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Bat algorithm for dam–reservoir operation

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Abstract

Optimizing reservoir operation rule is considered as a complex engineering problem which requires an efficient algorithm to solve. During the past decade, several optimization algorithms have been applied to solve complex engineering problems, which water resource decision-makers can employ to optimize reservoir operation. This study investigates one of the new optimization algorithms, namely, Bat Algorithm (BA). The BA is incorporated with different rule curves, including first-, second-, and third-order rule curves. Two case studies, Aydoughmoush dam and Karoun 4 dam in Iran, are considered to evaluate the performance of the algorithm. The main purpose of the Aydoughmoush dam is to supply water for irrigation. Hence, the objective function for the optimization model is to minimize irrigation deficit. On the other hand, Karoun 4 dam is designed for hydropower generation. Three different evaluation indices, namely, reliability, resilience, and vulnerability were considered to examine the performance of the algorithm. Results showed that the bat algorithm with third-order rule curve converged to the minimum objective function for both case studies and achieved the highest values of reliability index and resiliency index and the lowest value of the vulnerability index. Hence, the bat algorithm with third-order rule curve can be considered as an appropriate optimization model for reservoir operation.

Keywords Water supply · Hydropower generation · Optimization models · Reservoir operation · Rule curve · Bat algorithm

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Introduction

Background

Water resource managers intend to control available water resources to optimize benefit so that the available water resources are enough when critical conditions happen such as during drought periods (Bou-Zeid and El-Fadel [2002](#page-13-0); Galelli et al. [2014;](#page-13-1) Bozorg-Hadad et al. [2014a](#page-13-2), [b\)](#page-13-3). Considering the limitations of existing optimization algorithms, water resource decision-makers face a serious challenge when optimizing reservoir operation (Ramesh et al. [2013](#page-14-0); Fallah-Mehdipour [2011](#page-13-4); Bozorg-Hadad et al. [2008a,](#page-13-5) [b](#page-13-6)). Thus, decision-makers search for efficient optimization algorithms for achieving optimal reservoir operation with high reliability (Bozorg-Hadad et al. [2009;](#page-13-7) Noory et al. [2012\)](#page-14-1). Optimal operation of water resources reservoirs by optimization models is the appropriate way for better management, planning and operating available water systems (Bozorg-Hadad et al. [2014a](#page-13-2), [b](#page-13-3); Jothiprakash and Shanthi [2006](#page-13-1); Yang [2010](#page-14-2)). Currently, decision-makers apply mathematical models or software for providing proper decisions,

such as determination of water release or reservoir storage, to accomplish optimal operation (Zhao et al. [2014](#page-14-3); Bozorg-Hadad et al. [2014a](#page-13-2), [b](#page-13-3)). Considering reservoir constraints and the stochastic behavior of reservoir inflow pattern, the major challenge for these models is the ability to attain the optimal operation rule in adequate computation time. It is important for decision-makers to optimize water release pattern, minimize irrigation deficit, hydropower deficit and adequately meet downstream demands (Fallah-Mehdipour et al. [2012a,](#page-13-8) [b](#page-13-9); Taghian et al. [2013](#page-14-4); Shokri et al. [2013;](#page-14-5) Mousavi et al. [2005\)](#page-14-6). A variety of mathematical methods, such as dynamic programming, nonlinear programming, and linear programming, have been for reservoir operation. Although these methods lead to proper reservoir operation, several limitations have been experienced when applying them. For example, the use of a nonlinear and non-convex objective function makes optimization inefficient and sometimes nonconvergent. Dynamic programming requires a relatively long computational time to converge if the reservoir has several decision and state variables.

Recently, meta-heuristic algorithms have found numerous applications in engineering optimization and water resource management. Reddy ([2006\)](#page-14-7) found that the ant colony method for reservoir operation satisfactorily supplied water demands downstream of a dam. Using the honey bee mating method for reservoir optimization, Afshar et al. ([2007\)](#page-13-10) were able to meet downstream demands well. Applying this method for multipurpose reservoir operation, Bozorg-Hadad et al. ([2008a](#page-13-5), [b\)](#page-13-6) showed it converged to the global solution well. Chang and Cheng ([2009](#page-13-11)) found that the genetic algorithm supplied the different demands for a multiobejctive problem well. Fallah-Mehdipour et al. [\(2012b](#page-13-8)) showed that for reservoir operation the genetic programming method obtained the global solution in less computational time than did the genetic algorithm. For reservoir operation, Zhang et al. ([2012\)](#page-14-2) found that the guide particle swarm algorithm had a high-reliability index for water supply. Garousi et al. [\(2016\)](#page-13-9) were able to meet downstream demands well by the firefly algorithm for reservoir operation. Thus, the abovementioned studies show that meta-heuristic algorithms have satisfactorily performed in water resource management. One of the powerful algorithms in engineering optimization is the bat algorithm (Yang [2010](#page-14-2)) which acts based on position and sound of bats. The bats use echolocation ability for finding a prey. Yang ([2010\)](#page-14-2) found global optimum solutions well by applying the bat algorithm for benchmark functions. Reddy and Manoj ([2012\)](#page-14-8) applied the bat algorithm for finding the optimum capacitor placement to reduce loss of energy. For energy optimization, Niknam et al. ([2013](#page-14-9)) showed the bat algorithm led to better distribution and management of power energy than did the genetic algorithm.

The objective of this study is to investigate the Bat Algorithm (BA) as an optimization algorithm for dam and reservoir operation based on different orders of rule curves. The algorithm is evaluated for two different case studies in Iran, one designed for supplying irrigation demands while the other is designed for hydropower generation. In most studies, a linear decision rule for water release is considered and the released water is computed based on inflow and reservoir storage (Bozorg-Hadad et al. [2008a](#page-13-5), [b,](#page-13-6) [2009,](#page-13-7) [2014a,](#page-13-2) [b](#page-13-3)). The mathematical model of decision rule is considered, based on simple equations which do not lead to optimum reservoir operation; but different kinds of nonlinear decision rules can generate better simulation of a reservoir system. Thus, this paper employs nonlinear equations for reservoir operation.

