#### **ORIGINAL ARTICLE**



# **Scenario‑based numerical simulation to predict future water quality for developing robust water management plan: a case study from the Hau River, Vietnam**

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#### **Abstract**

Rapid population growth, urbanization, industrialization, and climate change are the key drivers causing serious water pollution around the globe. Considering the impacts of these key drivers, this study employed the Water Evaluation and Planning (WEAP) simulation tool to simulate the future water quality in a nearly 60-km stretch of the Hau River, Vietnam. The business-as-usual (BAU) scenario; scenarios with measures (WM), i.e., wastewater treatment plants (WWTPs) for treating 75% (WM75) and 100% (WM100) of total future wastewater generated; and the optimistic scenario (WM\_Opt., i.e., WM100+additional treatment plants for river water (RWTPs)) to achieve the desired water quality, were applied to simulate the future Hau River water quality for the year 2030. Result suggests that the average values of biochemical oxygen demand (BOD), total coliform (TC), nitrate  $(NO<sub>3</sub><sup>-</sup>)$ , and phosphate  $(PO<sub>4</sub><sup>3-</sup>)$  in the wet season of 2030 under BAU scenario will be increased by 16.01%, 40.85%, 30.49%, and 20.22%, respectively, in comparison to those of the current year, i.e., 2018. In the dry season, these rates will be increased by 27.80%, 65.94%, 31.05%, and 20.64%, respectively. Under the scenario with measures (WM75 and WM100), although the Hau River water quality was improved but did not reach the desired limits, especially for BOD and  $PO_4^{3-}$ levels in the downstream region. However, under the WM\_Opt. scenario, the average simulated values of both BOD and  $PO<sub>4</sub><sup>3-</sup>$  will be significantly declined by 76.53% and 63.96%, respectively as compared to the current situation and help to achieve river water quality under class A. This study is providing policy-relevant scientific information, vital for sustainable water resource management.

**Keywords** Water pollution · Biochemical oxygen Demand · Total coliform · Nitrate · Phosphate · Water quality modeling · River water and wastewater treatment · The Hau River · Vietnam

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### **1 Introduction**

Water resources are intrinsically linked to the sustainable and inclusive development of human societies (Heinemann et al. [2002](#page-34-0); FAO AQUASTAT [2016](#page-34-1)). This natural resource even signifes its importance in the development of developing countries/regions like South East Asia (SEA) (Moglia et al. [2012\)](#page-35-0). Accessibility to safe water for humans is a major challenge, which is well recognized under the United Nations Sustainable Development Goals (SDGs) (Dell'Angelo et al. [2018](#page-34-2); Avtar et al. [2020](#page-33-0)). However, many SEA countries, including Vietnam, have currently faced several challenges posed by the increasing demand for clean water supplies. The situation is exaggerated by periodic droughts and contamination of both surface and groundwater (Wakida and Lerner [2005](#page-36-0); UNDP [2006](#page-36-1); Moglia et al. [2008\)](#page-35-1). Rapidly developing urban centers are acting as the focal point for this clean water scarcity (Grohmann [2009;](#page-34-3) Henriques [2011\)](#page-34-4). This has implications for better policies, planning, and management processes required to secure adequate safe water supplies for the future (UNDP [2006](#page-36-1)).

Water resources management (WRM), especially for the river bodies is a complex task in growing urban areas with a great number of difficulties and challenges (Phung et al.  $2015$ ; Hoekstra et al.  $2018$ ; Trung et al.  $2019a$ ). The key difficulties and challenges are rapid population growth, urbanization and industrialization; unsustainable agriculture; inadequate wastewater management infrastructure; the lack of awareness and common practices; and poor governance. Besides, water security is further skewed with extreme weather conditions, viz., foods and droughts, induced due to climate change (Konings [2012;](#page-34-6) Mukate et al. [2018;](#page-35-2) Trung et al. [2019a\)](#page-36-3). Therefore, it is a challenging task for developing countries to maintain the sustainable water supply and achieve the SDGs due to their unplanned and indiscriminating WRM policies with the absence of larger socio-economic factors (Downing [2012\)](#page-34-7). Therefore, inclusive and transdisciplinary research is a timely and critical need to achieve urban water security in the future (Kumar [2019](#page-35-3)).

In Vietnam, Dong Thap, Vinh Long, and Can Tho are not only three of the most crucial developing cities but also the regional urban centers in the Vietnamese Mekong Delta (VMD). Moreover, Can Tho is also considered one of the fve nation's largest economic centers (Konings [2012](#page-34-6); Moglia et al. [2012\)](#page-35-0). With their enormous river and canal networks (more than 3,405 km), water is the center of everyday life and underpins these cities' economies, including agriculture, aquaculture, industry, transportation, services, and tourism (Konings [2012;](#page-34-6) Nguyen et al. [2016\)](#page-35-4). Currently, the Hau River supplies the main source of water for these three cities. Being the main tributary of the Mekong River, the Hau River fows across the entire area of Can Tho City and the large parts of Dong Thap and Vinh Long Cities' areas with a total length of nearly 60 km. However, the fast urbanization and industrialization, population growth, and climate change (mainly change in rainfall) have placed enormous pressure on these river water resources, especially the water quality. The situation gets worse with the low levels of residents' awareness, lack of sufficient regulations, and inadequate water infrastructure (Quan et al. [2013;](#page-36-4) Trung et al. [2019a](#page-36-3)). Noticeably, although wastewater is rapidly increasing due to the population growth, rapid urbanization, industrialization, and changes in water consumption behavior, the investment for the water management infrastructure does not keep pace with the developing demand (Long and Cheng [2018](#page-35-5); Trung et al. [2019a\)](#page-36-3). Moreover, these cities' wastewater management is limited to the collection of wastewater from both point and non-point pollution sources through open and underground sewer lines, and disposal of untreated wastewater into the rivers (Trung et al. [2019a](#page-36-3)). Thus, a huge amount of untreated wastewater from

both domestic and industrial activities of these cities is directly discharged into the Hau River and its diversion network (CTCPC [2019b](#page-33-1)). As a result, the Hau River is getting contaminated with physical, chemical, and biological pollutants, exceeding the desired surface water quality of class A of the National Technical Regulation on Surface Water Quality (NTRSWQ) (CTCPC [2019b;](#page-33-1) DOC [2019](#page-34-8)).

