**RESEARCH PAPER**



# **The Impact of a Large amount of Ultra‑fne Sunfower Ash With/ without Polypropylene Fiber on the Characteristics of Sustainable Self‑compacting Concrete**

**Rawaa Dheyaa Saleh1 · Nahla Hilal1 · Nadhim Hamah Sor2,3**

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## **Abstract**

The performance of self-compacting concrete (SCC) manufactured from ultra-fne sunfower ash (UFSFA) and reinforced by waste polypropylene fbers has been experimentally investigated. In this study, three diferent UFSFA percentages of (0, 25, and 50%) by weight were used as cement replacements with two variable lengths of polypropylene fber of 1% by volume. All the SCC mixes were examined for slump fow diameter and time, V-shape fow time, L-box height ratio, fresh density and segregation resistance depending on EFNARC procedures. As well as the dry density, fexural and compressive strengths, water absorption and ultrasonic pulse velocity (UPV) were performed to indicate the hardened characteristics. The fndings revealed that the workability of the mixtures was reduced specially for the longer fber incorporation which the slump fow diameter decreased about 21%, but all the results of the tests were within the standard limitations except for the L-box test. The outcomes of compressive and fexural strengths declined with increasing content of UFSFA about 38 and 43%, respectively, but the strengths developed with the addition of fber about 27 and 50% for the compressive and fexural strengths, respectively. The utilization of UFSFA and polypropylene fber had a negative efect on water absorption and UPV values. Depending on the UPV results, all the mixtures can be considered good quality concrete.

**Keywords** Self-consolidating concrete · Ultra-fne sunfower ash · Workability · Hardened properties · Water absorption · Ultrasonic pulse velocity

# **1 Introduction**

In developing countries, the problem of waste accumulation can cause many environmental and health issues. With the recent trend toward sustainability in all research areas, civil engineers are looking for new and innovative materials to

 $\boxtimes$  Nahla Hilal nahla.naji@uofallujah.edu.iq Rawaa Dheyaa Saleh Rawaa.saleh@uofallujah.edu.iq Nadhim Hamah Sor

nadhim.abdulwahid@garmian.edu.krd

- <sup>1</sup> Department of Scholarships and Foreign Relation Fallujah, University of Fallujah, Fallujah, Iraq
- <sup>2</sup> Civil Engineering Department, University of Garmian, Kurdistan Region, Kalar, Iraq
- Department of Civil Engineering, Harran University, Sanlıurfa, Turkey

substitute cement. It is responsible for generating  $CO<sub>2</sub>$  emissions during its production as stated by Hilal et al. [2021b](#page-12-0)). As illustrated by Singh et al. [\(2013\)](#page-12-1), producing one ton of cement generates one ton of  $CO<sub>2</sub>$ . Accordingly, the trend of utilizing waste materials in concrete has been increasing in order to eliminate the utilization of cement as well as its environmental and health problems. Various kinds of waste materials produced from agricultural products have attracted researcher's attention due to their availability. Thus far, various kinds of agro waste materials have been used as minerals or admixtures in concrete. For example, Fathi and Fathi [\(2014\)](#page-12-2) studied the efect of sugar beet fber and tragacanth gum on the characteristics of clean concrete, the use of rice husk ash was also examined by Failed [\(2018\)](#page-12-3); Sor and Hamah [2018;](#page-13-0) Öz et al. [2017\)](#page-12-4). In addition, recycled carrot powder was used as a cement replacement by Al-Zubaidi ([2015](#page-12-5)). The mechanical properties of concrete produced with coconut shell ash were experimentally investigated by Adebakin et al. ([2019\)](#page-11-0), Hilal et al. [\(2021a\)](#page-12-6), and Oyedepo et al. [\(2015\)](#page-12-7). Moreover, palm oil fuel ash and fy



