1)$$

$$\rho_{12} = \mu_H \left(\frac{S_{s2}}{K_s + S_{s2}} \right) \left(\frac{S_{O2}}{K_{OH} + S_{O2}} \right) X_{BH} \quad (2)$$

$$\rho_{21} = \mu_H \left(\frac{S_{s1}}{K_s + S_{s1}} \right) \left(\frac{K_{OH}}{K_{OH} + S_{O1}} \right) \left(\frac{S_{NO1}}{K_{OH} + S_{NO1}} \right) \eta_g X_{BH} \quad (3)$$

$$\rho_{22} = \mu_H \left(\frac{S_{s2}}{K_s + S_{s2}} \right) \left(\frac{K_{OH}}{K_{OH} + S_{O2}} \right) \left(\frac{S_{NO2}}{K_{OH} + S_{NO2}} \right) \eta_g X_{BH} \quad (4)$$

$$\rho_{31} = \mu_A \left(\frac{S_{O1}}{K_{OA} + S_{O1}} \right) \left(\frac{S_{NH1}}{K_{NH} + S_{NH1}} \right) X_{BA} \quad (5)$$

$$\rho_{32} = \mu_A \left(\frac{S_{O2}}{K_{OA} + S_{O2}} \right) \left(\frac{S_{NH2}}{K_{NH} + S_{NH2}} \right) X_{BA} \quad (6)$$

Table 2 Process values that are treated as constant

Anoxic reactor volume	V_1	2000 m ³
Aerobic reactor volume	V_2	3999 m ³
Active heterotrophic biomass concentration	$X_{B,H}$	2500 g/m ³
Active autotrophic biomass concentration	$X_{B,A}$	150 g/m ³
Influent rate	Q_{in}	1272 m ³ /h
Biodegradable substrate concentration in influent	$S_{S,in}$	92 g/m ³
Ammonium compound concentration in influent	$S_{NH,in}$	31.5 g/m ³
Oxygen transfer coefficient	K_{La}	5 (1/h)

Table 1 Parameter values

Oxygen saturation concentration	$S_{O,sat}$	8 (g/m ³)
Heterotrophic max specific growth rate	μ_H	4 (day ⁻¹)
Half saturation coefficient for heterotrophs	K_s	10 (g COD/m ³)
Oxygen saturation coefficient for heterotrophs	K_{OH}	0.2 (g COD/m ³)
Oxygen saturation coefficient for heterotrophs	K_{NH}	1 (g N/m ³)
Oxygen saturation coefficient for autotrophs	K_{OA}	0.4 (g COD/m ³)
Heterotrophic yield	Y_H	0.67 (g COD oxidized)
Autotrophic yield	Y_A	0.24 (g N oxidized)
Nitrogen fraction in biomass	i_{xB}	0.08 (g N/gCOD)
Dimensionless coefficient	η_g	0.8 (no units)

Table 3 Time dependent variables

S_{NH1}	Ammonium concentration exiting anoxic zone g/m ³
S_{NH2}	Ammonium concentration exiting aerobic zone g/m ³
S_{NO1}	Nitrate + Nitrite concentration exiting anoxic zone g/m ³
S_{NO2}	Nitrate + Nitrite concentration exiting aerobic zone g/m ³
S_{S1}	Biodegradable substrate concentration exiting anoxic zone g/m ³
S_{S2}	Biodegradable substrate concentration exiting aerobic zone g/m ³
S_{O1}	Oxygen concentration exiting anoxic zone g/m ³
S_{O2}	Oxygen concentration exiting aerobic zone g/m ³
Q_a	Internal recycle flow m ³ /h

$$\frac{dS_{NH1}}{dt} = \frac{1}{V_1} [Q_{in}S_{NHin} + Q_aS_{NH2} - (Q_{in} + Q_a)S_{NH1}] - i_{Xb}\rho_{11} - i_{Xb}\rho_{21} - \left(i_{Xb} + \frac{1}{Y_A}\right)\rho_{31} \tag{7}$$

$$\frac{dS_{O1}}{dt} = \frac{1}{V_1} [Q_aS_{O2} - (Q_{in} + Q_a)S_{O1}] - \left(\frac{1 - Y_H}{Y_H}\right)\rho_{11} + \left(\frac{4.57}{Y_A} + 1\right)\rho_{31} \tag{13}$$