Since the systems of the natural environment and anthropogenic sources are multivariate and complex, river water quality management requires a fundamental understanding of spatial and temporal variations in the water characteristics, including hydro-morphological, chemical, and biological parameters (Phung et al. [2015\)](#page-36-2). For this purpose, diligent monitoring of water quality is a necessity for maintaining a clear water environment and reducing the fnancial burden to local government in managing polluted water at the later stage (Simeonov et al. [2004;](#page-36-5) Shrestha and Kazama [2007](#page-36-6)). Moreover, to design a robust management plan, there is a requirement for both current water quality conditions and also possible future scenarios. The formulation of water quality management strategies requires an interdisciplinary analysis of various potential causes of water quality degradation as well as corresponding solutions (Kannel et al. [2007;](#page-34-9) Mishra et al. [2017\)](#page-35-6). With the above-mentioned facts, computer-based mathematical models have been developed since the 1980s to assess and predict water quality. In WRM, the favorable approaches vary from simplifed conceptual, empirical to data-based ones. Most models can provide detailed insight into water quality conditions in a river system such as SWAT, QUAL2K, and MIKE. However, there has mostly been an absence of policy-setting issues in detail (Mishra et al. [2017;](#page-35-6) Kumar [2019\)](#page-35-3). Specifcally, the Integrated Water Resource Management (IWRM) model targets diferent components of water resource governance (socio-economic status, hydrometeorological factors, agriculture, industries, wastewater, etc.) and thereby integrates them for the policy- and decision-making processes (Blanco-Gutiérrez et al. [2013;](#page-33-2) Frija et al. [2015\)](#page-34-10). Several IWRM numerical models like RIBASIM (River Basin Simulation Model), SWMM (Storm Water Management Model), VENSIM (The System Dynamic Model), WEAP (Water Evaluation and Planning), and WBalMo (Water Balance Model) have been developed and widely used to address water security issues (Sieber and Purkey [2015;](#page-36-7) Kumar [2018;](#page-35-7) Trung et al. [2019a\)](#page-36-3). The application of an optimal water quality model depends on data availability, calculation time, and intended output variables.

Particularly, the WEAP model is not data-intensive and free of charge, and thus, it has been widely used in water quality modeling, especially in data-scarce developing countries (Slaughter et al. [2014;](#page-36-8) Sieber and Purkey [2015](#page-36-7)). Besides, this integrated model has also been widely used to analyze the efficiency of different planned adaptation and mitigation measures for improving the water resources in the future (Kumar [2018;](#page-35-7) Mirauda and Ostoich [2018\)](#page-35-8). Noticeably, despite the lack of detailed water quality variables like several other models, the WEAP model supports scenario formation functionalities. These scenarios can be built with the involvement of policy and strategic plans in this model, based on diferent variables such as population growth, industrialization, and urbanization, climate change, land use land cover (LULC) change, the status of treatment plants and sewerage network, hydropower generation, etc. (Mishra et al. [2017](#page-35-6); Kumar [2018](#page-35-7)). Moreover, the WEAP hydrology module also allows the estimation of rainfall-runoff and pollutant travel from a catchment to water sources (Ingol-Blanco and McKinney [2013](#page-34-11)).

With the aforementioned facts, this research aims to quantify the current conditions and future outlook of water quality for the year 2030 in the Hau River fowing through Can Tho, Dong Thap, and Vinh Long Cities, Vietnam, based on four possible scenarios. The first scenario is BAU, where the effects of all possible key factors (population growth, urbanization, and climate change) afecting the Hau River water quality are considered.

The others are scenarios with measures, in which water treatment plants are considered as the mitigation measure for improving the river water environment. The second scenario denoted by "WM100" refers to an ideal situation considering 100% completion of the WWTPs of capacity planned in the cities' master plans, whereas the third scenario, i.e., "WM75" refers to 75% completion of the planned WWTPs because of some delay in the project completion in the cities. Considering the existing gap to achieve the desired water quality with WM100, a fourth scenario named "WM\_Opt." with the same characteristics and components as the MW100 scenario and the addition of RWTPs at serious water pollution centers along the Hau River was applied.

### **2 Study area**

The study area comprised Can Tho (north latitudes/east longitudes:  $9^{\circ}$  55'  $08'' - 10^{\circ}$  19' 38ʺ/105° 13ʹ 38ʺ–105° 50ʹ 35ʺ), Dong Thap (10° 07ʹ 59ʺ–10° 58ʹ 22ʺ/105° 12ʹ 00ʺ–105° 56' 01"), and Vinh Long (9° 52' 45"-10° 19' 50"/105° 41' 25"-106° 17' 00") Cities, the main regional urban center locating in the middle of the VMD (CTCPC [2020](#page-33-3); MOPI [2020\)](#page-35-9) (Fig. [1\)](#page-4-0). Can Tho City has nine districts, including fve urban districts and four rural ones, with a total area of nearly  $1,440 \text{ km}^2$  (CTCSO [2019](#page-34-12)). The total areas of Dong Thap and Vinh Long Cities are approximately  $3,384 \text{ km}^2$  (including two urban districts and ten rural ones) and  $1,526 \text{ km}^2$  (one urban district and seven rural ones), respectively (VNCSO [2018\)](#page-36-9). Can Tho is also the fourth most populous city in Vietnam (nearly 1.283 million in 2018 and projected to be 2.1 million by 2030), with approximately 67% living in urban and peri-urban areas (CTCSO [2019](#page-34-12); UN DESA [2019\)](#page-36-10). Similarly, Dong Thap and Vinh Long are also considered populous cities in Vietnam with nearly 1.694 and 1.052 million people in 2018, respectively (VLCSO [2018;](#page-36-11) VNCSO [2018](#page-36-9)).

Located in the middle of the Lower Mekong Basin, the altitude of these cities is at a maximum of 1 m above mean sea level and lower from the east-north  $(+1 \text{ m})$  to west-south (+0.8 m). In terms of weather, all these cities lie in a tropical and monsoonal climate with two distinctive seasons, i.e., rainy (May to November) and dry (December to April). Its average annual humidity is 83%, and annual rainfall ranges between 1,500 and 1,800 mm (of which 90% of rainfall occurs during the rainy season) (Konings [2012](#page-34-6)). The northeast of Can Tho City and the southwest of Dong Thap and Vinh Long Cities are bordered by the Hau River, one of the two main branches of the Mekong River fowing in Vietnamese territory (Fig. [1\)](#page-4-0). During the rainy season (September to October), the average water fow in the Hau River reaches  $15,000-16,000$  m<sup>3</sup>/s. In contrast, during the dry season, this flow drops to  $1,600-1,700 \text{ m}^3$ /s leading to water shortages in the cities (Neumann et al. [2011\)](#page-35-10).

The whole VMD region, including three cities, is characterized by fat terrain with a dense waterway network, which is essential to people's livelihoods. Here, water is central to everyday life and underpins the local economy, including industry, agriculture, transport, and commerce and service (Neumann et al. [2011\)](#page-35-10). In Can Tho City, noticeably, all fve urban districts, namely, Ninh Kieu, Cai Rang, Binh Thuy, O Mon, and Thot Not, are located along the Hau River in the northeast of the city. Similarly, Lap Vo and Lai Vung districts of Dong Thap City and Binh Tan and Binh Minh districts of Vinh Long City are also bordered by the Hau River (Fig. [1\)](#page-4-0). They are the nine most populous and crucial districts contributing to these cities' general development. Although the area of these nine districts only accounts for nearly  $20\%$  (1,156 km<sup>2</sup>) of the total area of Can Tho, Dong Thap, and Vinh Long Cities; their population makes up nearly 34% (nearly 1.338 million



<span id="page-4-0"></span>**Fig. 1** The location of the study area's nine districts along the Hau River (light blue) with several hydrological and water quality monitoring stations (dark blue) from upstream to downstream areas

people) of these cities' total population in 2018 (CTCSO [2019\)](#page-34-12). Table [1](#page-5-0) shows the nine districts' area and population characteristics in the study area (VLCSO [2018;](#page-36-11) VNCSO [2018;](#page-36-9) CTCSO [2019;](#page-34-12) UN DESA [2019;](#page-36-10) MOPI [2020\)](#page-35-9).