ash (Thomas et al. [2017](#page-13-1)), waste ceramic powder (Sor et al. [2021](#page-12-8)), metakaolin (Bheel et al. [2021b](#page-12-9)), egg shell ash (Aadi et al. [2021](#page-11-1)) and ground granulated blast furnace slag ("Utilization of Corn Cob Ash as Fine Aggregate and Ground Granulated Blast Furnace Slag as Cementitious Material in Concrete." [2021a](#page-12-10)) were used as a Pozzolanic mineral admixture in the production of sustainable concrete. The siliceous or siliceous aluminous materials have a pozzolanic activity. However, these materials show little inherent cementitious behavior alone, they can react with the saturated calcium hydroxide (C–H) to produce calcium silicate hydrates (C–S–H). In cement hydration, the C–H and C–S–H are considered the main compounds (Omrane et al. [2017\)](#page-12-11). The role of the pozzolanic material is to form the secondary C–S–H to which the strength of the matrix is attributed. Replacing cement by a pozzolanic material delays the hydration at the beginning but increases the strength later. Recently, many researchers have established experimental works to identify the role of the pozzolanic materials in producing green concrete (Memon and Khan [2018](#page-12-12); Mermerdaş et al. [2020;](#page-12-13) Abed et al. [2021](#page-11-2); Mohammed et al. [2021](#page-12-14); Zeynep et al. [2019](#page-13-2); Yu et al. [2018](#page-13-3)).

Raji and Samuel  $(2015)$  $(2015)$  $(2015)$  investigated the effect of using egg shell as a fne aggregate in concrete, which the normal fne aggregate was totally replaced with egg shell. The outcomes revealed that the use of egg shell signifcantly decreased the compressive strength.

Tan et al. [\(2018](#page-13-4)) studied on the effect of egg shell powder as a cement replacement of 5, 10, 15 and 20% on concrete properties. The results revealed that the fexural and compressive strengths increased about 51.1 and 57.8%, respectively, for the optimum ratio 15%. Also, the water absorption was reduced about 50%. However, the increasing of egg shell powder reduced the compressive strength while immersing in the acidic and alkaline solutions. The reason of that is due to the high calcium content of the egg shell, which reacts easily in the acid and alkaline environment.

Borhan et al. ([2020](#page-12-16)) studied on the effect of various types of fbers from waste materials (wood, polyvinyl chloride, aluminum, and iron fling) on fresh and hardened characteristics of SCC with diferent volume ratios. From the outcomes, it is concluded that the compressive and fexural strengths increased and the workability reduced with increasing of all fbers content in SCC mixture.

Parthasarathi et al. ([2017\)](#page-12-17) experimentally studied the impact of using silica fume in addition to egg shell powder as a partial substitution of cement. The replacement proportions of egg shell ash were (5, 10 and 20%) of cement weight and for silica fume were (2.5, 5 and 7.5%). The fndings revealed that adding only egg shell powder to the mixture increased the compressive strength by about 15%. Further improvements on compressive strength were recorder using both egg shell powder and silica fume, however from an



economic point of view, the use of egg shell powder only is sufficient for obtaining high strength. It is also found that increasing the percentage of egg shell powder in the mixture reduces the split tensile strength and this reduction will continue if the reinforcement is used with concrete.

Many researchers investigated the use of rice husk as a partial substitute to cement in concrete manufacture. Asman et al. (Failed [2017\)](#page-12-18) conducted an experimental study on the use of egg shell and rice husk as a partial cement replacement. It can be concluded that at day 7, the compressive and fexural strength of concrete samples made with egg shell powder and rice husk were lower than that of the control sample. However, an improvement in the strength between day 7 and day 28 was noticed and this is due to the high amount of amorphous silica that gives positive infuence to the strength of concrete.

Al-Hadithi et al. (Al-Hadithi [2019\)](#page-12-19) reported that the slump flow diameter and L-box height ratio of SCC decreased and the strengths increased with increasing the waste Polyethylene terephthalate content which is used as a fiber.

Señas et al.  $(2012)$  $(2012)$  $(2012)$  experimentally studied the effect of sunflower ash as a cement replacement of 5, 10 and 15% by weight, for cement mortar production. The results indicated that the fuidity and the compressive strength of the mixture decreased with the increasing sunfower ash content. Moreover, the initial and fnal setting times were delayed compared to the control specimen.

Sisman and Gezer ([2013a\)](#page-12-21) established an experimental study to examine the efect of sunfower seed husk (SSH) on the characteristics of concrete. The SSH was used as a partial replacement of aggregate by using 5, 10, 20 and 30%. The results showed that the density, compressive strength, and tensile strength were decreased, but the rate of water absorption was increased at 28 and 90.

Fleysher et al. ([2015\)](#page-12-22) conducted an experimental research to examine the infuence of sunfower oil waste on the properties of cement. It can be concluded that the sunfower oil waste can be utilized as a chemical admixture in cement mortars or concrete. The test admixture causes a decrease in water absorption by about 34%. It is indicated that the admixture resulted from food waste processing could be added as a plasticizer in concrete manufacture.