The current paper uses nonlinear decision rules for reservoir operation. Different evaluation indices, such as reliability, vulnerability, and index reliability for comparing different performances of different rule curves, are examined. The first case study is related to minimizing irrigation deficits for three different order rule curves. The second case study is Karoun 4 dam which is designed to produce hydropower. Hence, the proposed optimization model is applied to increase hydropower generation by proper water release policy.

Problem statement

Dam and reservoir operation optimization is highly nonlinear and multimodal with noise, which makes the search for global optimal solution difficult. It is, therefore, important to find an algorithm that can lead to an optimal solution efficiently. The bat algorithm and bat intelligence algorithm are such algorithms.

1–4 reasons for modeling by bat algorithm

In recent years, various evolutionary algorithms such as genetic algorithm, particle swarm, and other evolutionary algorithms have been introduced to solve optimization problems. These algorithms have advantages and disadvantages. For instance, the genetic algorithm gets trapped by local optimization and finds local optima instead of global optimum (Ehteram et al. [2018\)](#page-13-12). The particle swarm algorithm has an early convergence in solving some problems that cause immature answers instead of the main answer (Karami et al. [2018](#page-14-10)). Some of the evolutionary algorithms are slow in convergence and achieving the optimum answer. Also, it is difficult to determine the initial random parameters in other algorithms (Ehteram et al. [2017a](#page-13-4)). Two important features in the evolutionary algorithms are exploration and exploitation capabilities. The exploration capability shows the ability of the algorithm to search freely without considering any achievements during the search process. In contrast, the exploitation is called the algorithm's attention to its achievements during the search process. Obviously, the more the exploration capability is in a search algorithm, the more random and unpredictable behavior can be seen for the algorithm (Ehteram et al. [2017b\)](#page-13-13). On the contrary, enhancing the exploitation in an algorithm causes this algorithm to be more accountable and more cautious.

The bat algorithm is an evolutionary algorithm, has a simpler process in determining the initial parameters of the method, has an advantage over other algorithms by setting the correct value of the parameters, and has the ability to balance the exploration and exploitation capabilities (Yang [2010](#page-14-2)). In addition, the algorithm has a faster convergence rate than genetic, particle swarm, and ant algorithms (Yang [2010\)](#page-14-2). Also, with the ability of local searching, it clearly identifies the range of main answers. Furthermore, the process of determining the parameters is simpler than other evolutionary algorithms.

This study considers optimization of the operation of Aydougmoush and Karun 4 dams reservoirs. These were selected for the importance of generating electricity and providing irrigation needs. Karun 4 dam is a key dam for power supply in Iran. The optimal way of releasing water and providing adequate storage behind the dam is an important issue. Therefore, the values of water release are defined as unknown values or candidates for solution by the algorithm.

Methodology

Bats are mammals which have a wing and produce intense pulses which return from the surroundings. Each bat provides 10–20 numbers of pulses per second and the frequency domain is between 25 and 100 kHz. When the bats are close to the prey, the number of pulses is about 200. Bats generate loud sounds so that the returning sounds and receiving of returning sounds direct bats to the best place for finding food (Merritt [2010](#page-14-11)). Bats continuously emit echolocation signals (Yovel et al. [2008\)](#page-14-12) and identify their surroundings and locate preys by analyzing the returning echoes in the auditory system. Bats use the echolocation to identify the difference between obstacle and preys. It is these abilities of bats that have inspired the development of bat algorithm (BA) and bat intelligence algorithm (BIA) for solving optimization problems such as dam and reservoir operation.

For simplification, the bat algorithm makes certain assumptions:

- 1. All bats use the echolocation ability for the identification of prey and obstacle. The pulses returning from the surroundings help bats identify obstacles from preys.
- 2. Bats fly randomly with velocity v_i , constant frequency f_{min} , wavelength and loudness A_0 at position x_i .
- 3. The loudness changes from a large positive value (A_{max}) to a small positive value (A_{min}) .
- 4. The frequency changes between f_{min} and f_{max} corresponding to the wavelength which changes between λ_{min} and λ_{max} . The frequency can be modified based on each problem.
- 5. The generated sounds for the bats have a specific pulse rate so that the maximum value for the pulsation rate is 1 and the minimum value for the pulsation rate is 0 and the pulsation rate can be updated in each iteration.

The velocity, position, and frequency can be updated based on the following equations:

$$
f_1 = f_{\min} + (f_{\max} - f_{\min})\beta,
$$
\n(1)

$$
v_1(t) = [y_1(t-1) - Y_*] \times f_1(t),
$$
\n(2)

$$
y_1(t) = y_1(t-1) + v_1(t),
$$
\n(3)

where β is a random vector between 0 and 1, $v_1(t)$ is velocity at time step *t*, $y_1(t-1)$ is position at time $t-1$ and Y_* is the best position among other positions for the bat algorithm.

The updating procedure for the bat algorithm is similar to the particle swarm optimization (PSO) because the pace and range of the movement of bats are controlled by f_1 as in PSO.

Next, the local search is defined based on a random walk as:

$$
y(t) = y(t-1) + \varepsilon A(t),
$$
\n(4)

where *A* is the loudness at time step *t* and ϵ is a random value between −1 and 1. The loudness (*A*) and pulse rate should be updated for each iteration. When bats find their prey, the loudness decreases, but the pulse rate increases for each bat. The pulsation rate is updated as:

$$
r_1^{t+1} = r_1^0 \left[1 - \exp\left(-\gamma t\right) \right] A_1^{t+1} = \alpha A_1^t,\tag{5}
$$

where α , γ are constant parameters, $A_1^t \rightarrow 0$, $r_1^t \rightarrow r_1^0$ when $0 < \alpha < 1$, $\gamma > 0$ and α parameter is similar to the cooling factor in the simulated annealing alorithm. Figure [1](#page-3-0) shows the bat algorithm.

