**RESEARCH ARTICLE**



# **Structural health monitoring: detection of concrete faws using ultrasonic pulse velocity**

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#### **Abstract**

Assessment of concrete structures is a mandatory task of engineers to keep the structure safe and well performing. Nondestructive testing methods seem to be more appropriate for this task. They are reliable, quick and does not cause signifcant damage to the concrete surface. The non-destructive testing application is an interesting strategy to monitor the conditional strategy of the concrete structures, such as homogeneity of material, density, strength evolution, internal defects and faws in concrete structures. This paper aims to study the homogeneity of the concrete structure using contour mapping technique by interpretation of ultrasonic pulse velocity values. For this study, the authors performed an ultrasonic pulse velocity test on beams and columns of an old car parking shed made of reinforced cement concrete. The contour map generated using velocity values of UPV shows the level of homogeneity. The variations of color in the contour map indicate the presence of defects in the structure. This interpretation strategy will be a suitable tool for assessment of concrete structures.

**Keywords** Concrete · Ultrasonic pulse velocity · Contour mapping · Homogeneity · Defects · Interpretation

# **1 Introduction**

Reinforced concrete structures play an essential part in the infrastructures in most of the countries. Many of the older structures which were built in the frst half of the 20th century are reaching the end of their expected designed life. However, when the concrete structures are not appropriately designed, or production process is inadequately controlled, faws and defects can be introduced early in the structure. All this defects and flaws can affect the performance and reduce its strength and durability. The worst part is that some of the newer structures were also showing early deterioration and defects. This increasing problem of deterioration gives a strong argument for the validation on new techniques and development of new tools that would allow us to monitor the conditional state of elements and provides the data on their potential service life [[1\]](#page-7-0).

In these situations, the application of non-destructive techniques proven to be very suitable for these tasks to check and monitor the conditional state of the constructed

 $\boxtimes$  T. Barkavi barkavicivil@gmail.com facilities, because these are quick, accurate and does not cause any considerable damage to the concrete surface.

The use of these non-destructive technique methods will provide important data to design a preventive maintenance plan. It is known that the early detection of problems for a quick evaluation and intervention.

Over the last few decades, they are several NDT methods came into force from laboratory curiosities to quality control tools in diferent productive felds. These methods are nowadays commonly using to check the homogeneity in structure, changes in surface, the presence of cracks or other physical characters [[2\]](#page-7-1).

Among all NDT techniques available in the market for damage detection and defects detection, Ultrasonic pulse velocity are being used widely in civil engineering applications because of low cost, ease of operation and this test does not cause any considerable damage to the concrete structure surface [\[3\]](#page-7-2).

## **2 Non‑destructive testing**

NDT equipment can be used to assess the condition of existing structures. NDT methods are those that do not cause any damage to the element tested or leave only small lesions that can be rectifed easily after the testing is done. In the case of

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newer structures, these tests can be used for monitoring the strength evaluation and to check the quality of the concrete. The properties that can be evaluated with these techniques are density, uniformity, modulus of elasticity and compressive strength, surface hardness, the location of re-bars, gaps and cracks can also be investigated. The main reason for the development of these techniques is the dynamic nature of the concrete quality. The application of these techniques is an interesting tool to check and control these structures. Strength and durability are the most used property to control concrete structures.

Several specifc situations in which the use of NDT methods can be considered attractive [\[4](#page-7-3)]:

- quality control of construction in situ;
- assessing the potential durability of the concrete;
- determining the position and quantity of reinforcement;
- for the assessment of post-earthquake structural integrity;
- determining the concrete uniformity;
- modification to an existing structure;
- monitoring of structures affected by external works;
- monitoring long-term changes in concrete properties;
- location and determination of the extension of cracks, voids, honey-combing and similar defects within the concrete structure.

One of the objectives of development NDT techniques is a reliable assessment of concrete defects even when there is an accessible only from a single surface. The application of NDT demand will probably increase since aging infrastructure requires regular inspection for early planning of repair and intervention, to ensure structural performance and human safety.