According to the previously reviewed literature, using agricultural wastes in concrete results in compressive and tensile strength reduction. In order to overcome this problem, plastic fbers have been widely used for this purpose. Mohammed et al. (Mohammed and Rahim [2020\)](#page-12-23) studied the efect of PET waste fber on the behavior of high strength concrete. The proportions of the added PET were 0.75 and 1% of the mix volume. The fndings showed an improvement in the cracking performance without compressive strength enhancement.

From the previous studies, it is concluded that all types of fbers such as polyethylene, wood, polyvinyl chloride, aluminum, iron fling, sugar beet, basalt, steel and polypropylene with diferent volume ratios had a negative efect on fresh properties and positive on hardened characteristics of SCC (Fathi and Fathi [2014;](#page-12-2) Borhan et al. [2020;](#page-12-16) Al-Hadithi [2019](#page-12-19); Mohammed and Rahim [2020](#page-12-23)).

Generally, there were a restricted number of researchers utilized SFA as a cement or fne aggregate replacement in the mortar or conventional concrete production (Señas et al. [2012](#page-12-20); Sisman and Gezer [2013a;](#page-12-21) Fleysher et al. [2015](#page-12-22); Mohammed and Rahim [2020\)](#page-12-23) then the using of SFA required more examinations specially with waste polypropylene fber in SCC.

# **2 Research Signifcance**

Due to the great amount of cement required for producing SCC, using agricultural waste materials as a partial replacement for cement and recycling waste polypropylene as fber can result in economical and sustainable concrete. In addition, limited research on the use of SFA in SCC has been undertaken. Hence, this research tries to study the mechanical properties of SCC manufactured with ultra-fne sunflower ash (UFSFA) reinforced by plastic fibers. UFSFA has been added using three various proportions of (0, 25, and 50%) and the polypropylene fbers were added at 1% of the total mix volume. Previously, the combination efect of using UFSFA with waste polypropylene fbers on SCC characteristics was not investigated.

## **3 Experimental Program**

#### **3.1 Materials**

Ordinary Portland Cement (OPC), commercially known as (AL-mass) conforming with the Iraqi Standard Specifcation No.5/1984 (Iraqi Specifcations [1984\)](#page-12-24), was used in this research. Agricultural ash extracted from burning sunfower shells was also utilized as a partial substitute to cement. Sunfower Ash (SFA) has passed through various treatment processes to obtain Ultra-fine Sunflower Ash (UFSFA) and enhance its pozzolanic activity, as shown in Fig. [1](#page-2-0) and demonstrated by using SEM in Fig. [2.](#page-3-0) Table [1](#page-3-1) presents the chemical composition of OPC and UFSFA. Natural fne and coarse aggregate were utilized. Figure [3](#page-3-2) shows sieving analysis for the fne and coarse aggregate. The used sand matches with the requirements of Iraqi standard specifcation No. 45/1984 (No [1984](#page-12-25)). For the purpose of mixing and curing, drinkable water was utilized. Polycarboxylic, which is a type of super plasticizer (SP), was used for producing SCC



**Fig. 1** Transforming RSF to UFSFA

<span id="page-2-0"></span>specimens. Plastic straps manufactured from polyethylene terephthalate (PET) were also used as reinforcing materials. They were cut into pieces using scissors with aspect ratio  $(L/D = 35.45)$  $(L/D = 35.45)$  $(L/D = 35.45)$  as shown in Fig. 4. They were added to the mixture as a percentage of 1% of the whole concrete mixture volume. Their specifc gravity, thickness and water absorption rate were  $1.300 \text{ kg/m}^3$ , 0.8 mm, and 0%, respectively, see Table [2.](#page-4-0)

## **3.2 Preparation of SFA and PP Fibers**

The following treatment procedure has been adopted for preparing SFA. Firstly, the original sunfower shells were water cleaned to remove all contaminants and suspended dust and hence let to dry in the sun for two days. Then, they were placed in an electrical oven at  $105 \pm 5$  °C for 24 h in order to remove moisture and enable the sieving process. Later, the dried SFA was sieved through a 300 µm sieve to remove coarse particles. In order to increase the surface area and improve the SFA pozzolanic reactivity, the SFA was crushed for fve hours using a ball mill. The previously crushed SFA was burnt at a temperature of  $500 \pm 50$  °C using a gas furnace for the purpose of removing unburned carbon, which has a negative effect on the pozzolanic characteristics. Thus, the original SFA converted into treated SFA. Finally, to produce ultra fne sunfower ash, further grinding to the treated SFA for 4 h has been undertaken. The polypropylene straps, which are locally available in large amount, were collected and manually cut into pieces.