Bat algorithm for reservoir operation

The bat algorithm has random parameters, such as frequency with unit Hz and sound height pulse with unit dB. For both dams, the water release is considered as the decision variable which is based on the amount of water released from the dam in such a way that the defined objectives at the downstream are satisfied. For example, irrigation needs for the Aydougmoush dam, and the hydropower needs for the Karun 4 dam will have to be met. The purpose of this study is to accurately determine the values of water release, which can be used to generate electricity and meet other needs. Since the bat algorithm and other evolutionary algorithms

Fig. 1 Bat algorithm

require an initial population, the water release values are considered as the initial population in the algorithm. The bat algorithm cycle must be completed once to determine the true and accurate amounts of water release. The purpose of this cycle is to determine the best values of water release after the initial guess of these values as the initial population. The water release values are entered into the algorithm as the initial location of the bat and the initial population.

The algorithm determines the best location of the bats or water release values, using Eq. [4](#page-2-0) for which it is required to obtain the speed from Eq. [3](#page-2-1). There is a random frequency parameter in Eq. [3,](#page-2-1) whose value must be determined from Eq. [2](#page-2-2). After these steps, the bat algorithm updates the location of bats based on the local search and exploration capability using Eq. 5. This requires the sound loudness parameter the value of which is initially the best value reported in Yang [\(2010\)](#page-14-2). Then, based on sensitivity analysis as shown in in Table [1,](#page-4-0) the exact values are obtained. In this procedure, considering the presence of several random parameters whose initial values are obtained based on the initial guess or the literature, the value of a variable parameter and the values of the rest of the parameters are kept constant. Then, the changes of the objective function are checked based on the changes in the parameter value, and whenever the objective function has the best value (for example, in a minimization problem when the objective function has its lowest value), the random parameter is considered to have its best value. To determine that the water release values have their best values and are optimal, the objective function is calculated, and in the case that it does not have the best value, the pulse rate increases according to Eq. 6 and the volume of the sound is reduced. The decision variable for the simplification in the computation process and thus, the initial water release values are inserted into the bat algorithm based on the initial population. Then, the reservoir storage using the continuity equation is computed, based on the initial values of released water. Then, the released water values and reservoir storage values are compared with the specified constraints and if the constraints are not satisfied, then penalties are added to the objective function. Then, the objective function is computed for each population. This procces is repeated for all operational periods. Then, the bat algorithm, shown in Fig. [1,](#page-3-0) is applied to all populations until the number of iteration reaches the maximum iteration. Then, the algorithm finishes and the optimal values of released water are obtained.

Case studies

General description of the case study

The reservoir of Karun 4 dam is located on the Karun River. This dam is the largest double-arched concrete dam and the fifth high dam and the source of hydropower in the world. The objectives of Karun 4 dam are the following:

- 1. Hydropower generation of 2000 MW annually.
- 2. Controlling surface waters of the area.
- 3. Controlling seasonal floods.

It is important to release water from the dam that the annual supply of hydropower and other desired objectives are met optimally. Therefore, the amount of water released in the dam is a decision variable and is unknown. The water release values are determined from Eqs. [10,](#page-5-0) [11](#page-5-1), and [12.](#page-5-2) The unknown coefficients in the equations must be determined by the bat algorithm according to the objective function and the specified constraints, and then the amount of water release is determined. Therefore, the main issue is the release method so that the deficiencies are minimized and the objective function is defined based on the difference between requirements and the release of water.

The Ayudoghmous dam is selected as the second case study. Aydoghmoush dam is located in East Azarbaijan Province, Iran. The purpose of the dam is to provide irrigation for 15,000 hectares of agricultural land. The maximum, average, and minimum requirements for the demand volume are equal to 39.57 \times 10⁶ $\frac{\text{m}^3}{\text{month}}$, 12.12 \times 10⁶ $\frac{\text{m}^3}{\text{month}}$, and 0, respectively. Therefore, water should be released in such a way that the reservoir storage is not less than the minimum value, and the downstream water requirements are met. The amount of water drainage from the dam for supplying the downstream needs is considered as a decision variable and thus, this value can be calculated based on optimization by the bat algorithm.

Aydoughmoush dam

The Aydoughmoush dam is located in the eastern part of Azarbaijan province, Iran. This dam supplies the irrigation demands of 15×10^3 ha. The inflow to this reservoir is 228×10^3 m, and the maximum and minimum storage are 145.7×10^3 and 8.9×10^3 m³, respectively. The period of 10 years (1991–2000) was selected for the case study. Figure [2a](#page-6-0) shows inflow and details of the study. There are extensive gardening and agriculture practices in this area that need water supply. Also, there are several drought periods that should be considered in reservoir operation.

The objective function is the minimization of irrigation deficit:

$$
\text{MinimizeOF} = \sum_{t=1}^{T} \left(\frac{D_t - R_t}{D_{\partial \text{max}}} \right)^2,\tag{8}
$$

where D_t is the demand water (MCM), R_t is the released water (MCM), and D_{max} is the maximum demand (MCM).

The continuity equation is defined as:

$$
S_{t+1} = S_t + I_t - R_t - SP_t - Loss_t,
$$
\n(9)

where S_t is the reservoir storage at time *t*, I_t is the inflow at time *t*; SP_t is the overflow at time *t*; and Loss is the reservoir losses at time *t*.