<span id="page-5-0"></span>

The surface water of the Hau River is the main water source supplied for all daily activities in these nine districts, while residents in the remaining rural districts mostly use groundwater for their water demand. Although there are abundant water resources available in the Hau River to cater to these nine districts' general demand, the growth of point and non-point pollution sources in recent years has increasingly become a matter of concern, especially for this river's water quality management. Particularly, point wastewater sources generated by rapid population and industrial growth along the Hau River are con-sidered the main reasons causing water pollution in this river (CTCPC [2019b\)](#page-33-1). To support this fact, seven monitoring stations and spatio-temporal trends in BOD, TC,  $NO<sub>3</sub><sup>-</sup>$ , and  $PO<sub>4</sub><sup>3–</sup>$  concentrations at an annual average for some monitoring stations across the study area in the period of 2009–2018 are shown in Fig. [2](#page-6-0). As can be seen from Fig. [2b](#page-6-0), [c,](#page-6-0) [d](#page-6-0), and [e](#page-6-0), all BOD, TC,  $NO_3^-$ , and  $PO_4^3$ <sup>-</sup> concentrations have mostly shown upward trends over these 10 years except for slight decreases from 2013 to 2015 thanks to a series of environmental projects (rehabilitating canal network, building an industrial WWTP, etc.) implemented during these three years (CTCPC [2014](#page-33-4); ODAPMU [2016\)](#page-35-11). However, these parameters' levels, especially BOD and TC, have signifcantly re-increased since 2016 due to the rapid urbanization, population growth, and land use changes in recent years (CTCPC [2019b\)](#page-33-1). Noticeably, BOD and TC concentrations are much higher compared to class A of the NTRSWQ, while an opposite trend can be seen in  $NO<sub>3</sub><sup>-</sup>$  and  $PO<sub>4</sub><sup>3-</sup>$  levels that are within the permissible ones. Another thing to note is that the pollution status is more serious in the Hau River downstream areas, where the most populous districts of the study area are located with many residential zones, trading centers, and industrial parks.

Noticeably, despite the reticulated water supplies system, there is a lack of sufficient WWTPs to handle both domestic and industrial wastewater generated in these heavily urbanized districts (CTCPC [2019b](#page-33-1)). At present, only one domestic WWTP with a treatment capacity of  $30,000 \text{ m}^3$  per day is functional in the study area. It accounts for only more than 25% of the study area's total daily discharged domestic wastewater (CTCPC [2019b;](#page-33-1) DOC [2019](#page-34-8)). Regarding industrial wastewater treatment, although the WWTPs for industrial parks in the study area are currently being operated, the existing treatment capacity and efficiency do not meet the increasing water demand for industrial activities along the Hau River (Trung et al. [2019b\)](#page-36-12). According to CTCPC ([2019b\)](#page-33-1),



<span id="page-6-0"></span>**Fig. 2** The BOD (**b**), TC (**c**), NO<sub>3</sub><sup>–</sup> (**d**), and PO<sub>4</sub><sup>3–</sup> (**e**) concentrations at a number of monitoring stations (**a**) across the study area in the 10-year period of 2009–2018

only fve centralized WWTPs are currently being used for industrial wastewater treatment with a total treatment capacity of 70,000  $m<sup>3</sup>$  per day, accounting for nearly 64% of the total daily industrial wastewater generated in the study area. Consequently, the remaining domestic and industrial wastewater is discharged directly into the Hau River and its diversion network.

In the current cities' master plan, therefore, developing policies and strategies to sustain rapid urbanization, signifcant industrial development, economic activities while maintaining desired water resources for the future are prioritized in these three cities (Long and Cheng [2018;](#page-35-5) MOPI [2018;](#page-35-12) DTCPC [2020](#page-34-13)). One of the current priorities for these cities is to expand advanced centralized WWTPs (CTCPC [2016,](#page-33-5) [2019a](#page-33-6)) to cater to all wastewater generated and achieve the ambient water quality of class A prescribed by the Vietnamese national standard (CTCPC [2016\)](#page-33-5).

### **3 Methodology**

### **3.1 The WEAP model**

In this research, the WEAP model was used to simulate rainfall-runoff and water quality variables in diferent scenarios to assess water quality management and policy alternatives amidst the population growth, industrial extension, and climate change in the Hau River basin. Spatio-temporal river pollution analyses were carried out in the scenario of wastewater increase and then proposed both new rehabilitated WWTPs and RWTPs. The fowchart for the research methodology is shown in Figure [13](#page-30-0) [Appendix 1.](#page-30-1)

#### **3.2 Hydrological/catchment simulation**

With support from the geographic information system (GIS) and remote sensing (RS), the study area's schematic maps were generated in the WEAP interface (Fig. [3\)](#page-8-0). Based on the geographical, topographical, geomorphological, hydrological, and climatic features as well as confuence points, then, the study area was required to be divided into nine smaller catchments. These catchments will be one of the main components for the hydrological simulation in the WEAP model (Sieber and Purkey [2015](#page-36-7); Kumar [2018](#page-35-7)).

Due to data availability, the rainfall-runoff method (simplified coefficient method) among fve diferent methods available was applied for the catchment simulation in this research. This method can simulate diferent components of the hydrologic cycle, including the catchment's potential evapotranspiration (by using crop coefficients), surface runof, and infltration of rainfall. In more detail, the available precipitation for the potential evapotranspiration (PrecipAvailableForET) and the catchment's potential evapotranspiration (ETpotential) were calculated by equations based on the real precipitation (Precip), catchment's area (Area), efective precipitation (PrecipEfective), reference evapotranspiration (ETreference), and crop coefficients  $(K_c)$  (Eqs. [\(1](#page-7-0)) and ([2\)](#page-7-1)). The runoff to both groundwater (RunofToGW) and surface water (RunofToSurfaceWater) can be calculated with Eqs. ([3\)](#page-7-2), ([4\)](#page-7-3), and ([5](#page-7-4)) (Sieber and Purkey [2015\)](#page-36-7).

$$
PrecipAvailableForET_{LC} = Precip_{HU} \times Area_{LC} \times 10^{-5} \times PrecipEffective_{LC}
$$
 (1)

<span id="page-7-4"></span><span id="page-7-3"></span><span id="page-7-2"></span><span id="page-7-1"></span><span id="page-7-0"></span>
$$
E\text{Tpotential}_{\text{LC}} = \text{E} \text{Treference}_{\text{HU}} \times \text{Kc}_{\text{LC}} \times \text{Area}_{\text{LC}} \times 10^{-5} \tag{2}
$$

$$
Runoff_{LC} = Max (0, PrecipAvailableFor ET_{LC} - ET potential_{LC})
$$
 (3)

$$
RunoffToGW_{HU} = \sum_{LC} (Runoff_{LC} \times RunoffToGWFraction_{LC})
$$
 (4)

RunoffToSurfaceWater<sub>HU</sub> = 
$$
\sum_{LC}
$$
(Runoff<sub>LC</sub> × (1 – RunoffToGWFraction<sub>LC</sub>)) (5)

where subscript LC is the land cover and subscript HU is the hydro-unit.