$$\frac{dS_{NH2}}{dt} = \frac{1}{V_2} [(Q_{in} + Q_a)S_{NH1} - (Q_{in} + Q_a)S_{NH2}] - i_{Xb}\rho_{12} - \left(i_{Xb} + \frac{1}{Y_A}\right)\rho_{32} \tag{8}$$

$$\frac{dS_{O2}}{dt} = \frac{1}{V_2} [(Q_{in} + Q_a)S_{O1} - (Q_{in} + Q_a)S_{O2}] - \left(\frac{1 - Y_H}{Y_H}\right)\rho_{12} + \left(\frac{4.57 - Y_A}{Y_A}\right)\rho_{32} + KLa(S_{O,sat} - S_{O2}) \tag{14}$$

$$\frac{dS_{NO1}}{dt} = \frac{1}{V_1} [Q_aS_{NO2} - (Q_{in} + Q_a)S_{NO1}] - \left(\frac{1 - Y_H}{2.86Y_H}\right)\rho_{21} + \frac{1}{Y_A}\rho_{31} \tag{9}$$

Optimal Control

In the multiobjective nonlinear optimal control (MOOC) strategy (Flores Tlacuahuaz et al. 2012; Sridhar 2019) used in this work the single objective optimal control problem is first solved for each of the objective functions. For a multiobjective optimal control problem

$$\frac{dS_{NO2}}{dt} = \frac{1}{V_2} [(Q_{in} + Q_a)S_{NO1} - (Q_{in} + Q_a)S_{NO2}] - \left(\frac{1 - Y_H}{2.86Y_H}\right)\rho_{22} + \frac{1}{Y_A}\rho_{32} \tag{10}$$

$$\min \Phi(x, u) = (\phi_1, \phi_2, \phi_3, \phi_4, \phi_5 \dots \phi_n)$$

subject to $\frac{dx}{dt} = K(x, u)$

$$h(x, u) \leq 0 \tag{11}$$

$$\frac{dS_{ss1}}{dt} = \frac{1}{V_1} [Q_{in}S_{ssin} + Q_aS_{ss2} - (Q_{in} + Q_a)S_{ss1}] - \left(\frac{1}{Y_H}\right)\rho_{11} - \left(\frac{1}{Y_H}\right)\rho_{21} \tag{11}$$

$$x^L \leq x \leq x^U$$

$$u^L \leq u \leq u^U,$$

$$\frac{dS_{ss2}}{dt} = \frac{1}{V_2} [(Q_{in} + Q_a)S_{ss1} - (Q_{in} + Q_a)S_{ss2}] - \left(\frac{1}{Y_H}\right)\rho_{12} - \left(\frac{1}{Y_H}\right)\rho_{22} \tag{12}$$

the single objective optimization problems are solved independently minimizing each ϕ_i ($i = 1, 2, 3 \dots n$) individually. This will lead to minimized values.

ϕ_i^* ($i = 1, 2, 3, \dots n$). Then the problem that will be solved is

$$\min \sqrt{\left\{ \sum_{i=1}^n (\phi_i - \phi_i^*)^2 \right\}}$$

subject to $\frac{dx}{dt} = K(x, u)$ (16)

$$h(x, u) \leq 0$$

$$x^L \leq x \leq x^U$$

$$u^L \leq u \leq u^U$$

The optimization program, Pyomo (Hart et al. 2017), where the differential equations are automatically converted to a Nonlinear Program (NLP) using the orthogonal collocation method (Biegler 2007) is used for performing the dynamic optimization calculations. The Lagrange-Radau quadrature with three collocation points and ten finite elements are chosen. The resulting nonlinear optimization problem was solved using the solver BARON 19.3 (Tawarmalani and Sahinidis 2005), accessed through the Pyomo-GAMS27.2 (Bussieck and Meeraus 2004) interface.

Table 4 Values of Objective functions

Minimizing SS_2	1.4958E-5
Minimizing NH_2	13.1765
Minimizing NO_2	23.3579
Multiobjective Optimal control	0.5927

BARON implements a Branch-and-reduce strategy and provides a guaranteed global optimal solution. This procedure does not involve the use of weighting functions nor does it impose additional parameters or additional constraints on the problem unlike the weighted function or the epsilon correction method. (Miettinen 1999).