## **3.3 Concrete Mix Design And Specimens Preparation**

The procedure of SCC production adopted by EFNARC (EFNARC [2005](#page-12-26)) in addition to technique used by Kheder et al. (2010) (Kheder and Al Jadiri [2010](#page-12-27)) was followed in this study. Mixing proportions of the SCC for one cubic meter are illustrated in Table [3](#page-4-1). For each mix, six





## <span id="page-3-0"></span>**Fig. 2** SEM of UFSFA

<span id="page-3-1"></span>**Table 1** Chemical composition of cement and UFSFA

Chemical composition %	<b>OPC</b>	<b>UFSFA</b>
	22	51
$\mathrm{SiO}_2$ $\mathrm{Fe}_2\mathrm{O}_3$	4	16
$\text{Al}_2\text{O}_3$	4	11
CaO	63	13
MgO	1.75	14
Loss of ignition		1.5
Others	7	6



<span id="page-3-2"></span>**Fig. 3** Sieve analysis for fne and coarse aggregate







**Fig. 4** Types of polypropylene fbers

<span id="page-3-3"></span>cubes with dimensions of  $15 \times 15 \times 15$  cm were made to calculate the compressive strength. Six Prism molds of  $100 \times 100 \times 500$  mm dimensions were also prepared to determine the flexural strength. Figure [5](#page-4-2) shows the casted cubes and prisms samples. All the molded samples were left for 24 h and then water cured at 20 °C until testing. Prior to casting, the fresh properties for the mixes were

<span id="page-4-0"></span>



calculated. The hardened properties, which involved dry density, compressive strength, flexural strength, absorption rate and UPV, were performed independently for each mix. The SFA has been used to partially replace cement. The percentages of replacement were 0, 25, and 50%. The water content was kept constant at 0.45. The plastic fibers were added to the mixture as a percentage of 1% of the whole concrete volume.

 $[M0]$  = SCC control mix.  $[M1] = SCC (75\% OPC, 25\% UFSFA).$  $[M2] = SCC (50\% OPC, 50\% UFSFA).$ [M3]=SCC (75% OPC, 25% UFSFA, 1% Fibers−7 cm).  $[M4] = SCC$  (75% OPC, 25% UFSFA, 1%) Fibers—9 cm).  $[M5] = SCC$  (50% OPC, 50% UFSFA, 1%) Fibers—7 cm).  $[M6] = SCC$  (50% OPC, 50% UFSFA, 1%) Fibers—9 cm).

#### **3.4 Testing Procedure**

The current research tries to assess the fresh and hard properties of SCC samples containing SFA and plastic fbers by investigating T50, slump, L-box, v-funnel, segregation and fresh density in addition to dry density, compressive strength, fexural strength, absorption rate and UPV. To obtain slump and T50, a slump cone was flled up with the mixed concrete without leveling and compacting as illustrated in EFNARC [\(2005\)](#page-12-26). Then, the cone is carefully lifted in the vertical direction. The average of two opposite direction was measured to calculate the slump fow diameter. In addition, T50 was also calculated by determining the time required for concrete to reach 500 mm spread diameter, as seen in Fig. [6.](#page-5-0)

For L-box test, an equipment consisting of vertical and horizontal rectangular parts was used with respective standard (EFNARC [2005\)](#page-12-26) procedures. The two parts were separated using a movable plate. The concrete mix was placed into the vertical part. When the movable gate was lifted, concrete few from the vertical part to the horizontal part. The ratio of the height of concrete in the horizontal part to that in the vertical part was calculated. This ratio indicates the passing ability of SCC.

In order to evaluate the fowability of the fresh mixture, V-shaped funnel equipment was utilized with respective standard (EFNARC [2005](#page-12-26)) procedures, see Fig. [7](#page-5-1). The test equipment was flled with the fresh concrete. Then, the mixture was left to completely flow out. The required time for complete discharge of the mix from the equipment was

<span id="page-4-1"></span>

#### <span id="page-4-2"></span>**Fig. 5** Casting of cubes and prisms samples

SCC for one cubic meter





min), as specifed by ASTM C78 (ASTM [2010](#page-12-29)).

for discharge.

to ASTM C138 [\(1986\)](#page-12-32).