Pollutant transport from a catchment accompanied by rainfall-runoff is enabled by ticking the water quality modeling option. The surface runoff takes the accumulated pollutants from the catchment surfaces during non-rainy days to the water bodies. The formula of the



<span id="page-8-0"></span>**Fig. 3** The current (2018) and future (2030) schematic maps in the WEAP interface showing key components utilized for the Hau River surface water quality modeling

castaway catchment surface pollutants is by multiplying runoff volume and concentration (Sieber and Purkey [2015\)](#page-36-7). All parameters and datasets required for this hydrological/catchment simulation in this research are shown in Table [2.](#page-10-0)

#### **3.3 Water quality simulation**

Developed from the Streeter-Phelps model, the WEAP employs descriptive models of point source pollutant loadings that can simulate the impact of wastewater on receiving waters from demand sites and WWTPs. In terms of simulation of oxygen balance in a river, two processes are identifed, which are consumption by decaying organic matter and re-aeration induced by an oxygen defcit. The DO and BOD removal from water is a function of water temperature, settling velocity, and water depth (Eqs.  $(6)$  $(6)$ ,  $(7)$  $(7)$  $(7)$ ,  $(8)$  $(8)$ , and ([9\)](#page-11-1)) (Sieber and Purkey [2015](#page-36-7)).



**Fig. 3** (continued)

<span id="page-9-1"></span><span id="page-9-0"></span>
$$
BOD_{\text{final}} = BOD_{\text{init}} \exp^{\frac{-k_{\text{rBOD}}L}{U}} \tag{6}
$$

where 
$$
k_{\text{rBOD}} = k_{\text{d20}}^{1.047(T-20)} + \frac{v_s}{H}
$$
 (7)

BODinit: the BOD concentration of the pollutant loading (mg/l) at the top of the reach;

 $BOD<sub>final</sub>$ : the BOD concentration of the pollutant loading (mg/l) at the end of reach; *T*: the water temperature (in degrees Celsius); *H*: the depth of the water (m); and *L*: the reach length (m);

*U*: the water velocity in the reach; and  $v_s$ : the settling velocity (m/s);

 $k_r = 0.4$ ;  $k_d = 0.4$ , and  $k_a = 0.95$ : the re-aeration, decomposition, and reaction rates, respectively (1/time);

<span id="page-10-0"></span>



 $k_{d20}$ : The decomposition rate at a reference temperature of 20 °C.

Oxygen concentration in the water is a function of water temperature, DO, and BOD:

<span id="page-11-1"></span><span id="page-11-0"></span>Oxygen saturation or OS = 
$$
14.54 - (0.39T) + (0.01T^2)
$$
 (8)

$$
O_{\text{final}} = \text{OS} - \left(\frac{k_{\text{d}}}{k_{\text{a}} - k_{\text{r}}}\right) \left(\exp^{\frac{-k_{\text{r}}L/U}{U}} - \exp^{\frac{-k_{\text{r}}L/U}{U}}\right) \text{BOD}_{\text{init}} - \left[\left(\text{OS} - O_{\text{init}}\right) \exp^{\frac{-k_{\text{r}}L/U}{U}}\right] \tag{9}
$$

where  $O_{\text{init}}$  is the oxygen concentration (mg/l) at the top of the reach and  $O_{\text{final}}$  is the oxygen concentration (mg/l) at the end of the reach.

Even though these diferent parameters should be examined and measured in the individual research, most water quality parameters have been estimated from the secondary database due to the lack of time and funds (Bowie et al. [1985\)](#page-33-7).

#### **3.4 Data requirement**

To simulate the current status and predict the future for river water quality in the WEAP model, various input data were collected and its summary is shown in Table [2.](#page-10-0) The point pollution sources' locations and concentrations, past spatio-temporal water quality, WWTPs' and RWTPs' locations and capacities, past and projected population, water consumption, and current drainage networks were considered required datasets for the water quality simulation. The historical and future climate (rainfall, evaporation, temperatures, etc.), hydrological data (river fow-stage-width relationships, river length, surface water infows, etc.), LULC, and catchments' areas and locations were utilized for hydrological simulation. Besides, the cities' documents (reports, master plans, etc.) were also collected for the scenario building and evaluation.

To model water quality with WEAP, schematic diagrams (the virtual representation of the contributory elements of the water pollution in the study area) for both the current stage (the year 2018) and the target year (the year 2030) are prepared as shown in Fig. [3.](#page-8-0) As mentioned earlier, the Hau River basin was also divided into nine smaller catchments (CMs) areas with inter-basin transfers. Water demand sites (red dots) for residential (DSRs) and industrial (DSIs) zones mainly give a schematic representation of current and future water consumption in the study area. Based on the study areas' population and industrial distribution, their areas and locations were identified in the catchments along the Hau River. Locations of existing and planned WWTPs were also identified based on the cities' reports and master plans, while proposed RWTPs' sites were located based on the simulated results of river water quality conditions from the model using the scenario with measures (WM100). Located along the Hau River, gauges (dark blue nodes) represent water quality and streamflow monitoring stations. For more details, water quality data were monthly measured at Thot Not, Thom Rom, O Mon, Tra Noc, Binh Thuy, Khai Luong, and Cai Sau stations, while streamflow data were monthly recorded at three stations, namely Thot Not, Tra Noc, and Can Tho.

Historical climatic data, including daily temperature, rainfall, and evaporation for the 40 years from 1979 to 2018, were collected from the Department of Hydro-Meteorology of the cities (DONRE [2019](#page-34-14)). Instead of using rainfall from 2018, the 10-year average rainfall in the period of 2009–2018 from this study area was considered the current rainfall for setting up the model to reduce the bias infuence of climate variability on simulated outcomes. For climate change impact on water quality analysis, the future prediction of precipitation was done using diferent Global Climate Models (GCMs), and Representative Concentration Pathway (RCP) output was used after downscaling and bias correction. Changes in average monthly precipitation were calculated as an indicator of climate change on river water quality. Statistical downscaling and trend analysis (a less demanding computation technique to reduce biases in the precipitation frequency and intensity) (Goyal and Ojha [2011\)](#page-34-15) were employed to get monthly climate variables. A historical rainfall analysis with monthly precipitation data of 25 years (1989–2013) was integrated. In the proposal of climate change mitigation strategies, understanding and examining the potential impacts of climate change are vital. Based on the above-mentioned historical rainfall analysis of 25 years, in this research, an assessment of the possible climate change (mainly rainfall change) in the study area over the period 2020–2044 was carried out by using MRICGCM3 and MIROC5 as GCMs with RCP4.5 and RCP8.5 emission scenarios.