Results and Discussion

Three single objective and one multiobjective optimal control problems were solved. First, the biodegradable substrate was minimized (objective value = 1.4958E-5), subject to

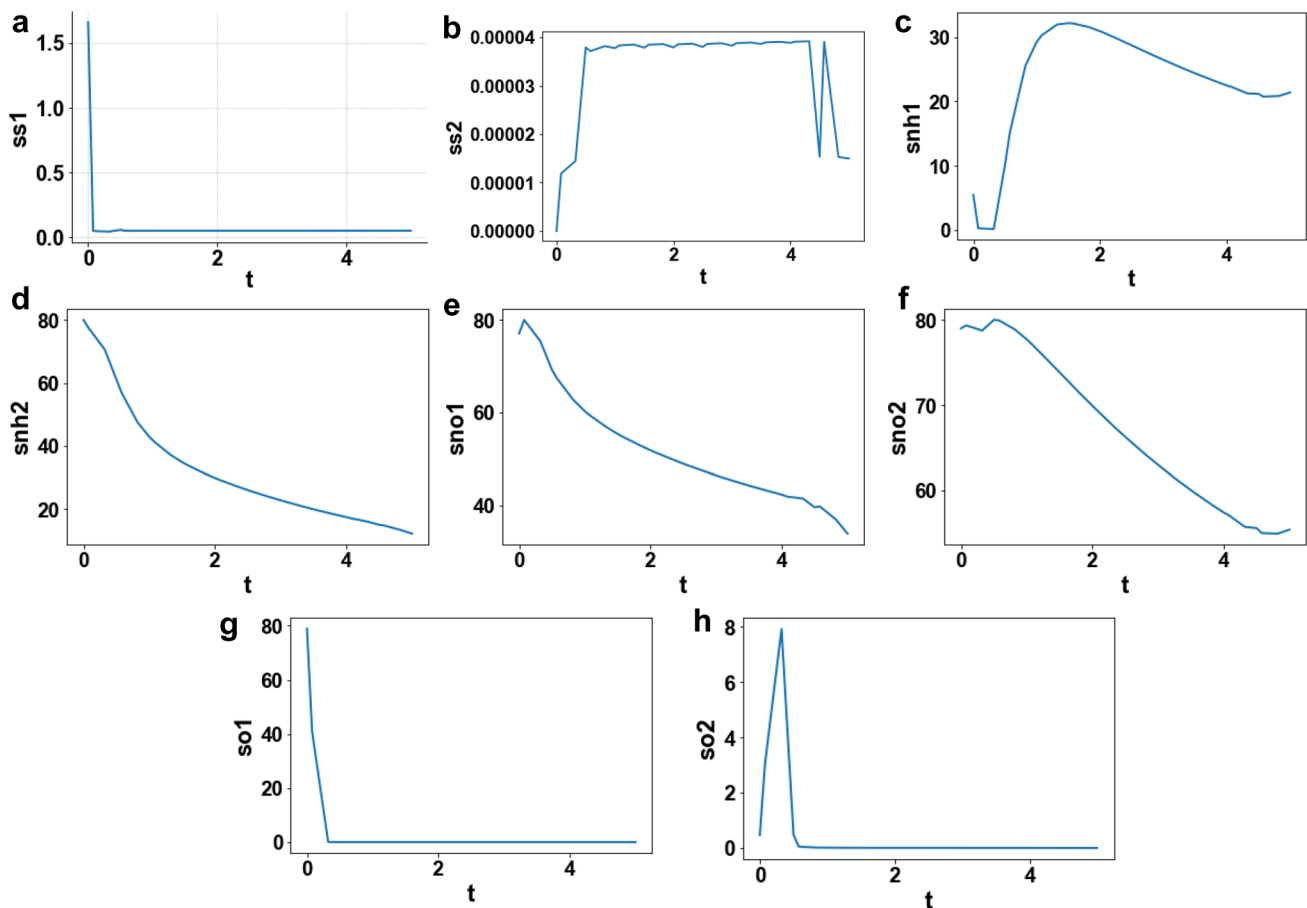


Fig. 1 **a** Minimizing Ss_2 (Ss_1 vs t), **b** Minimizing Ss_2 (Ss_2 vs t), **c** Minimizing Ss_2 (Snh_1 vs t), **d** Minimizing Ss_2 (Snh_2 vs t), **e** Minimizing Ss_2 (Sno_1 vs t), **f** Minimizing Ss_2 (Sno_2 vs t), **g** Minimizing Ss_2 (So_1 vs t), **h** Minimizing Ss_2 (So_2 vs t)