**4 Test Results and Discussion**

**4.1 Fresh Properties of SCCs**

determine. An adequate concrete mix requires short time

For the compressive strength, the test was implemented via using 15 cm cubes at the age of 28 days. A hydraulic machine of 2000 KN was used to apply the loads accordance with ASTM C39 (ASTM C39M-20 [2020](#page-12-28)). In order to determine the flexural strength, prisms of  $100 \times 100 \times 500$  mm dimensions were examined using three point bending fexural machine. The load was applied at a rate of  $1.0 \ (N/mm^2)$ 

Before performing the compression strength test, the cubic samples were tested for a UPV was performed according to ASTM C597 (ASTM C597-16 [2016\)](#page-12-30). The absorption rate was determined using the cubic samples following the guidelines of ASTM C642 (ASTM [2008](#page-12-31)). Moreover, the cubes were also used to determine the dry density of the concrete samples, and the adopted procedure was according



**Fig. 6** Slump flow test

<span id="page-5-0"></span>

**Fig. 7** V-funnel test process

<span id="page-5-2"></span><span id="page-5-1"></span>**Table 4** Fresh properties results of SCCs



The slump flow diameter decreased gradually with increasing replacement level of UFSFA and increasing length of polypropylene fbers content as shown in Fig. [8](#page-6-0). However, the slump fow diameter reduced about 7 and 14% by using 25 and 50% of UFSFA as replacement of cement, respectively. The similar impact of sun fower ash on slump test is reported by researchers (Grubeša et al. [2019](#page-12-33); Shahbazpanahi

The results of fresh properties of SCC mixtures are given in Table [4.](#page-5-2) However, slump fow time and diameter, fresh







<span id="page-6-0"></span>Fig. 8 Difference of slump flow diameter and slump classes of SCC with diferent mixtures



<span id="page-6-1"></span>**Fig. 9** Difference of  $T_{50}$  for SCCs with different mixtures

and Faraj [2020](#page-12-34)). Meanwhile, the diameter of slump flow reduced furthermore by addition of polypropylene fber and the longer fiber had more effect on the reduction. The respective standard indicated three classes of slump fow diameter depending on the required application. All the results of this experimental were within the class range of SF1 except the control mix was within the class SF2 as shown in Fig. [8.](#page-6-0) The same efect of diferent fber content was reported by other researchers (Zeynep et al. [2019](#page-13-2); Karmegam and Kalidass [2019](#page-12-35)). Zeynep et al. (Zeynep et al. [2019](#page-13-2)) reported that the slump flow of mortar was reduced by increasing the steel and basalt fbers content.

On the other hand, the slump flow time  $(T_{50})$  of SCC mixtures increased by increasing replacement level of UFSFA with cement and polypropylene fber content as well as the longer length of fber elapsed more time as demonstrated in Fig. [9](#page-6-1). Generally, all the results of  $T_{50}$  for the mixtures satisfed the specifed range illustrated in the respective standard.

Moreover, from the results of slump flow time and diameter concluded that the using of this type of waste,



<span id="page-6-2"></span>**Fig. 10** Correlation between slump flow diameter and  $T_{50}$  of SCCs



<span id="page-6-3"></span>**Fig. 11** Results of V-funnel fow time of diferent SCCs mixtures

polypropylene as fber in SCC was suitable and satisfed the respective standard criteria despite of increasing the fowability time specially due to its length was long.

Furthermore, there are a good relationship found between the results of slump flow time and diameter expressed in  $R^2$ as shown in Fig. [10](#page-6-2).

#### **4.1.2 V‑Funnel Flow Time**

This test is another kind of tests which is used to indicate workability of SCCs. Moreover, the results of the effects of UFSFA and polypropylene fiber on V-shape flow time for SCC mixtures are presented in Table [4](#page-5-2) and explained graphi-cally in Fig. [11.](#page-6-3) However, the times required to flow the SCC through the V-funnel increased gradually with increasing UFSFA and polypropylene fber incorporation in the mixtures and elapsed further time for longer fber. According to the respective standard, there are two classes of viscosity and all the V-funnel results of SCC mixtures were within the VF2 except the control mix and satisfed the EFNARC criteria as shown in Fig. [11](#page-6-3). Clearly seen that all the SCC mixtures were categorized as VS2/VF2 except the control mix which is categorized as VS1/VF1 as shown in Fig. [12.](#page-7-0)





<span id="page-7-0"></span>**Fig. 12** V-funnel flow time versus  $T_{500}$  for SCCs

Furthermore, from the outcomes of V-funnel flow time, it is concluded that the using of this type of waste polypropylene as fber in SCC was suitable and satisfed the respective standard criteria despite of increasing the fowability time specially due to its length was long.