Similarly, LULC and hydrological data used in this research were also collected from the cities' Department of Hydro-Meteorology (DOHM), Department of Construction (DOC), and Department of Natural Resources and Environment (DONRE). As mentioned above, Thot Not, Tra Noc, and Can Tho are streamflow monitoring stations in the study area (Fig. [1](#page-4-0)). In which, monthly streamflow data have been measured in Thot Not and Tra Noc since 2013, while Can Tho station was established for monitoring since 2001 (DONRE [2019;](#page-34-14) MRC [2020](#page-35-13)). In this research, the collected data were applied for the calibration and validation processes by comparing them with the simulated values from the WEAP hydrology module simulation. In detail, average monthly streamflow values for the period of 2013–2018 at upstream stations, namely, Thot Not and Tra Noc, were used for calibration, while validation was separately done at Can Tho station located at the downstream area because of observed data availability for a longer period. Parameters utilized for the calibration process are effective precipitation and runoff/infiltration ratio, and the steps used to achieve the best fit are shown in Table [3](#page-12-0).

In this research, the Hau River water quality was assessed and simulated under the impact of wastewater discharged from industrial and domestic activities of cities' nine districts along the Hau River banks. Thus, these districts' population and industrial data in the period of 2013–2018 were collected from the cities' Statistical Offices (VNCSO) [2018;](#page-36-9) CTCSO [2019](#page-34-12)). The DSRs mainly give a schematic representation of domestic

Parameters	Initial values	Best-fit values	<b>Steps</b>	
Streamflow calibration				
Effective precipitation $(\%)$	100	98	$\pm 0.5$	
Runoff/infiltration ratio $(\%)$	50/50	40/60	± 5/5	
Water quality calibration				
Riverhead's BOD (mg/l)	8	5	$\pm 0.5$	
Riverhead's TC (CFU/100 ml)	500	650	$\pm 25$	
Riverhead's $NO_3$ <sup>-</sup> (mg/l)		0.5	$\pm 0.1$	
Riverhead's $PO43- (mg/l)$	0.07	0.055	$\pm 0.005$	

<span id="page-12-0"></span>**Table 3** Summary of parameters and steps used for streamfow and water quality calibration processes

water consumption (130, 150, and 180 L/capita/day in the periods of 2013–2015 (DONRE [2015;](#page-34-16) DOC [2019\)](#page-34-8), 2016–2025 (Trung et al. [2019b\)](#page-36-12), and 2026–2030 (MOC [2011](#page-35-14); CTCPC [2019b\)](#page-33-1), respectively) and wastewater pollution loads (domestic discharge) per capita (DOC [2019\)](#page-34-8). In order to evaluate the impact of population growth on the future river water quality, the future population distribution at these DSRs was also calculated by ratio method using the United Nations Department of Economic and Afairs (UNDESA) projected growth rate (UN DESA [2019](#page-36-10)). In terms of industry, the water consumption and demand of current and future DSIs were calculated and assumed based on the cities' reports and master plans (CTCPC [2019b;](#page-33-1) WASSC [2013](#page-36-13), [2020\)](#page-36-14).

For river water quality data, BOD, TC,  $NO<sub>3</sub><sup>-</sup>$ , and  $PO<sub>4</sub><sup>3-</sup>$  concentrations, which were monitored on a monthly basis for the period of 2013–2018 by the Can Tho Centre of Natural Resources and Environment, were used (DONRE [2020](#page-34-17)). This observed data of river water quality were seasonally compared with the simulated ones with the aim of calibrating and validating the WEAP water quality modeling component. In the calibration process, the seasonal values of BOD, TC,  $NO<sub>3</sub><sup>-</sup>$ , and  $PO<sub>4</sub><sup>3–</sup>$  concentrations in the wet (May to November) and dry (December to April) periods for the year 2013–2018 at four upstream stations, namely, Thot Not, Thom Rom, O Mon, and Tra Noc, were used for calibration, while concentrations at Binh Thuy, Khai Luong, and Cai Sau stations located in lower areas were utilized for validation. Table [3](#page-12-0) shows the Hau Riverhead water quality parameters adjusted for the calibration process.

In the study area, the numbers of centralized WWTPs and RWTPs as wastewater and river water handling infrastructures considered for current and future scenarios were 6 (DOC [2019\)](#page-34-8) and 25 (Master Plans), respectively (Fig. [3](#page-8-0)). In the scenarios with mitigation measures, the sewerage connection rates for households, industrial factories, and the Hau River were assumed as equals to the percentages of total wastewater and river water being transported to centralized WWTPs and RWTPs. In other words, clogging and leakage of sewerage pipelines were neglected. In this research, the sewerage pipeline systems for households and industrial factories were the only input sources of domestic and industrial wastewater into the Hau River and its tributaries, while the system used to collect polluted river water for treating was also applied for transporting river water only. In terms of technology, current WWTPs adopted in the model was of up-fow anaerobic sludge blanket reactor coupled with trickling flter (UASB-TF) type (Chernicharo and Nascimento [2001;](#page-33-8) JICA [2010\)](#page-34-18) (with the fltering velocity: only 0.1–0.5 m/h) in wastewater treatment, and their average treatment efficiency for BOD, TC,  $NO<sub>3</sub><sup>-</sup>$ , and  $PO<sub>4</sub><sup>3–</sup>$  levels are calculated as 86.47%, 89.63%, 50.79%, and only 31.59% respectively (DOC [2019\)](#page-34-8). However, the key aim of the cities' master plans is to achieve class A of surface water quality (BOD ≤6 mg/l, TC ≤ 5,000 CFU/100 ml,  $NO_3^-$  ≤ 5 mg/l, and  $PO_4^{3-} \leq 0.2$  mg/l) (MONRE [2015\)](#page-35-15) prescribed by Vietnamese national standard with all possible mitigation measures in the future (DOC [2019\)](#page-34-8). Therefore, more advanced treatment technology—the up-fow anaerobic sludge blanket reactor coupled with sequencing batch reactor (UASB-SBR) type (Khan et al. [2013](#page-34-19); Kumar [2018\)](#page-35-7)—will be adopted for both new WWTPs and RWTPs in the future scenarios of 2030 (CTCPC [2016](#page-33-5)). Consequently, the average treatment efficiency of these new treatment plants for 2030 will be 97.26%, 99.72%, 75.55%, and 76.65% for the BOD, TC,  $NO_3^-$ , and  $PO_4^{\,3-}$  levels, respec-tively (DOC [2019](#page-34-8)). (For details, Table [6](#page-31-0) of [Appendix 2](#page-31-1) indicates the characteristics of the existing and planned WWTPs as well as proposed RWTPs along the Hau River in the study area.)