Eqs. 1 to 14 The second optimal control problem involves the minimization of ammonium concentration in the effluent (objective value = 13.1765), while the third optimal control problem deals with the minimization of the nitrite and nitrate compounds (objective value = 23.3579). In the multiobjective optimal control problem the function minimized is $\sqrt{(\sum S_{S_2} - 1.4958E - 5)^2 + (\sum S_{NH_2} - 13.1765)^2 + (\sum S_{NO_2} - 23.3579)^2}$. The multiobjective optimal control objective function value obtained is 0.5927. Table 4 shows the values of the objective functions.

Figure 1a–f shows the concentration profiles when the concentration of the biodegradable substance was minimized Fig. 2a–f shows the profiles when the concentration of the ammonium compounds was minimized while Fig. 3a–f is obtained when the nitrite/nitrate compositions are minimized. Figure 4a–f shows the profiles when the

multiobjective optimal control was performed. Figure 5a shows the Pareto curve that gives the variation of S_{S_2} with S_{O_2} .

Two following important issues can be observed from these results:

1. The multiobjective optimal control yields qualitatively similar results in the cases of S_{S_2} (Figs. 1b and 4b) and S_{NO_2} (Figs. 3f and 4f) while in the case of S_{NH_2} (Figs. 2d and 4d–a) the multiobjective optimal control profile seems to be qualitatively different from the single optimal control where only S_{NH_2} was minimized. However, this could be remedied by increasing the total operation time from 5 to 8 h when the multiobjective optimal control was performed (Figs. 2d and 4d–b). This demonstrates that increasing the time will cause the multiobjective optimal control to be as effective as the