The released water is computed based on the linear and nonlinear equations as:

$$
R_t = a_i + b1_i S_t + c1_i I_t,
$$
\n(10)

$$
R_t = a_i + b1_i S_t^1 + b2S_t^2 + C1_i I_t^1 + C2_i I_t^2,
$$
\n(11)

$$
R_t = a_i + b1_i S_t^1 + b2_i S_t^2 + b3_i S_t^3 + c1_i I_t^1 + c2_i I_t^2 + c3_i I_t^3,
$$
\n(12)

where *a*, *b* and *c* for a released value are known as decision variables and are computed by optimization. Equations [10,](#page-5-0) [11](#page-5-1) and [12](#page-5-2) show the first-, second- and third-order rule curves.

The loss and overflow are computed as follows:

$$
Losst = At × EVt
$$

\n
$$
SPt =\begin{bmatrix} 0 \leftarrow \text{if}(S_t) \leq S_{\text{max}} \\ S_{\text{max}} - S_t \leftarrow \text{if}(S_t) \geq S_{\text{max}} \end{bmatrix},
$$
\n(13)

where A_t = the area of the reservoir lake, EV the evaporation depth (mm), and S_{max} the maximum storage.

Also, there are some constraints for reservoir optimization:

$$
0 \le R_t \le D_t
$$

\n
$$
S_{\min} \le S_t \le S_{\max}.
$$
\n(14)

If the constraints are not satisfied, the following equations are considered as penalty functions:

$$
P_{1,t} = \begin{bmatrix} 0 \leftarrow \text{if}(S_{t+1}) > S_{\text{min}} \\ \frac{(S_{\text{min}} - S_{t+1})^2}{S_{\text{min}}} < \text{otherwise} \end{bmatrix},\tag{15}
$$

$$
P_{2t} = \left[\frac{0 \leftarrow \text{if}(S_{t+1} < S_{\text{max}})}{(S_{t+1} - S_{\text{max}})^2} \leftarrow \text{otherwise} \right],\tag{16}
$$

Fig. 2 a Inflow to the reservoir, **b** location of case study, **c** Karoun basin, and **d** mathematical model for reservoir operation

Fig. 2 (continued)

where, P_1 , P_2 and P_3 are known as penalty functions. These penalty functions are added to the objective function.

Karoun 4 dam

The bat algorithm is evaluated for generating an optimal policy for reservoir operation for power generated by the downstream power plant based on three rule curves for water release. The Karoun 4 dam is 180 km southwest of Sharkord and the Karun River has the highest discharge of all Iranian rivers. The dam reservoir is operated for power generation for many industrial centers in this region. Hydropower station is near the dam and can produce 2107 MW power annually. Also, the maximum water release is $450 \times 10^3 \frac{\text{m}^3}{\text{month}}$. The maximum generated power for powerhouse is 1000×10^6 w. The Karin 4 dam is shown in Fig. [2](#page-6-0)c. The objective function for minimizing power deficit is expressed as:

$$
\text{Minimize}(\text{OF}) = \frac{1}{T} \left[\sum_{t=1}^{T} \left(1 - \frac{P_t}{\text{PPC}} \right)^2 \right],\tag{18}
$$

where OF the objective function; P_t the power generated; and PPC the total installed capacity for the powerhouse.

The power generated is computed as:

$$
P_t = \frac{\gamma' \times \eta \times \Delta H \times \text{Dis Re}_t}{10^6 \times n},\tag{19}
$$

where γ' the specific weight water; η the efficiency of the power plant; ΔH the difference of between water level in the downstream and upstream areas; DISRe_t the water released from the powerhouse; and n: a coefficient for evaluation of powerhouse performance.

The quantity ΔH is computed as:

$$
\Delta H_t = \left(\frac{H_t + H_{t+1}}{2}\right) - \text{TR},\tag{20}
$$

where H_t the water level at the beginning of the period; and H_{t+1} the water level at the end of the period.

Equations [9,](#page-5-3) [10,](#page-5-0) [11](#page-5-1), [12](#page-5-2) and [13](#page-5-4) are used again. Also, another constraint is considered as:

$$
0 \le P_t \le \text{PPC},\tag{21}
$$

where P_t the power generated.

Thus, if the above equation is not satisfied, the following penalty function is considered:

$$
P_{t} = \left[\frac{0 \leftarrow if \left(P_{t} < \text{PPC}\right)}{P_{t} - \text{PPC}} \leftarrow \text{otherwise}\right].\tag{22}
$$

Thus, if the equation was not satisfied, the penalty function (Eqs. [15,](#page-5-5) [16](#page-5-6) and [22\)](#page-8-0) would be used and added to the objective function.

Concepts in reservoir equations

Water release Water release is considered as the decision variable, i.e., a variable whose value is obtained from the algorithm and shows the amount of water drainage from the dam to meet the downstream needs (Al-aqeeli et al. [2015\)](#page-13-14).

Loss Each reservoir faces losses. For example, evapotranspiration from the reservoir surface can be one of the reasons for the loss in water level and reservoir storage. The loss is obtained from the product of the surface of the reservoir and the depth of evaporation (["Aydoughmoush dam](#page-5-7)", Eq. [13\)](#page-5-4) (Al-aqeeli et al. [2016\)](#page-13-0).

Overflow In case of excess water exceeding the reservoir storage, the additional amount flows out through the dam spillway, which is called overflow. After applying the algorithm each time, the amount of water is compared to the current storage.