As mentioned earlier, the compulsory processes of the water quality simulation are the calibration and validation processes performed with river discharge and water quality parameters. Once satisfed statistically, numerical simulation using diferent future scenarios called business-as-usual (BAU) scenario and scenarios with mitigation measures (WM75, WM100, and WM\_Opt. with WWTPs and RWTPs) will be applied.

Assumptions made.

The following assumptions are made for this study.

(i) No consideration about non-point pollution sources because of the unavailability of the dataset. However, if the data are available, the WEAP model can handle simulation of non-point source pollutants like pollutants from agricultural runoff. For example, if we would like to consider the simulation of pollutants coming from agricultural runof, we need to know the area of agricultural felds, chemical composition and amount of fertilizers used, amount of irrigation water used, etc., then we have an empirical formula to calculate the pollutant generated from runoff. Specifcally, according to USEPA ([2001\)](#page-36-15), Sieber and Purkey ([2015](#page-36-7)), and Kipyego and Ouma ([2018](#page-34-20)), the non-point sources' load contribution from each land use type at catchment level is estimated using the simple and export coefficient methods based on the data availability and applicability on a watershed. In these methods, the following equation is used for the estimation of constituents such as total suspended solids (TSS), BOD, NO*x* (nitrate and nitrite), ammonia, and fecal coliforms.

$$
L_{\rm p} = \sum_{\rm p} (L_{\rm PU} \times A_{\rm U}) \tag{10}
$$

where  $L_p$  is the pollutant load (kg/year);  $L_{PI}$  is the pollutant export coefficient for each land use (kg/hectare/year); and  $A_U$  is the area by certain land use (hectare).

- (ii) No consideration about the seasonal fuctuations of water consumption and wastewater discharge. Because the diferences in water consumption and wastewater discharge between the wet and dry seasons are insignifcant (WASSC [2020](#page-36-14));
- (iii) No consideration about the mixing of stormwater with domestic wastewater in the sewerage pipelines. This is because, since 2017, domestic wastewater drainage pipelines have been operating independently (DOC [2019\)](#page-34-8).

#### **4 Results and discussion**

#### **4.1 Future pattern of precipitation**

To study the climate change impact on water quality, the change of rainfall pattern in the future was forecast using the GCMs (MRICGCM3 and MIROC5) and RCPs (RCP4.5 and RCP8.5). Comparison of average monthly rainfall for current, i.e., 10-year average, and future, i.e., 2030, years is shown in the line chart in Fig. [4.](#page-15-0) Although it is evident that the diferences between annual average rainfall for both the current and future simulated values are small, this study further investigates the impact of seasonal variation in rainfall patterns on water quality.

#### **4.2 Population and industrial growth**

In this study area, population growth and intensive industrialization were considered to have a potential threat to river water quality in the future. The current population of the



<span id="page-15-0"></span>**Fig. 4** The current and future monthly precipitation at the Can Tho station

study area in 2018 (base year) collected from the cities' statistical offices was 1,337,931 people (VNCSO [2018](#page-36-9); VLCSO [2018\)](#page-36-11). Based on the projected growth rates in the UN DESA's report (UN DESA [2019\)](#page-36-10), the ratio method was used for the study area's future population prediction (see Table [7](#page-33-9) of [Appendix 2](#page-31-1) for projected population growth rates of the study area from 2018 to 2030). The current (2018) and future (2030) populations of each district/catchment as well as the total population of the study area are calculated and shown in Table [1.](#page-5-0) It is predicted that the estimated population in the target year 2030 (1,974,767 people) will be 47.6% higher than that in the base year 2018 (1,337,931 people). Remarkably, this increasing population has resulted in much higher water demand for domestic activities in the study area's residential areas in the future, from 73.25 million  $m<sup>3</sup>$  in 2018 to 130.33 million  $m<sup>3</sup>$  in 2030. Regarding industrial development in the future, based on the cities' reports and master plans, the used data are the number of new planned industrial parks; their locations and areas; and water demand (MOC [2009;](#page-35-16) DONRE [2015;](#page-34-16) VLCPC [2018](#page-36-16)). Eight new planned industrial zones considered in the study area are High-Tech Can Tho, O Mon, Bac O Mon, Song Hau 2, Song Hau 3, HighTech Lai Vung, Dong Binh, and Binh Tan. The total area and daily water consumption of these planned industrial zones were 3,090 ha and 299.3 million  $m<sup>3</sup>$ , respectively (MOC [2011](#page-35-14); MOPI [2018;](#page-35-12) DTCPC [2015,](#page-34-21) [2020](#page-34-13)). Figure [14](#page-30-2) of [Appendix 1](#page-30-1) indicates the study area's comparative water demands of residents and industry between 2018 and 2030. In the absence of precise information, the amount of wastewater being generated from both domestic and industrial sites for the WEAP modeling was calculated based on the actual demand. For example, in the case of the domestic sector, wastewater generated was roughly 80% of the total water being consumed (DOC [2019](#page-34-8)), whereas for the industrial sector it was 75% (JICA [2017;](#page-34-22) CTCCP [2019b](#page-33-1); WASSC [2020](#page-36-14)).

#### **4.3 Water quality simulation**

#### **4.3.1 Model performance evaluation**

For streamflow and water quality calibration processes, effective precipitation, runoff/infiltration ratio, and the Hau Riverhead water quality parameters (with their best-ft values



<span id="page-16-0"></span>**Fig. 5** The comparative values of average monthly observed and simulated streamfow for the year 2013– 2018 for calibration (Thot Not and Tra Noc stations) and validation (Can Tho station) processes in the Hau River

shown in Table [3\)](#page-12-0) were adjusted using the trial-and-error method. These processes aim to obtain simulated results having a high association with observed values. The association level is represented by correlation coefficients  $R$ -squared  $(R^2)$ , root mean square error (RMSE), and mean absolute percentage error (MAPE). The calibration and validation results of hydrological components through comparative results between the average monthly observed and simulated streamfow for the year 2013–2018 at three hydraulic stations (namely Thot Not, Tra Noc, and Can Tho) in the Hau River are shown in Fig. [5](#page-16-0). Basically, both simulated streamfow and observed streamfow for Thot Not, Tra Noc, and Can Tho stations have shown high associations supported by  $R^2 \approx 0.96$ , RMSE $\approx 0.17$ , and MAPE≅15.57%;  $R^2 \cong 0.99$ , RMSE≅0.10, and MAPE≅6.06%; and  $R^2 \cong 0.99$ , RMSE $\cong$  0.11, and MAPE $\cong$  5.73%, respectively.

Results for calibration and validation processes in case of water quality parameters, i.e., BOD, TC,  $NO_3^-$ , and  $PO_4^3^-$ , were shown through the comparison between average seasonally observed and simulated concentrations for the year 2013–2018 at seven stations, namely Thot Not, Thom Rom, O Mon, Tra Noc (selected for calibration), Binh Thuy, Khai Luong, and Cai Sau (used for validation). As Figs. [6](#page-17-0), [7](#page-18-0), [8](#page-18-1) and [9](#page-19-0) show, signifcant associations are also found between observed and simulated average seasonally BOD, TC,  $NO_3^-$ , and  $PO_4^{3-}$  levels in both the wet and dry periods at selected stations. These matches were expressed through the fluctuations of  $R^2 \approx 0.76-0.99$ , RMSE $\approx 0.02-0.16$ , and MAPE $\approx$  2.48–16.61% for the comparison of these four parameters. Thus, the calibration and validation results for both streamfow and water quality parameters of the Hau River had demonstrated high reliability and suitability of the model performance for both hydrological and water quality modeling.