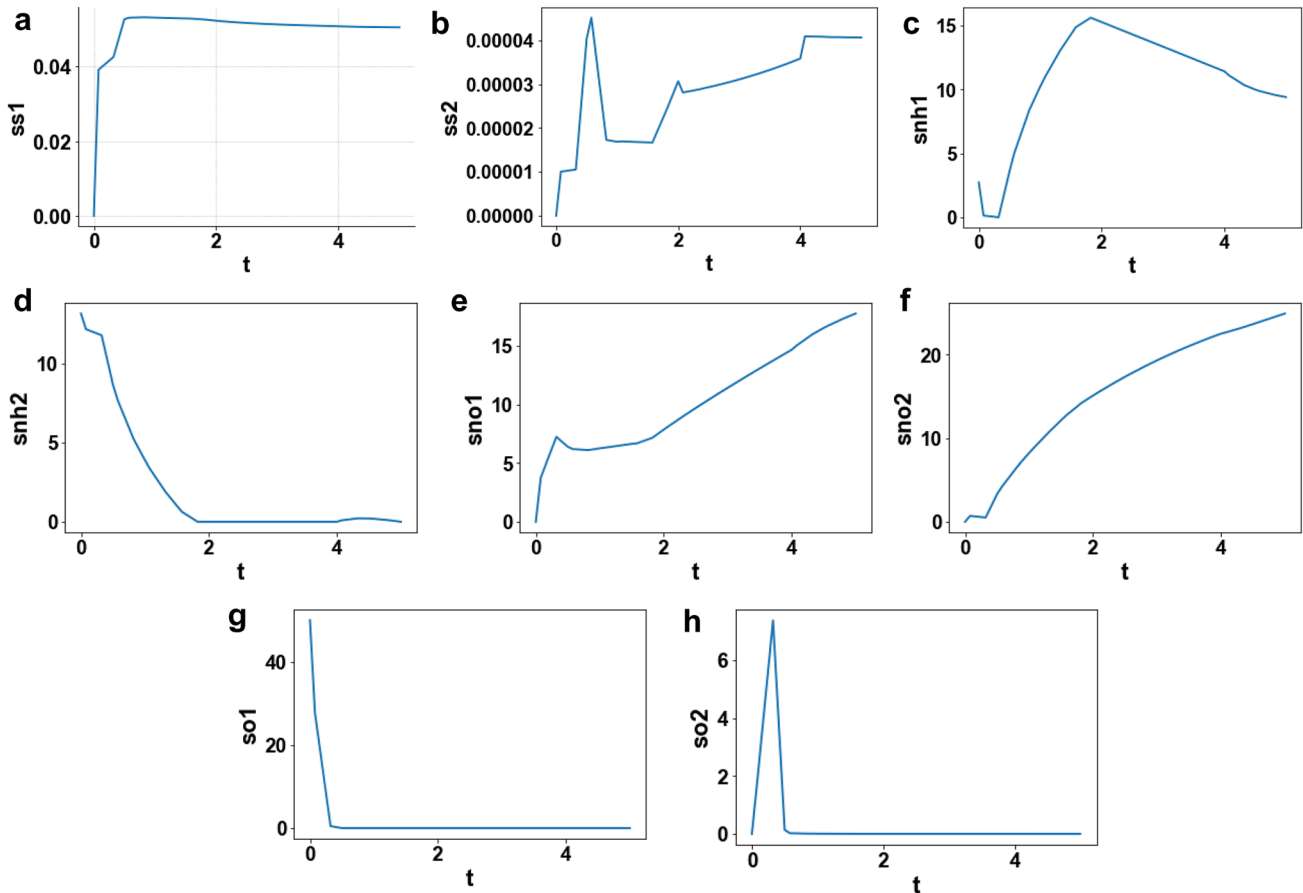


Fig. 2 a Minimizing Snh2(Ss1 vs t), b Minimizing Snh2(Ss2 vs t), c Minimizing Snh2(Snh1 vs t), d Minimizing Snh2(Snh2 vs t), e Minimizing Snh2(Sno1 vs t), f Minimizing Snh2(Sno2vs t), g Minimizing Snh2(So1 vs t), h Minimizing Snh2(So2 vs t)