Reservoir operation by bat algorithm

The optimal operation of reservoirs based on the bat algorithm involves the following steps:

- 4. The initial parameters of the method, such as the loudness of sound, pulse, and frequency, are entered into the algorithm as the initial guess and then sensitivity analysis is performed.
- 5. With the values of water release as the initial population and the decision variable, their best values are obtained by the algorithm, which are then introduced into the algorithm as the initial location of the bats.
- 6. There is a constraint for each case. For example, in light of constraints 13 and 14 for the Aydoghmoush dam, the values of water release should be compared with the maximum and the minimum water releases. Also, according to the continuity equation, there is an relationship between reservoir storage at the new time and a prior period, based on which the storage amount is calculated. If the water release values and storage in the permitted range are not satisfied, penalties such as Eqs. [15](#page-5-5)[–17](#page-8-1) and Eq. [22](#page-8-0) are considered.
- 7. There is an objective function for each reservoir. For example, an objective in relation to a reservoir is to minimize irrigation deficit and in another reservoir, it is to minimize hydropower deficit. If there are penalty functions based on the previous stage, they are added to the objective function.
- 8. The location of the bats or the initial water release values should be updated for the algorithm's next repetition, based on Eqs. [1–](#page-2-3)[3](#page-2-1).
- 9. The ability the algorithm to obtain the global solution is evaluated using Eq. 5, and if the algorithm is trapped in a local optimum, a new answer is generated.
- 10. The repeat counts are compared with the maximum repeat. In the case of being equal, the algorithm is complete, otherwise we return to the beginning of the algorithm.

Performance measures

Different indices are used for evaluation of the bat algorithm for different rule curves:

First case study

1. *Volumetric reliability* This index considers the ratio of released water to total demand in the operation period (Ehteram et al. [2017a\)](#page-13-4).

$$
\alpha_{\rm V} = \frac{\sum_{i=1}^{N} \sum_{t=1}^{T} R}{\sum_{i=1}^{N} \sum_{i=1}^{T} D},\tag{23}
$$

where *R* the released water; *D* the demand; and α_V the volumetric reliability

2. *Vulnerability index* This index shows the maximum intensity of failure during the operation period (Ehteram et al. [2017a\)](#page-13-4).

$$
\lambda = \underset{i=1}{\overset{N}{\text{Max}}} \left(\underset{i=1}{\overset{T}{\text{Max}}} \left(\frac{D_t - R_t}{D_t} \right) \right), \tag{24}
$$

where λ is the vulnerability index.

3. *Resiliency index* This index shows the way of existence of a system from a failure during the operation period.

$$
\gamma_i = \frac{f_{\rm si}}{F_i},\tag{25}
$$

where γ the resiliency index; f_{si} the number of failures series; and F_i the number of failure series. *Reliability index* This index shows the percentage of the generated hydropower to the capacity of power plant.

$$
R = \frac{\sum_{t=1}^{T} \left[\left(P_t | P_t < \text{PPC} \right) \vee \left(\text{PPC} | P_t \ge \text{PPC} \right) \right]}{T \cdot \text{PPC}},\tag{26}
$$

where $\sum_{t=1}^{T} \left[\left(P_t | P_t \lt \text{PPC} \right) \vee \left(\text{PPC} | P_t \ge \text{PPC} \right) \right]$ shows the generated hydropower and *T* ⋅ PPC illustrates the maximum generation power.

 Resiliency index This index indicates the ability of system for escaping from failure (Ehteram et al. [2017b](#page-13-13)):

Re =
$$
\frac{N_{t=1}^{T-1} (P_t < \text{PPC}|P_{t+1} \ge \text{PPC})}{N_{t=1}^T (P_t < \text{PPC})},
$$
 (27)

where $N_{t=1}^{T-1} (P_t < \text{PPC}|P_{t+1} \ge \text{PPC})$ shows the number of periods in which the system can exit from a failure and $N_{t=1}^T (P_t < PPC)$ shows the total deficit power.

 Vulnerability index This index shows the value of deficit during operation periods (Ehteram et al. [2017a](#page-13-4)).

$$
V = \frac{\sum_{t=1}^{T} \left[\left(\text{PPC} - P_t | P_t < \text{PPC} \right) \cup \left(0 | P_t \ge \text{PPC} \right) \right]}{T},\tag{28}
$$

where term $(PPC - P_t|P_t < PPC) \cup (0|P_t \ge PPC)$ identifies the total deficit in a particular period.

Evaluation of bat algorithm in reservoir operation

Different indexes, introduced by Hashimoto et al. ([1982](#page-14-13)), are used to evaluate water releases by the bat algorithm and its ability to properly exploit the reservoir. The system efficiency in supplying downstream needs based on the amount of abandoned water relative to need, which is known as the Reliability Index, is calculated. Such an index for Karun 4 dam represents the percentage of electricity generated to the maximum produced electricity during the period of operation. Higher percentage of the index represents a better performance of the algorithm. Another indicator is the Vulnerability Index which indicates the maximum failure ratio during the operation period. For the Karun 4 reservoir, this index shows the average failure rate. Another indicator is the Resiliency Index. This index indicates that if the system is faced with deficit or failure in some periods, how fast does the system switch from failure or the deficit period. For instance, if a 12-month operation period fails in four periods, the failure sequence affects the system performance. In other words, four consecutive failures have a different effect than the case that in each period, failure is followed by a non-failure period.

Results and discussion

The bat algorithm with three different rule curves was been applied to both case studies and the three performance indicators were calculated for each case.

Aydoughmoush dam

Table [1](#page-4-0) shows the sensibility analysis for the bat algorithm for the first-, second- and third-order rule curves. It can be noted that the best population size for first-, second- and third-order rule curves was 60, 60 and 40, respectively. The maximum frequency (f_{max}) for first-, second- and third-order rule curves was 5, 5 and 3, respectively. The minimum loudness (A_0) for first-, second- and third–order rule curves was 0.20, 0.10 and 0.10, respectively.