<span id="page-17-0"></span>**Fig. 6** The comparative values of average seasonally observed and simulated BOD levels in the period of 2013–2018 utilized in the calibration (**a**, **b**) and validation (**c**, **d**) processes



<span id="page-18-0"></span>**Fig. 7** The comparative values of average seasonally observed and simulated TC levels in the period of 2013–2018 utilized in the calibration (**a**, **b**) and validation (**c**, **d**) processes



<span id="page-18-1"></span>**Fig. 8** The comparative values of average seasonally observed and simulated NO<sub>3</sub><sup>−</sup> levels in the period of 2013–2018 used in the calibration (**a**, **b**) and validation (**c**, **d**) processes



<span id="page-19-0"></span>**Fig. 9** The comparative values of average seasonally observed and simulated  $PO_4^{3-}$  levels in the period of 2013–2018 used in the calibration (**a**, **b**) and validation (**c**, **d**) processes

### **4.3.2 Predicting future water quality using scenario analyses**

Scenario analysis was done to predict possible future water quality status in both the wet and dry seasons and evaluate the efectiveness of the proposed countermeasures in water pollution mitigation. In this study, the future changes in BOD, TC,  $NO<sub>3</sub><sup>-</sup>$ , and  $PO<sub>4</sub><sup>3</sup>$ concentrations were simulated and analyzed under four diferent future scenarios. While the scenario with BAU employs key drivers to water quality infuence such as population growth, industrial development, and climate change (mainly rainfall changes); countermeasures, i.e., wastewater treatment policy change (WWTPs' and RWTPs' application) will be included in other scenarios. Noticeably, the study area's government has often faced a number of difculties related to the fnancial limitation, technology changes, and policy barriers that lead to the delayed completion of WRM projects in recent years (listed in Table [8](#page-33-10) of [Appendix 2](#page-31-1) as examples). Therefore, future scenarios with mitigation measures had been built based on the study area's more realistic conditions in recent years. Currently, to solve the current and future Hau River water pollution, the study areas' government is planning to build from 11 to 14 new centralized WWTPs with the national budget and the Japanese Official Development Assistance (ODA) loan. The major target of this investment is to collect and treat 75 to 100% of the total domestic and industrial wastewater discharged from the study areas' point pollution sources by 2030 (CTCPC [2016](#page-33-5)). Based on the cities' current condition and future strategies, therefore, two future mitigation measures (with centralized WWTPs applied for treating 100% (WM100) and only 75% (WM75) of total wastewater, respectively) were assumed and analyzed. In addition, to achieve the ambient water quality of class A prescribed by the Vietnamese national standard (CTCPC [2016\)](#page-33-5) in case the WM100 scenario is unable to bring about the expected efectiveness, an optimistic scenario (WM\_Opt.) has been also

built. This additional scenario possesses similar components as the WM100 scenario but with the addition of fve RWTPs for treating surface water at seriously polluted locations along the Hau River in 2030 (Fig. [3](#page-8-0)). As previously mentioned, sites of planned WWTPs were identifed based on the cities' reports and master plans, while proposed RWTPs' sites were located based on the pollution status of the Hau River water quality, which was simulated from the model.

One thing to note is that according to CTCPC [\(2019b\)](#page-33-1) and MOC [\(2020](#page-35-17)), in the study area, the total average costs investing for a water treatment plant (treatment capacity: 30,000 m<sup>3</sup> per day) with the technology of UASB-TF and UASB-SBR are nearly 22.69 and 24.25 million USD, respectively. All scenarios used in the study as well as their key characteristics, components, and estimated costs are briefy indicated in Table [4](#page-21-0).

In the four considered scenarios, the BOD, TC,  $NO<sub>3</sub><sup>-</sup>$ , and  $PO<sub>4</sub><sup>3-</sup>$  concentrations of the Hau River surface water were simulated on a seasonal basis for the year 2030. After establishing proposed scenarios, the Hau River water quality simulations were modeled, and obtained results are shown in the bar charts and maps in Fig. [10.](#page-22-0) In addition, these simulated results are also illustrated at the spatial scale in Figs. [11](#page-24-0) and [12.](#page-25-0) The diferences in simulated parameters' concentrations caused by rainfall changes from the GCMs with RCP4.5 and RCP8.5 emission scenarios are shown by small bars on the top of bar charts. It can be noticed that the impacts of rainfall changes on BOD, TC,  $NO_3^-$ , and  $PO_4^{3-}$  concentrations of the Hau River water in both wet and dry seasons were negligible. This might be explained by the insignifcant diference between the increased volume of rainfall in the future (as shown in Fig. [4\)](#page-15-0) and the Hau River streamfow.

In the current year, as can be seen from Fig. [10,](#page-22-0) simulated BOD, TC,  $NO_3^-$ , and  $PO_4^{3-}$ concentrations from most monitoring stations, except for some ones in the Hau River downstream areas, in the wet season were not too high, fuctuating from 5.36 to 11.42 mg/l, from 3,391 to 6,127 CFU/100 ml, from 0.57 to 0.93 mg/l, and only from 0.06 to 0.09 mg/l respectively. Compared to class A of the NTRSWQ, the Hau River surface water quality in the wet season of 2018 was relatively polluted in terms of BOD and TC at Binh Thuy, Khai Luong, and Cai Sau stations despite this river's large discharge during the rainy period (Fig. [10a](#page-22-0) and [c\)](#page-22-0). Noticeably, regarding  $NO_3^-$  and  $PO_4^{3-}$ , these parameters' current levels were much lower compared to the permissible ones of class A (Fig. [10e](#page-22-0) and [g\)](#page-22-0). For the BAU scenario, consequently, the BOD and TC pollution has increasingly become more serious in 2030 under the impacts of intensive population and industry growths, and rainfall change without any appropriate mitigating measure applied, while  $NO<sub>3</sub><sup>-</sup>$  and  $PO<sub>4</sub><sup>3</sup>$ concentrations are still within the desired limits of class A1. In this scenario, it can be seen that the BOD and TC levels in the wet season at four downstream stations of the Hau River basin have sharply increased and are much higher compared to class A, ranging from 7.95 to 14.48 mg/l and from 6,470 to 9,973 CFU/100 ml, respectively (Figs.  $10a$  and [c](#page-22-0) and [11a\)](#page-24-0). Because of the key drivers (population growth, industrial development, and climatic changes), the average levels of BOD, TC,  $NO_3^-$ , and  $PO_4^{3-}$  in the wet season will be increased by 16.01%, 40.85%, 30.49%, and 20.22%, respectively by 2030 compared to 2018 (Table [5\)](#page-25-1).