## **4.1.3 L‑Box Ratio**

Another fresh testing is L-box height  $(H_2/H_1)$  ratio which indicate the passing ability for SCC mixture. However, the fluidity of the SCC mixture will increase with increasing height ratio. The results of *H*2/*H*1 decreased with increasing UFSFA and fber content in the mixtures as presented in Table [4](#page-5-2) and explained in Fig. [13](#page-7-1). The using of polyethylene fber in SCC mixtures had more signifcant efect on the reduction in the height ratio results and lower than the minimum value indicated in EFNARC [\(2005](#page-12-26)) but the mixes without fber were within the specifed ranges explained in the respective standard. The reduction due to the UFSFA



<span id="page-7-1"></span>**Fig. 13** Results of L-box height ratio of diferent SCCs mixtures



## **4.1.4 Fresh Density**

The density of SCC ingredients had significant effect on the fresh density. However, the fresh density of SCC mixtures decreased by increasing UFSFA and polypropylene fbers utilization as illustrated in Fig. [14](#page-7-2). The same outcomes were reported by researchers (Grubeša et al. [2019](#page-12-33); Shahbazpanahi and Faraj [2020](#page-12-34)).

## **4.1.5 Segregation Analysis**

The segregation resistance test is one of the important tests required for SCC mixtures specially for higher slump flow classes according to EFNARC (EFNARC [2005\)](#page-12-26). Meanwhile, all results of segregation resistance obtained in this experimental were within the limitations specifed in the respective standard as presented in Table [4](#page-5-2) and demonstrated in Fig. [15.](#page-8-0) The segregation resistance enhanced as UFSFA incorporation increased in SCC mixtures. On the other hand, the segregation ratio increased with addition of polypropylene fber specially for the longer length of fber.

## **4.2 Hardened Concrete Properties**

The dry density, water absorption, ultrasonic pulse velocity, compressive and fexural strengths were performed for all SCC mixtures as shown in Table [5](#page-8-1).

#### **4.2.1 Compressive Strength**

The important test that refects the mechanical characteristics is compressive strength and has the efect on the other



<span id="page-7-2"></span>**Fig. 14** Results of fresh density of diferent SCCs mixtures





<span id="page-8-0"></span>**Fig. 15** Results of segregation resistance of diferent SCCs mixtures

<span id="page-8-1"></span>**Table 5** Results of hardened properties of SCCs

Mix	Dry Density kg/m <sup>3</sup>	$Com-$ pressive strength <b>MPa</b>	Flexural strength <b>MPa</b>	Absorp- tion $%$	UPV km/s
Mix0	2400	65	3.5	1.16	4.5
Mix1	2385	50	2.5	1.5	4.0
Mix2 2370		40	2.0	1.7	3.9
Mix3 2360		67	4.5	2.3	3.6
Mix4	2350	64	3.8	2.5	3.5
	Mix5 2340	54	3.5	2.6	3.6
Mix6	2330	51	3.0	2.7	3.5



<span id="page-8-2"></span>**Fig. 16** Results of compressive strength of diferent SCCs mixtures

durability properties. The results of compressive strength versus replacement levels of UFSFA and polypropylene fbers of SCC mixtures at 28 days curing age are given in Table [5](#page-8-1) and demonstrated in Fig. [16.](#page-8-2) However, the compressive strength decreased with increasing replacement level of UFSFA about 23 and 38% for 25 and 50%