Table [2](#page-10-0) shows results based on ten different random executions for the algorithm for Aydoughmoush Dam. Results were compared to the global solution with Lingo software (version. 11) which uses nonlinear programming. The minimal average solution was attained with the third-order rule curve as compared with the first- and second-order rule curves. The percentage of reduction of the average solution was equal to 6.3 and 16% as compared with the firstand second-order rule curves, respectively. The variation coefficient for the third rule curve was 50 and 70% of the resultant values attained using the first-order rule curve and the second-order rule curve, respectively. Also, the global solution based on Lingo software and nonlinear programming for the first-, second- and third-order rule curves was 1.77, 1.92, and 2.14 and as a result, the average solutions for first-, second- and third-order rule curves were 99, 98 and 98% of the global solution. Thus, the bat algorithm had the proper solution for different order rule curves, but the third-order rule curve had the least objective function and variation coefficient.

Table [3](#page-10-1) shows the three performance indicators for the first-, second- and third-order curves. The reliability index for the third-order rule curve was 98%, and it was 4 and 2% more than for the first- and second-order rule curves.

Table 2 Results for Aydoughmoush dam

Number of runs	First-order rule curve	Second-order rule curve	Third-order rule curve
1	2.12	1.90	1.78
2	2.15	1.92	1.79
3	2.12	1.90	1.78
4	2.12	1.90	1.78
5	2.12	1.90	1.78
6	2.12	1.90	1.78
7	2.12	1.90	1.78
8	2.12	1.90	1.78
9	2.12	1.90	1.78
10	2.12	1.90	1.78
Average solution	2.12	1.90	1.78
Variation coefficient	0.0044	0.0033	0.0021
Global solution	2.14	1.92	1.77

Table 3 Different indices for Aydoughmoush dam

Index	Reliability index $(\%)$	Vulnerability index $(\%)$	Resiliency index $(\%)$
First-order rule curve	94	12	38
Second-order rule curve	96	10	41
Third-order rule curve	98		45

In other words, the bat algorithm with the third-order rule curve matched the demand pattern for irrigation water use efficiently. In addition, it can be shown that the resiliency index for the third-order rule curve was 45% which outperformed the other order rule curves. On the other hand, the third-order rule curve procedure efficiently improved the first- and second-order rule curve procedures by 7–4%, respectively. Also, the third-order rule curve had the lowest value of vulnerability among other two methods. Such performance for the resilience index indicates that the proposed optimization model could overcome the deficit effectively.

Figure [3](#page-11-0) shows the convergence of three rule curves, and third rule curve had converged to the least objective function for the same number of iterations (Fig. [3b](#page-11-0)). Also, the maximum, minimum and average solutions converged to each other well. The released water for different months of operation, in Fig. [3c](#page-11-0), shows that the third-order rule curve had supplied demands better than did two other rule curves. The water release from the reservoir and the demand pattern for irrigation strongly matched which means that the optimization algorithm minimized the deficit. Finally, Fig. [3](#page-11-0)d shows that the storage pattern was more stable within the range of maximum and minimum storages of the reservoir.

Table [4](#page-11-1) shows the optimal monthly coefficient for third rule curve equation computed by the bat algorithm because the third-order rule curve had the best performance. Each coefficient was a decision variable for computation of released water based on the third-order rule curve. When the coefficient value was relatively small, the effect of that term on the released water was negligible. The value of b3 for January was small. Thus, the term b3S3 was negligible. The values of b1, b2 were also negligible. The value of released water for January was less dependent on the reservoir storage. These coefficients showed the importance of different terms which affected released water volume.

Karoun 4 dam

Table [5](#page-12-0) shows sensibility analysis and the best values of random parameters by the bat algorithm for Karoun 4 reservoir. Minimizing the hydropower deficit can be achieved when the objective function value for each parameter reaches the minimal value. This value was considered as the optimal random parameter. The best size of population for the

Fig. 3 Simulation results for **a** convergence way, **b** maximum, minimum and average solution for third-order rule curve, **c** released water, and **d** storage value

first-, second- and third-order rule curves was 60, 60 and 40, respectively. The best value of maximum frequency for the first-, second- and third-order rule curves was 3, 3 and 5, respectively. The minimum frequency for the first-, second- and third-order rule curves was 1, 1 and 2, respectively.

Table [6](#page-12-1) shows ten random results for three kinds of rule curve. Results show that the average solution for the three rule curves was close to a global solution. The Lingo software based on nonlinear programming method computed the global solution. The average solution of the third-order rule curve was less than of the other two rule curves, and the coefficient variation of the third-order rule curve was 5.33 and 3.66 smaller than first-order and second-order rule curves, respectively.

Figure [4](#page-12-2) shows the convergence versus the iteration number for the three rule curves. The bat algorithm with the third-order rule curve converged to the minimum value faster than the other rule curves. .

Figure [5](#page-12-3) shows that the generated power for the released water for the third-order rule curve was more than for the first-order rule curve and second-order rule curve. Table [7](#page-12-4) shows different indices for the three rule curves. The reliability index and the resiliency index had the highest percent and the vulnerability index had the smallest value for third-order rule curve. Table [8](#page-13-15) shows the coefficients for the third-order rule curve. For example, the values of c1 and c2 and c3 for April were small and thus, the inflow volume was less important in the computation of released water and reservoir storage played an important role for this month. One of the reasons is related to less inflow for this month. The values of b1 and b2 and b3 for May were negligible, and as a result, the released water volume was less dependent on the reservoir storage for this month. Also, the value of each coefficient for each rule curve was a decision variable and was computed by the optimization algorithm.