# **3 Ultrasonic pulse velocity testing**

Ultrasonic pulse velocity (UPV) method is useful and fexible which allows in-depth analysis of homogeneity of the material. Using UPV, it is possible to determine concrete homogeneity, quality, deterioration, the presence of internal flaws and voids [[5\]](#page-7-4).

Ultrasonic pulse velocity can be used for the detection of cracks and faws in hardened concrete [[6\]](#page-7-5). In concrete, the standards establish procedures for carrying out the tests, which is performed in order to determine the modulus of elasticity and monitor the variations of concrete characteristics [\[7\]](#page-7-6). Using UPV, and to make comparisons with reference specimens, it may estimate potential compressive strength.

For concrete tests, the method is usually based on the use of portable equipment, composed by the source/detector unit and handheld surface transducers, which works in the frequency range of 25–60 kHz [\[8](#page-8-0)]. The evaluation of UPV is a complex activity, which requires careful data collection and expert knowledge and sensitivity to obtain a diagnosis. To map the homogeneity of concrete in a structure, it is necessary to interpret and connect a large number of UPV data  $[5]$  $[5]$ .

#### **3.1 Probing methods of UPV**

Three diferent types of probing methods usually followed to record the experimental values are (Fig. [1\)](#page-1-0):

- *Direct or cross probing:* The receiver and the transmitter are opposite to each other on either side of the cube.
- *Semi direct probing:* The transmitter and the receiver are on any two perpendicular faces of a cube.
- *Indirect or surface probing:* The receiver and transmitter are on the same face of a cube.

## **3.2 Path length**

Theoretically, the path length traveled by the wave and the frequency of the wave (which is the same as the frequency of the transducer) should not affect the propagation time; therefore, they should not afect the pulse velocity. However, in practice, smaller path lengths tend to give more variable and



<span id="page-1-0"></span>**Fig. 1** Pulse velocity measurement confgurations. **a** Direct method. **b** Semi-direct method. **c** Indirect surface method. (Courtesy—Naik et al. [[9\]](#page-8-1))

slightly higher pulse velocity because of the inhomogeneous nature of concrete [[10\]](#page-8-2). RILEM [\[11](#page-8-3)] has recommended the following minimum path lengths:

- 1. 100 mm for concrete having a maximum aggregate size of 30 mm.
- 2. 150 mm for concrete having a maximum aggregate size of 45 mm.

# **3.3 Application of UPV to detect concrete uniformity**

Heterogeneities in the concrete within or between members cause variations in pulse velocity which, in turn, are related to variations in quality. Measurements of pulse velocity provide a means of studying the homogeneity and, for this purpose, a system of measuring points which covers uniformly the appropriate volume of concrete in the structure has to be chosen. The number of individual test points depends upon the size of the structure, the accuracy required. In a large unit of reasonably uniform concrete, testing on a 1 m grid is usually adequate but, on small units or variable concrete, a fner grid may be necessary. It should be noted that, in cases where the path length is the same throughout the survey, the measured time may be used to assess the concrete uniformity without the need to convert it to velocity. This technique is particularly suitable for surveys where all the measurements are made by indirect transmission, i.e., surface probing. It is possible to express homogeneity in the form of a statistical parameter such as the standard deviation or coefficient of variation of the pulse velocity measurements made over the grid [[12](#page-8-4)]. The qualitative way of representing the homogeneity of concrete is by contour mapping of the raw UPV data collected by indirect transmission.

#### **3.4 Collection of UPV data**

The phase-III of this program was conceived with the idea of detecting the unseen cracks and voids in concrete using ultrasonic pulse velocity testing. For this purpose, the data are to be collected using surface probing methods. The beam was tested using portable PUNDIT equipment, provided with 54 kHz transducers. According to Naik and Malhotra [\[13](#page-8-5)], this frequency range was chosen to minimize the influence of concrete strength variations in the measurements. The path length is made fxed as 100 mm for data collection, and a grid of  $150 \times 150$  mm squares is used. The coordinates for the contour map were the position in the grid (expressed as X and Y values) and the UPV measurement (which constituted the Z coordinate). Each indirect measurement was placed at the midpoint of the line connecting two points of the grid. Adequate intervals were stipulated for the UPV measurement, in order to create a contour map of the pulse velocities alongside the beam.