**Fig. 17** The sample after the compressive test

<span id="page-8-3"></span>replacement level, respectively. The same efect of sun flower ash incorporation on the compressive strength of concrete was reported by researchers (Grubeša et al. [2019](#page-12-33); Shahbazpanahi and Faraj [2020](#page-12-34); Sisman and Gezer [2013b](#page-12-36)). Meanwhile, the compressive strength enhanced about 34 and 28% for the mix containing 25% of UFSFA by using 1% polypropylene fber with 7 and 9 cm length, respectively. On the other hand, the compressive strength of the mixture contained 50% of UFSFA developed about 35 and 27.5% by using of 1% of the same fber with 7 and 9 cm length, respectively. However, the surface of the fber was not smooth as shown in Fig. [17](#page-8-3) which improved the bond the cement paste and led to increase in the compressive strength. Correspondingly, it is concluded that the polypropylene fber with 7 cm length had more efect on compressive strength development. The same efect of using fber on the compressive strength is concluded by researchers (Zeynep et al. [2019](#page-13-2); Karmegam and Kalidass [2019](#page-12-35)). Moreover, from the results of the compressive strength, it is concluded that the using of this type of waste polypropylene as fber in SCC was suitable and improved it about 27.5–35% with diferent fber length and SFSFA content. On the other hand, the workability of SCC decreased with increasing polypropylene fber content due to its length and shape which efect on the fowability, flling and passing ability but increased the compressive strength due to the reasons explained previously in this section. Actually, the quality of fresh properties signifcantly efect on the hardened characteristics specially the mixtures not satisfy the standard criteria which effect negatively on the hardened properties.



## **4.2.2 Flexural Strength**

The outcome of this test versus with UFSFA replacement levels with and without polypropylene fber utilization for all SCC mixes is presented in Table [5](#page-8-1) and depicted in Fig. [18.](#page-9-0) From the outcomes of this investigation, it is concluded that the fexural strength reduced as UFSFA replacement level increased in SCC mixtures at 28 days curing age. The same efect of sun fower ash incorporation on the fexural strength of concrete was reported by investigators (Grubeša et al. [2019\)](#page-12-33). On the other hand, the fexural strength enhanced



<span id="page-9-0"></span>**Fig. 18** Results of fexural strength of diferent SCCs mixtures

about 80 and 52% for the mix containing 25% of UFSFA by using 1% polypropylene fber with 7 and 9 cm length, respectively. However, the fexural strength of the mixture contained 50% of UFSFA enhanced about 75 and 50% by using of 1% of the same fber with 7 and 9 cm length, respectively. Correspondingly, it is concluded that the polypropylene fber with 7 cm length had more efect on fexural strength development. The same efect of using fber on the fexural strength is concluded by researchers (Zeynep et al. [2019\)](#page-13-2). Moreover, from the results of the fexural strength, it is concluded that the using of this type of waste polypropylene as fber in SCC was suitable and improved it about 50–80% with diferent fber length and SFSFA content. However, the surface of the fber was not smooth and resists the applied load to produce the crack in the tension layer for the sample under the applied point load as shown in Fig. [19](#page-9-1) which improved the bond the cement paste and led to increase in the compressive strength.

In addition, the relationship between compressive–fexural strengths is shown in Fig.  $20. R^2$  $20. R^2$  values were used to calculate the correlation between the test results. It was discovered that there is a strong relationship between the compressive and flexural strengths of  $R^2$  = 0.85 for SCC mixtures.

On the other hand, the workability of SCC decreased with increasing polypropylene fber content due to its length and shape which efect on the fowability, flling and passing



**Fig. 19** The sample after the fexural test

<span id="page-9-1"></span>



<span id="page-10-0"></span>**Fig. 20** Correlation between compressive and fexural strengths of **SCCs** 

ability but increased the fexural strength due to the reasons explained previously in this section. Actually, the quality of fresh properties significantly effect on the hardened characteristics specially the mixtures not satisfy the standard criteria which efect negatively on the hardened properties.

#### **4.2.3 Dry Density**

The dry density of SCC mixtures lowered by increasing UFSFA utilization and the incorporation of polyethylene fber in the mixtures led to more decrease in dry density as shown in Fig. [21](#page-10-1). However, the main reason for these reduction in dry density is return to the lower specifc gravity of UFSFA and polyethylene fber compared with other ingredients as reported by researchers (Sor and Hamah [2018](#page-13-0); Öz et al. [2017](#page-12-4); Hilal et al. [2021a](#page-12-6); Sisman and Gezer [2013b](#page-12-36)). However, it was clearly shown that the using of fber had more effect on lowering the dry density of SCC mixtures and this impact increased with increasing the length of fber that related to its lower specifc gravity compared with other ingredients.