Months	a	b_1	b_2	b_3	c_{1}	c_2	c_3
Jan	0.0000	0.0100	0.002	0.0001	0.3000	0.0010	0.0051
Feb	1.4500	0.8400	0.004	0.0040	0.2871	0.0023	0.0032
Mar	0.9000	0.7800	0.003	0.0005	0.2856	0.0034	0.0031
Apr	0.0000	0.8600	0.001	0.0006	0.2786	0.0056	0.0024
May	0.0000	0.8900	0.002	0.0005	0.2679	0.0053	0.0031
Jun	0.0000	0.9800	0.001	0.0001	0.2772	0.0067	0.0032
Jul	0.0000	0.7600	0.003	0.0040	0.2443	0.0078	0.0035
Aug	0.0000	0.7500	0.004	0.0005	0.2543	0.0065	0.0043
Sep	0.0000	0.7400	0.002	0.0001	0.2672	0.0056	0.0054
Oct	0.0000	0.7300	0.003	0.0040	0.2414	0.0045	0.0056
Nov	0.0000	0.7200	0.001	0.0005	0.2341	0.0034	0.0057

Table 4 Optimal monthly parameters for third rule curve (Aydoughmoush dam)

Table 5 Sensibility analysis for Karoun 4 reservoir	Size population	Objective function	f_{max}	Objective function	f_{\min}	Objective function	A_{max}	Objective function	A_{\min}	Objec- tive function	
	First-order rule curve										
	20	1.55	1	1.45	$\mathbf{0}$	1.46	$\overline{0}$	1.76	$\mathbf{0}$	1.55	
	40	1.45	3	1.39	1	1.39	0.3	1.39	0.1	1.39	
	60	1.39	5	1.41	2	1.43	0.6	1.45	0.20	1.23	
	80	1.46	7	1.44	3	1.48	0.9	1.47	0.30	1.2	
	Second-order rule curve										
	20	1.57	1	1.47	$\mathbf{0}$	1.46	0	1.49	$\overline{0}$	1.45	
	40	1.45	3	1.34	1	1.34	0.3	1.34	0.10	1.34	
	60	1.34	5	1.36	2	1.38	0.6	1.41	0.20	1.36	
	80	1.39	τ	1.45	3	1.45	0.9	1.45	0.30	1.39	
	Third-order rule curve										
	20	1.34	$\mathbf{1}$	1.42	$\mathbf{0}$	1.45	$\overline{0}$	1.56	$\mathbf{0}$	1.34	
	40	1.19	3	1.38	1	1.38	0.3	1.43	0.10	1.19	
	60	1.32	5	1.19	\overline{c}	1.19	0.6	1.19	0.20	1.32	
	80	1.33	τ	1.24	3	1.24	0.9	1.24	0.30	1.33	

Table 6 10 Random results for Karoun 4 reservoir

Fig. 4 Convergence for different methods for Karoun 4 reservoir

Fig. 5 Simulation results for **a** released water, and **b** generated power

Table 7 Different indices for Karoun 4 reservoir

Index	Reliability index $(\%)$	Vulnerability index $(\%)$	Resiliency index $(\%)$	
First-order rule curve	91	14	45	
Second-order rule curve	93	12	47	
Third-order rule curve	95	10	55	

Table 8 Optimal value for determination of coefficients of rule curve

Conclusions

This study investigates the potential of utilizing the Bat algorithm with the rule curve method to identify an optimal reservoir operation policy. The BA algorithm was evaluated for two case studies, namely, Aydoghmush dam and Karoun 4 dam. The algorithms successfully achieved the targeted objective function which was minimizing the deficit in irrigation and hydropower generation for Aydoghmush dam and Karoun 4 dam, respectively. For both case studies, the bat algorithm with the third-order rule curve achieved a global solution close to the one attained by Lingo software. To evaluate the performance of the algorithm, reliability index, resiliency index, and vulnerability index were determined for both case studies with simulation for 10 years of operation. Results showed that the bat algorithm with third-order rule curve achieved the highest value for the reliability and resilience indexes and the lowest value for the vulnerability index for both case studies. Therefore, it can be concluded that the bat algorithm with the third-order rule curve performed better than the other order's rule curves in deriving the optimal operating policy for reservoir operation.

References

- Afshar A, Bozorg Haddad O, Mariño MA, Adams BJ (2007) Honeybee mating optimization (HBMO) algorithm for optimal reservoir operation. J Franklin Inst 344(5):452–462
- Al-Aqeeli YH, AlMohseen KA, Lee TS, Aziz SA (2015) Modeling monthly operation policy for the Mosul Dam, northern Iraq. Int J Hydrol Sci Technol 5(2):179–193
- Al-Aqeeli YH, Lee TS, Aziz SA (2016) Enhanced genetic algorithm optimization model for a single reservoir operation based on hydropower generation: case study of Mosul reservoir, northern Iraq. SpringerPlus 5(1):797
- Bou-Zeid E, El-Fadel M (2002) Climate change and water resour ces in Lebanon and the Middle East. J Water Resour Plan Manag. [https://doi.org/10.1061/\(ASCE\)0733-9496\(2002\)128:5\(343\)](https://doi.org/10.1061/(ASCE)0733-9496(2002)128:5(343))
- Bozorg Haddad O, Afshar A, Mari˜no MA (2008a) Design operation of multi-hydropower reservoirs: HBMO approach. Water Resour Manag 22(12):1709–1722
- Bozorg Haddad O, Adams BJ, Mari ˜ no MA (2008b) "Optimum rehabilitation strategy of water distribution systems using the HBMO algorithm. J Water Supply Res Technol AQUA 57(5):327–350
- Bozorg Haddad O, Moradi-Jalal M, Mirmomeni M, Kholghi MKH, Mariño MA (2009) Optimal cultivation rules in multi-crop irrigation areas. Irrig Drain 58(1):38–49
- Bozorg-Haddad OB, Moravej M, Loáiciga HA (2014a) Application of the water cycle algorithm to the optimal operation of reservoir systems. J Irrig Drain Eng 141(5):04014064
- Bozorg-Haddad O, Karimirad I, Seifollahi-Aghmiuni S, Loáiciga HA (2014b) Development and application of the bat algorithm for optimizing the operation of reservoir systems. J Water Resour Plan Manag 141:04014097
- Chang LC, Cheng FJ (2009) Multi-objective evolutionary algorithm for operating parallel reservoir system. J Hydrol 377(1):12–20
- Ehteram M, Karami H, Mousavi SF, Farzin S, Kisi O (2017a) Optimization of energy management and conversion in the multireservoir systems based on evolutionary algorithms. J Clean Prod 168:1132–1142
- Ehteram M, Allawi MF, Karami H, Mousavi SF, Emami M, Ahmed ES, Farzin S (2017b) Optimization of chain-reservoirs' operation with a new approach in artificial intelligence. Water Resour Manag 31(7):2085–2104
- Ehteram M, Karami H, Farzin S (2018) Reducing irrigation deficiencies based optimizing model for multi-reservoir systems utilizing spider monkey algorithm. Water Res Manag 32(7):2315–2334
- Fallah-Mehdipour E, Haddad OB, Mariño MA (2011) MOPSO algorithm and its application in multipurpose multireservoir operations. J Hydroinf 13(4):794–811
- Fallah-Mehdipour E, Haddad OB, Mariño MA (2012a) Real-time operation of reservoir system by genetic programming. Water Resour Manag 26(14):4091–4103
- Fallah-Mehdipour E, Haddad B, Rezapour Tabari OMM, Mariño MA (2012b) Extraction of decision alternatives in construction management projects: application and adaptation of NSGA-II and MOPSO. Exp Syst Appl 39(3):2794–2803
- Galelli S, Goedbloed A, Schwanenberg D, van Overloop PJ (2014) Optimal real-time operation of multipurpose urban reservoirs:

case study in Singapore. J Water Resour Plan Manag. [https://](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000342) [doi.org/10.1061/\(ASCE\)WR.1943-5452.0000342](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000342)

- Garousi I, Bozorg-Hadad O, Hugo A (2016) Application of the firefly algorithm to optimal operation of reservoirs. J Irrig Drain Eng. [https://doi.org/10.1061/\(ASCE\)IR.1943-4774.0000932](https://doi.org/10.1061/(ASCE)IR.1943-4774.0000932)
- Hashimoto T, Stedinger JR, Loucks DP (1982) Reliability, resilience and vulnerability criteria for water resource system performance evaluation. Water Resour Res 18(1):14–20
- Jothiprakash V, Shanthi G (2006) Single reservoir operating policies using genetic algorithm. Water Resour Manag 20(6):917–929
- Karami H, Ehteram M, Mousavi SF, Farzin S, Kisi O, El-Shafie A (2018) Optimization of energy management and conversion in the water systems based on evolutionary algorithms. Neural Comput Appl.<https://doi.org/10.1007/s00521-018-3412-6>
- Merritt JF (2010) The biology of small mammals. John Hopkins University Press, Baltimore
- Mousavi SJ, Ponnambalam K, Karray F (2005) Reservoir operation using a dynamic programming fuzzy rule-based approach. Water Resour Manag 19(5):655–672
- Niknam T, Sharifinia S, Azizipanah-Abarghooee R (2013) A new enhanced bat-inspired algorithm for finding linear supply function equilibrium of GENCOs in the competitive electricity market. Energy Convers Manag 76:1015–1028
- Noory H, Liaghat AM, Parsinejad M, Haddad B, O (2012) Optimizing irrigation water allocation and multicrop planning using discrete PSO algorithm. J Irrig Drain Eng. [https://doi.org/10.1061/\(ASCE\)](https://doi.org/10.1061/(ASCE)IR.1943-4774.0000426) [IR.1943-4774.0000426](https://doi.org/10.1061/(ASCE)IR.1943-4774.0000426)
- Ramesh B, Mohan VCJ, Ressy VCV (2013) Application of bat algorithm for combined economic load and emission dispatch. J Electr Eng Telecommun 2(1):1–9
- Reddy MJ (2006) Ant colony optimization for multipurpose reservoir operation. Water Resour Manag 20(6):879–898
- Reddy VU, Manoj A (2012) Optimal capacitor placement for loss reduction in distribution systems using bat algorithm. IOSR J Eng 02(10):23–27
- Shokri A, Haddad BO, Mariño MA (2013) Algorithm for increasing the speed of evolutionary optimization and its accuracy in multiobjective problems. Water Resour Manag 27(7):2231–2249
- Taghian M, Rosbjerg D, Haghighi A, Madsen H (2013) Optimization of conventional rule curves coupled with hedging rules for reservoir operation. J Water Resour Plan Manag. [https://doi.org/10.1061/](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000355) [\(ASCE\)WR.1943-5452.0000355](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000355)
- Yang XS (2010) A new metaheuristic bat-inspired algorithm. In: Gonzalez JR et al (eds) Nature inspired cooperative strategies for optimization (NISCO 2010). Studies in Computational Intelligence, vol 284. Springer, Berlin, pp 65–74
- Yovel Y, Franz MO, Stilz P, Schnitzler H-U (2008) Plant classification from bat-like echolocation signals. PLoS Comput Biol 4:1–13
- Zhang R, Zhou J, Zhang H, Liao X, Wang X (2012) Optimal operation of large-scale cascaded hydropower systems in the upper reaches of the Yangtze River, China. J Water Resour Plan Manag. [https://](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000337) [doi.org/10.1061/\(ASCE\)WR.1943-5452.0000337](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000337)
- Zhao T, Zhao J, Yang D (2014) Improved dynamic programming for hydropower reservoir operation. J Water Resour Plan Manag. [https](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000343) [://doi.org/10.1061/\(ASCE\)WR.1943-5452.0000343](https://doi.org/10.1061/(ASCE)WR.1943-5452.0000343)