#### **3.5 Contour map**

Contour map displays lines/gradients that include points of equal value and separate points of higher value from points of lower value. In this research, a contour map of ultrasonic pulse velocities are generated, which can identify and map voids, honeycomb, cracks, delamination, and other damage in concrete. Imaging is implemented in many felds and has been demonstrated to be a powerful tool for the interpretation of signal data. However, imaging for the evaluation of concrete structures has not been widely used. The generated image shows where the readings are lower or higher. When no variations are shown, concrete is homogenous. Figure [2](#page-2-0) represents the contour map generated on a concrete surface.

#### **4 Structure inspection**

The car parking shed is an RCC framed structure consisting 2-bays along length and 1-bay in other direction. The apparent height of the column is 180 cm on an average, totally 6 columns. The length of the beam is varying from 290 to 323 cm. This car parking shed was constructed by students itself. The depth of beams 30 cm. The thickness of a brick layer on beam as parapet is around 11 cm. The grade of concrete used for construction is around M20. Figure [3](#page-3-0) shows a photograph of a car parking shed. The AutoCAD drawings of car parking shed with measurements and designation to beam and columns are shown in Figs. [4](#page-3-1) and [5.](#page-3-2)

#### **4.1 Visual observation**

- Corrosion of rebar.
- Cracks occurred at some places in the beam.
- Spalling of the concrete slab.
- Dampness and fungal growth are observed at some locations.



<span id="page-2-0"></span>**Fig. 2** Contour map generated on a concrete surface (Courtesy—Lorenzi 2014)



**Fig. 3** Photograph of car parking shed located at NITT

<span id="page-3-0"></span>

<span id="page-3-1"></span>**Fig. 4** Auto-CAD sketch of car parking showing the designation to beams



<span id="page-3-2"></span>**Fig. 5** Auto-CAD sketch of car parking showing the designation to columns

# **5 Data collection procedure**

To ensure the proper positioning of the transducers during the UPV data collection, a grid sheet of  $10 \times 10$  cm



**Fig. 6** Grid sheet of  $10 \times 10$  cm size for UPV testing

<span id="page-3-3"></span>

**Fig. 7** Photograph taken while testing of the structure

<span id="page-3-4"></span>was prepared which is shown in Fig. [6](#page-3-3). During the UPV test, the indirect measurement/surface probing methodology was adopted. The aim of this study is to verify the concrete homogeneity of concrete in the car parking shed using UPV, by mapping the data collected. The photograph taken while testing the structure was shown in Fig. [7](#page-3-4).

# **6 Data mapping**

For mapping of the UPV data, the location of the transducer in the grid is expressed as X and Y co-ordinates, and the UPV values are expressed as Z co-ordinate. Each indirect measurement is placed at the midpoint of the line connecting two points of the grid i.e., the indirect UPV measurements are taken at four points that made up a grid cell. The four values corresponding to readings taken between two neighboring horizontal or vertical grid points were positioned at the midpoint between these points. The map generated by plotting these values will show the variations in concrete homogeneity, determining region inside the concrete presenting diferences in homogeneity. The pattern of data collection is represented in Fig. [8.](#page-4-0)



**Fig. 8** Illustration showing the position of transducers and data collection points of UPV testing

#### <span id="page-4-0"></span>**6.1 Fixing gradient for contour map**

The contour gradient for the contour map is fxed based on the ultrasonic pulse velocity criterion given in Table [3](#page-6-0) of Condition Assessment of Buildings for Repair and Upgrading, Prepared under-GoI-UNDP Disaster, Risk Management Programme National Disaster Management Division Ministry of Home Afairs, Government of India New Delhi June 2007. Table [1](#page-4-1) gives the velocity criteria and the corresponding concrete quality and contour gradient.

### **7 Results**

As a prior test to prove the capacity of UPV to detect the homogeneity of the concrete surface, three concrete specimens, i.e., one cube of  $10 \times 10 \times 10$  cm, one cube of  $15 \times 15 \times 15$  cm and one beam of  $10 \times 10 \times 50$  cm were built with objects inside to simulate the imperfections or defects that could exist in the real structural elements.