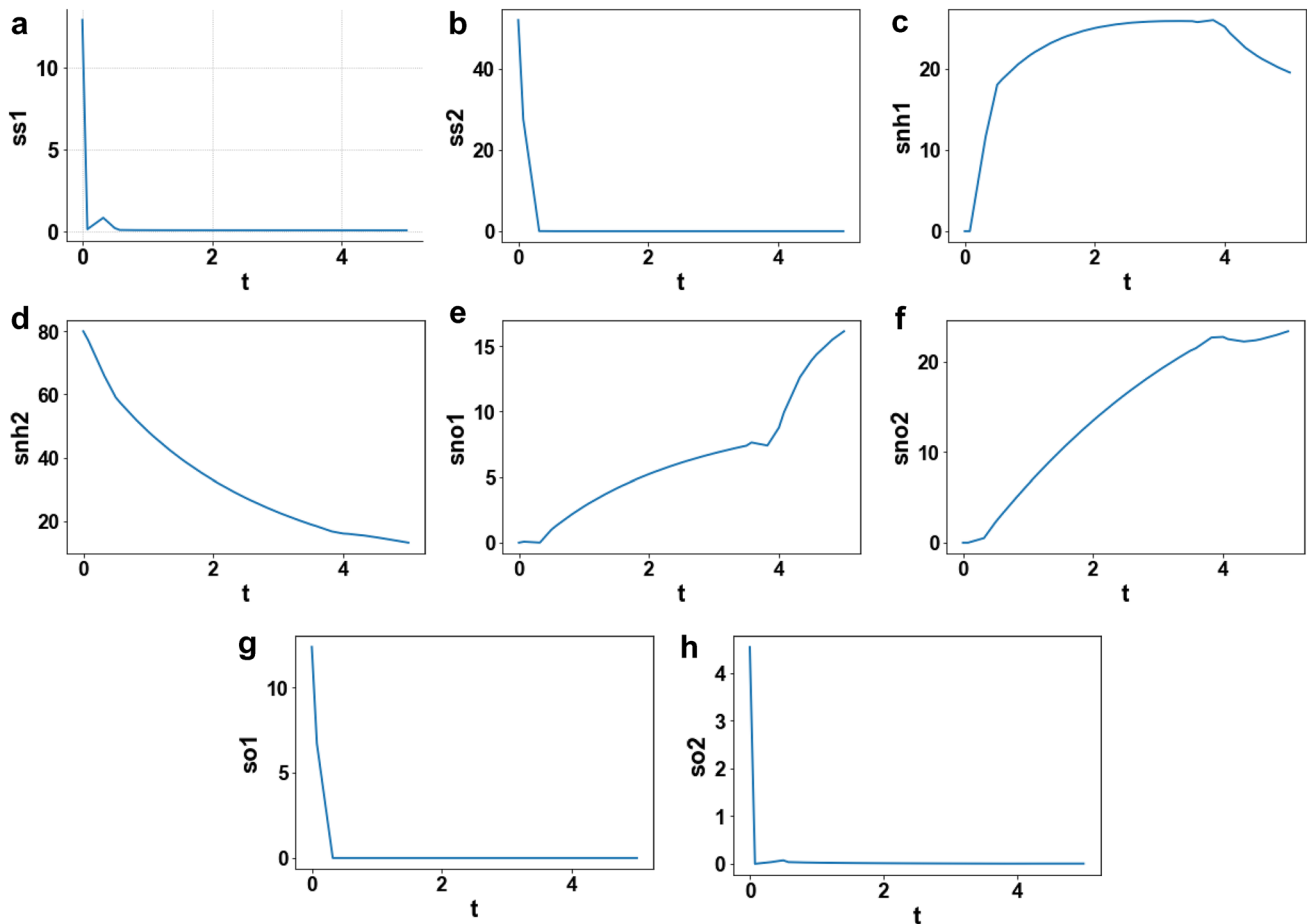


Fig. 3 **a** Minimizing $Sno2(Ss1$ vs $t)$, **b** Minimizing $Sno2(Ss2$ vs $t)$, **c** Minimizing $Sno2(Snh1$ vs $t)$, **d** Minimizing $Sno2(Snh2$ vs $t)$, **e** Minimizing $Sno2(Sno1$ vs $t)$, **f** Minimizing $Sno2(Sno2$ vs $t)$, **g** Minimizing $Sno2(So1$ vs $t)$, **h** Minimizing $Sno2(So2$ vs $t)$

single objective optimal control while controlling all the variables.

- The Pareto curve 5a shows a spike. This indicates that the wastewater treatment problem is highly nonlinear and local optimal strategies may not be as effective as global optimization strategies in obtaining the most beneficial solution.

The objective of this work was to minimize Nitrite/Nitrate, ammonium, and biodegradable organic contaminants. It was seen that in the case of the Nitrite/Nitrate and the biodegradable contaminants the multiobjective and single objective profiles were qualitatively similar, but this was not true in the case of the ammonium compounds. However, increasing the time of the operation for the multiobjective optimal control caused the profiles for the ammonium

contaminant to be qualitatively similar. This implies that by increasing the time of the operation the multiobjective optimal control could be made as effective as the single objective optimal control operation with the added advantage of being able to control all the variables. This is very beneficial to industry workers who can effectively purify the wastewater as much as possible before it is discharged into the environment.

Conclusions

Single and multiobjective optimal control using BARON which is a state of the art global optimization solver of the Wastewater treatment process is performed. The main aim of this optimal control was to minimize the Nitrite/Nitrate, ammonium and biodegradable organic contaminants. It is