<span id="page-10-1"></span>**Fig. 21** Results of dry density of diferent SCCs mixtures

#### **4.2.4 Water Absorption**

Water absorption test is an important test for indicating the durability of concrete. In this study, the effect of UFSFA and polypropylene fber on the water absorption of SCC mixtures at 28 days curing age was studied as presented in Table [5](#page-8-1) and demonstrated in Fig. [22](#page-10-2). From the outcomes of this research, it is concluded that the water absorption increased as UFSFA replacement level increased in SCC mixtures at 28 days curing age. On the other hand, the water absorption increased about 53 and 66% for the mixes containing UFSFA by using 1% polypropylene fber with 7 and 9 cm length, respectively. Correspondingly, it is concluded that the polypropylene fber with 9 cm length had more influence on increasing of water absorption ratio. However, the water absorption of all SCC mixtures recorded in this investigation was lower than 3% and demonstrates that SCC mixtures had a good durability (Ovbeniyekede et al. [2018](#page-12-37)).

On the other hand, the relationship between the dry density–water absorption is shown in Fig.  $23. R^2$  $23. R^2$  values were used to calculate the correlation between the test results. It was discovered that there is a strong relationship between the dry density and water absorption of  $R^2$  = 0.95 for SCC mixtures.

#### **4.2.5 Ultrasonic Pulse Velocity (UPV)**

The outcomes of ultrasonic pulse velocity versus of different SCC mixes contained UFSFA at various quantities with/without polypropylene fiber are presented in Table [5](#page-8-1) and explained graphically in Fig. [24.](#page-11-4) However, the values of UPV declined as UFSFA incorporation increased in SCC mixtures about 11 and 13% for replacement level of 25 and 50% of UFSFA, respectively, as shown in Fig. [24.](#page-11-4) Meanwhile, the value of UPV decreased as dry density and compressive strength of the mixtures decreased as reported by other investigators (Hilal et al. [2021a](#page-12-6); Shahbazpanahi and



<span id="page-10-2"></span>**Fig. 22** Results of water absorption of diferent SCCs mixtures







<span id="page-11-3"></span>**Fig. 23** Correlation between dry density and water absorption of  $SCCs$ 



<span id="page-11-4"></span>**Fig. 24** Results of UPV of diferent SCCs mixtures

Faraj [2020\)](#page-12-34). Subsequently, all the SCC mixes were within a good quality concrete depending on the UPV outcomes for the mixes (Leslie and Cheesman [1949](#page-12-38)).

# **5 Conclusion**

In this study, the effect of UFSFA as cement replacement with/without polypropylene fber on the fresh and hardened characteristics of SCC mixtures were calculated. Depending on the results, the following conclusions can be obtained:

The workability, fowability and segregation resistance of SCC mixtures reduced by increasing replacement level of UFSFA specially with fber utilization but within the specifed limitations.

The slump flow diameter decreased by increasing replacement level of UFSFA specially with the longer fber utilization in SCC mixtures and all the mixes were classifed as SF1 class except the control mix which categorized as SF2.



The value of slump flow time increased with increasing UFSFA in the mixes specially with using polypropylene fiber

The results of V-shape flow time increased with increasing UFSFA in SCC mixtures and all the mixes categorized as VF2 class except the control mix.

The results of H2/H1 height ratio decreased by increasing UFSFA in the mixtures specially with polypropylene fber which using of the fber led to record the results lower than the standard limitations.

Segregation resistance of SCC mixtures enhanced with increasing UFSFA in the mixes and the incorporation of fber had a negative efect on segregation resistance but all the results were within the allowable limitations.

The increasing replacement level of UFSFA in the SCC mixtures led to decrease in the compressive strength but the addition of fber to the mixes developed the compressive strength specially the shorter length.

The increasing quantity of UFSFA in the SCC mixtures led to reduce the fexural strength but the addition of fber to the mixes developed the fexural strength specially the shorter length.

The dry density decreased by increasing UFSFA utilization in the SCC mixtures with/without fber and the value of reduction increased with addition of fbers specially the longer length.

The water absorption ratio increased by increasing UFSFA utilization in the SCC mixtures with/without fber and the value of increment increased with addition of fbers specially the longer length.

The results of UPV decreased by increasing UFSFA in the mixtures specially with polypropylene fber specially the longer length but considered a good quality concrete depending on the obtained values for all the mixtures.

The utilizing of waste polypropylene as a fiber is successfully performed in SCC.

Successfully designed the SCC mixtures that can be used for structural purposes and recycling high amount of waste material and cleaning the environmental.

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