Figure [9](#page-4-2) shows the casted specimens. The mix proportion used to prepare the concrete specimens were 1:1.41:2.69 (cement:natural river sand:coarse aggregate), with a w/c ratio of 0.37, this resulted in a slump of 20 mm and a 28thday compressive strength of 48.25 Mpa, i.e., M40 grade of concrete.

After the curing period, a mesh of  $5 \times 5$  cm has drawn on all the specimens to proceed the UPV reading of the concrete specimens. UPV tests were performed with a portable device, equipped with low frequency (54 kHz) transducers.

To ensure proper positioning of the transducers during the UPV readings, a grid of  $5 \times 5$  cm lines were drawn (i.e., a measurement sheet was used to ensure the perfect positioning of the transducers during the tests) on all sides of all specimens. Figure [10](#page-5-0) shows the grids that are drawn for proper positioning.

The main objective of the test was to collect UPV data and verify if the analysis of the results could allow the



15\*15\*15 cm cube with tennis balls of 4cms diameter

 $10*10*10$  cm beam with thermocol pieces inside the specimen at three locations

<span id="page-4-2"></span>**Fig. 9** Casted specimens

S. no.	UPV value in $km/s$ (V)	Concrete quality	Color gradi- ent (chosen by author)
	V greater than 4.0	Very good	White
$\overline{c}$	V between $3.5$ and $4.0$	Good, but may be porous	Green
3	V between $3.0$ and $3.5$	Poor	Red
4	V between $2.5$ and $3.0$	Very poor	Yellow
5	V between 2.0 and 2.5	Very poor and low integrity	Orange
6	V less than 2.0 and reading fluctuating	No integrity, large voids suspected	Blue

<span id="page-4-1"></span>**Table 1** Velocity criteria and the corresponding concrete quality and color gradient

<span id="page-5-0"></span>**Fig. 10 a**, **b** Grids are drawn on specimens

 $(b)$ 



<span id="page-5-1"></span>**Fig. 11** Core samples of a concrete cube of the  $15 \times 15$  cm showing the presence of tennis ball inside concrete





<span id="page-5-2"></span>**Fig. 12** A core sample of beam showing the presence of thermocol



<span id="page-5-3"></span>**Table 2** Description of specimens and objects used to induce defects in specimens



<span id="page-6-0"></span>

detection of the objects inside the specimens. Surface mapping technique was used to interpret of results, as reported by Lorenzi et al. [[14\]](#page-8-6).

In order to check if the diagnosis made using the contour map images was reliable or not, the samples were broken at the points chosen based solely on the contour images. Figures [11](#page-5-1) and [12](#page-5-2) show the suspicious zones which are coinciding with the concrete specimen defects, providing clear evidence that the defects were accurately located using only NDT data, with no prior knowledge of their position.

Since the results got with known deformities are pleasing the car shed was tested for its concrete homogeneity. The contour map obtained for every element of the car shed is tabulated below in Table [3.](#page-6-0) Refer Figs. [4](#page-3-1) and [5](#page-3-2) for beam and column designations.

On referring Table [2](#page-5-3) for the concrete quality, the cube of  $15 \times 15$  cm is showing the contour map pointedly with blue color which depicts the exact location of the tennis ball knowingly kept inside the concrete cube. The beam contour map has three points of blue color on the beam showing the position of thermocol placed in the concrete beam.

From Table [3](#page-6-0), it is understood that beams are having more internal voids and cracks widespread. Among them FB-1, BB-1 and SB-2 are having more damages than other beams. Among the columns, C-4 has more damages whereas, other columns are in good condition.

The beams and columns are having more amount of green color where the concrete is said to be good. The red colour in the beam and column indicate that the concrete is poor the yellow color in the beam and column indicate that the concrete is very poor.