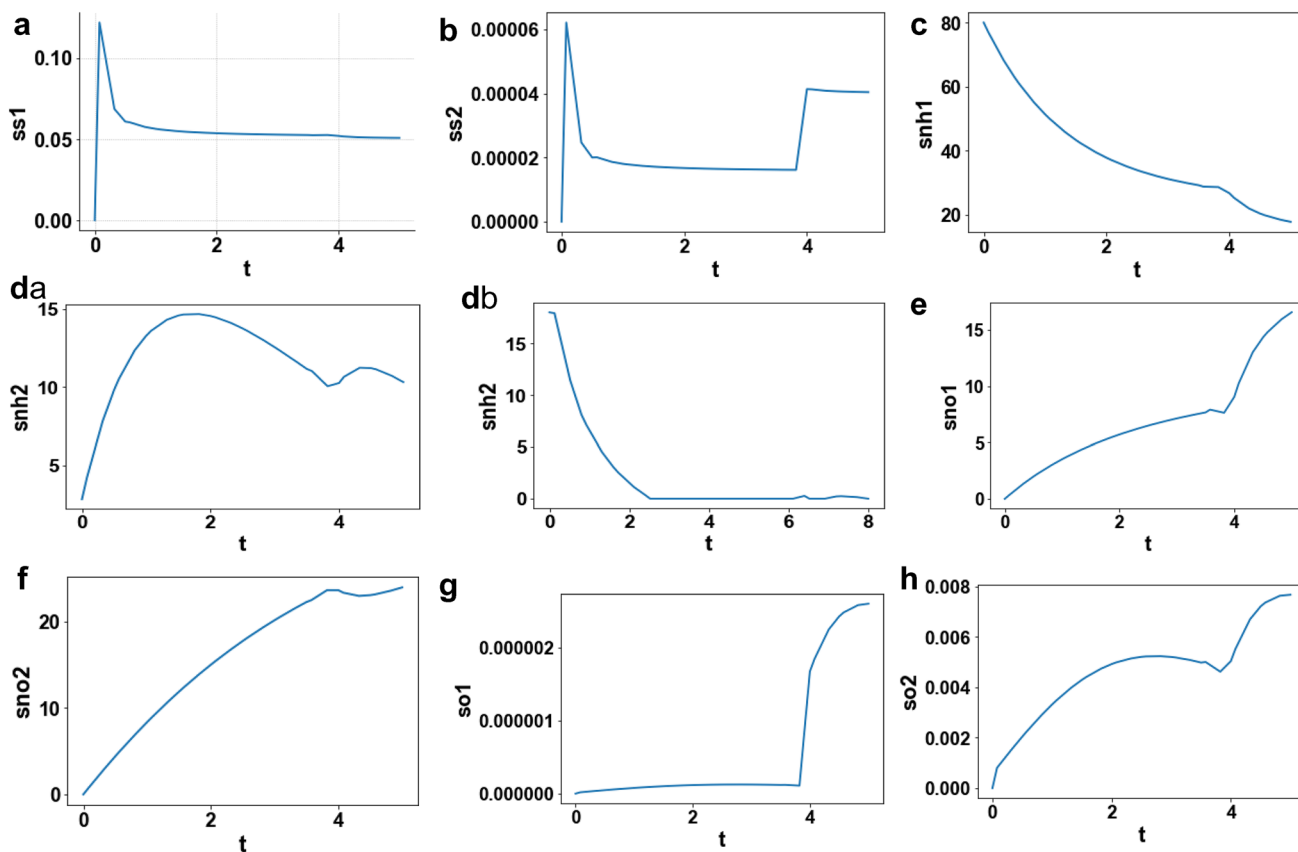


Fig. 4 **a** Multiobjective Optimal Control(Ss_1 vs t), **b** Multiobjective Optimal Control(Ss_2 vs t), **c** Multiobjective Optimal Control(Snh_1 vs t), **d-a** Multiobjective Optimal Control(Snh_2 vs t , $t=5$), **d-b** Multiobjective Optimal Control (Snh_2 vs t) with total time increase, **e** Multiobjective Optimal Control(Sno_1 vs t), **f** Multiobjective Optimal Control(Sno_2 vs t), **g** Multiobjective Optimal Control(So_1 vs t), **h** Multiobjective Optimal Control(So_2 vs t)

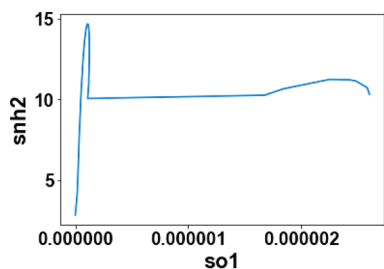


Fig. 5 Pareto Curve generated by multiobjective optimal control (Snh_2 vs So_1)

shown that by increasing the time of the operation the multiobjective optimal control could be made as effective as the single objective optimal control operation with the added advantage of being able to control all the variables. This provides an effective strategy for industries to obtain a more beneficial and to minimize the contaminants and ensure that

the wastewater discharged into the surroundings is not harmful to the environment.

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Declarations

Conflict of interest There is no conflict of interest. All the codes of ethical conduct have been adhered to.

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