# **8 Conclusions**

This study shows that the interpretation strategy of UPV data by contour mapping is adequate to organize and analyze the results. The use of grids to collect data works well and ensures that no region was overlooked. And from the contour map, the presence of non-homogeneous spots inside the structure is well identifed. This study demonstrates that the UPV test can be used to locate the pattern of cracks in the concrete structure. In the car parking shed, the contour map of the beam shows signifcant variations in color which shows heterogeneity in the concrete. Whereas in columns, the contour map has a widespread green color which means the velocity variation between 1500 and 3500 m/s which indicates moderate strength. Hence the column is in good condition, but beam needs immediate repair. The results of this study indicate that UPV, combined with the generation of surface contour maps may detect heterogeneity in concrete structures if any and these techniques contribute to a quick condition assessment of concrete structures. This study indicates that UPV tests are sensitive to variations in density and homogeneity and therefore may supply essential data for decision-making on the conditions of concrete structures, i.e., UPV tests can contribute for the quality control of concrete structures. The generated maps for the columns by UPV data indicates the level of homogeneity is more, whereas the contour map of beam shows significant variations in color which shows heterogeneity in the concrete surface as well as internally. Visualization tools may indicate the condition of the concrete structure, suggesting that this may be the best strategy to perform accurate analyses. The  $10 \times 10$  cm grid seemed to be useful in the case of columns, and  $15 \times 15$  cm showed effective in the case of beams. Hence, grid size depends upon the accuracy we want and also sometimes depends upon the size of the element under consideration.

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# **References**

- <span id="page-7-0"></span>1. Kulkarni DK, Teke Sudhakar S (2012) Health assessment of reinforced concrete structures—a case study. IOSR J Mech Civ Eng 12(6):37–42
- <span id="page-7-1"></span>2. Ongpeng J (2017) Ultrasonic pulse velocity test of reinforced concrete with induced corrosion. ASEAN Eng J 6(1):5–12
- <span id="page-7-2"></span>3. Adamatti DS, Lorenzi A, Chies J, Silva Filho LCP (2017) Analysis of reinforced concrete structures through the ultrasonic pulse velocity: technology parameters involved. IBRACON Struct Mater J 10(2):358–385
- <span id="page-7-3"></span>4. Lorenzi A, Fonseca Caetano L, Campagnolo JL, Silva Filho LCP (2012) Analyzing two diferent data processing strategies for monitoring concrete structures using ultrasonic pulse velocity. Revista Alconpat Int 2(3):182–194
- <span id="page-7-4"></span>5. Lorenzi A, Caetano LF, Chies JA, Silva Filho LCP (2014) Investigation of the potential for Evaluation of concrete faws using Nondestructive Testing Methods. ISRN Civil Eng 2014:11
- <span id="page-7-5"></span>6. International atomic energy agency (2002) IAEA-TCS-17: guidebook on non-destructive testing of concrete structures. Vienna, p 231
- <span id="page-7-6"></span>7. ABNT (2008) Concrete hardness—determination of density of waves in ultrasonic
- <span id="page-8-0"></span>8. Tafe A, Mayerhofer C (2003) Guidelines for NDT methods in civil Engineering. In: Proceedings of 5th international symposium on NDT in Civil Engineering, Berlin, Germany
- <span id="page-8-1"></span>9. Naik TR, Malhotra MV, Popovics JS (2004) The ultrasonic pulse velocity method. CRC Press
- <span id="page-8-2"></span>10. Jones R (1962) Non-destructive testing of concrete. Cambridge University Press, London
- <span id="page-8-3"></span>11. RILEM (1972) Recommendation NDT, testing of concrete by the ultrasonic pulse method. Paris
- <span id="page-8-4"></span>12. BS 1881: Part 203: 1986 - Testing concrete – Recommendations for measurement of velocity of ultrasonic pulses in concrete
- <span id="page-8-5"></span>13. Naik TR, Malhotra VM (1991) The ultrasonic pulse velocity method. In: Malhotra VM, Carino N (eds) Handbook on non destructive testing of concrete. CRC Press, p 343

<span id="page-8-6"></span>14. Lorenzi A, Silva Filho LCP, Lorenzi LS, Shimomukay R, Chies JA (2011) Monitoring Concrete structures through UPV results and Image analysis. e-Journal of Nondestructive Testing, Germany

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