In the dry season, the Hau River water pollution has intensively become more serious in both the current (2018) and the coming years (2030). Simulated BOD, TC, and  $PO_4^{3-}$ levels at most stations, especially for the ones downstream, in 2018 were much higher compared to class A of the NTRSWQ, fuctuating from 6.22 to 64.34 mg/l, from 4,024 to  $14,306$  CFU/100 ml, and 0.08 to 0.45 mg/l, respectively (Figs.  $10b$ , [d,](#page-22-0) and [h](#page-22-0)). Noticeably, simulated BOD, TC, and  $PO_4^{3-}$  levels at four downstream stations in 2030 for the

<span id="page-21-0"></span>

Table 4 The summary of key components of four different scenarios used for the future water quality simulation **Table 4** The summary of key components of four diferent scenarios used for the future water quality simulation



<span id="page-22-0"></span>**Fig. 10** The average seasonally comparative results of simulated BOD ( $\bf{a}$ ,  $\bf{b}$ ), TC ( $\bf{c}$ ,  $\bf{d}$ ), NO<sub>3</sub><sup>-</sup> ( $\bf{e}$ ,  $\bf{f}$ ), and PO<sub>4</sub><sup>3−</sup> (**g**, **h**) levels for the current status and future scenarios. The error bars show differences in simulated levels caused by rainfall changes from the GCMs with RCP4.5 and RCP8.5 emission scenarios



**Fig. 10** (continued)



<span id="page-24-0"></span>**Fig. 11** The seasonally spatial variations of BOD (red numbers), TC (black),  $NO<sub>3</sub><sup>-</sup>$  (blue), and  $PO<sub>4</sub><sup>3-</sup>$  (gray) levels simulated in scenarios in the wet season

BAU scenario will further deteriorate in comparison with these levels in the current year, ranging from 17.64 to 89.21 mg/l, from 12,700 to 27,345 CFU/100 ml, and from 0.22 to 0.59 mg/l, respectively (Fig.  $12a$ ). These significantly high levels are mostly due to untreated wastewater discharged from future domestic and industrial activities as well as low discharge and severe droughts in the Hau River during the dry period. Regarding  $NO<sub>3</sub><sup>-</sup>$  level in the BAU scenario, only the nitrogenous pollution at Cai Sau station should be noted and solved in the future (Fig. [10f](#page-22-0)). Table [5](#page-25-1) also shows the average increased percentages of future BOD, TC,  $NO_3^-$ , and  $PO_4^3$ <sup>-</sup> levels in the dry season as compared to these levels in 2018. From simulated results, it can be clear that these parameters' current and future concentrations at all monitoring stations along the Hau River in the dry season are much higher than those in the wet one. For the year 2030, without any measures applied, indeed, the average BOD, TC,  $NO<sub>3</sub><sup>-</sup>$ , and  $PO<sub>4</sub><sup>3-</sup>$  levels in the dry season are from two to threefold as much as those in the wet one. Remarkably, the highest diference in TC level can be seen at Cai Sau station, at 9,973 CFU/100 ml in the wet season as opposed to 27,345 CFU/100 ml in a dry one (Figs. [11a](#page-24-0) and [12a\)](#page-25-0). For BOD, this diference has amounted to more than sixfold, at 14.48 mg/l as compared to 89.21 mg/l (Figs. [11a](#page-24-0) and [12a\)](#page-25-0). In terms of  $NO_3^-$  and  $PO_4^{3-}$  levels, the highest differences are also noted at the lowest monitoring station, at 1.21 mg/l as compared to



<span id="page-25-0"></span>**Fig. 12** The seasonally spatial variations of BOD (red numbers), TC (black),  $NO_3^-$  (blue), and  $PO_4^{3-}$  (gray) levels simulated in scenarios in the dry season

<span id="page-25-1"></span>![](_page_25_Picture_273.jpeg)

5.93 mg/l and 0.12 mg/l as opposed to 0.59 mg/l respectively (Figs. [11a](#page-24-0) and [12a](#page-25-0)). Key factors responsible for the diferences in river water pollution levels between the wet and dry seasons in the study area are seasonal changes in rainfall and especially the Hau River discharge.

With the aim of generating more detailed and practical use of this result to support the cities' policymakers and master plans, the seasonal impact of each considered driver on the Hau River water quality was separated and assessed individually and the result was shown in Table [5.](#page-25-1) There is an insignificant difference in contribution rates between the two seasons. For BOD concentration, industrial growth has accounted for the highest percentage of contribution, fluctuating from 98.74 to 98.81%, whereas the opposite is true of rainfall change and population growths (only  $0.11-0.16\%$  and  $1.08-1.10\%$ , respectively). This is due to the fact that the typical average BOD concentration of industrial wastewater was nearly 13-fold higher than that of residential wastewater, at 1,000 mg/l as opposed to 80 mg/l for the latter (JICA [2017](#page-34-22); CTCPC [2019b;](#page-33-1) DOC [2019\)](#page-34-8). Moreover, the increased volume of industrial wastewater of the study area in 2030 considered in this model will be very large in comparison to that of domestic wastewater (Figure [14](#page-30-2) of [Appendix 1\)](#page-30-1). Regarding the increased TC level in 2030, the industrial expansion and population growth have shared similarly high rates of contribution to TC pollution in both the wet and dry seasons, ranging from 43.44 to 43.66% and 55.51 to 55.74%, respectively. This similarity can be explained by the converse TC level in domestic and industrial sewages. Specifically, although the volume of the forecast wastewater from domestic activities in 2030 is only about one-fifth of the industrial wastewater  $(57.08 \text{ million m}^3 \text{ versus}$ 299.03 million  $m<sup>3</sup>$ ), the average recorded TC level of the former is much higher than that of the latter (8.8 million CFU/100 ml versus 1 million CFU/100 ml) (Phung et al. [2015;](#page-36-2) Nga et al. [2018;](#page-35-18) DOC [2019](#page-34-8)). Noticeably, the contribution through the change in future rainfall trend and volume is also relatively small (only 0.82–0.83%). This result is consistent with the statement mentioned earlier in the future rainfall forecast.

In terms of  $NO_3^-$  and  $PO_4^3$  increases, the contribution rates of industrial development in both the wet and dry seasons fuctuate around 60%, while around 34% are contributed by population growth. These were due to that although typical average  $NO<sub>3</sub>$ <sup>-</sup> and  $PO_4^3$ <sup>-</sup> levels of domestic wastewater in the study area were higher than those of industrial wastewater (at NO<sub>3</sub><sup>-</sup>:19 mg/l and PO<sub>4</sub><sup>3-</sup>:1.5 mg/l as opposed to NO<sub>3</sub><sup>-</sup>: 5 mg/l and  $PO_4^{3-}$ : 0.4 mg/l for the latter (JICA [2017;](#page-34-22) CTCPC [2019b](#page-33-1); DOC [2019](#page-34-8))), the volume of the predicted wastewater from residential activities is much lower compared to the industrial waste. One thing to note is that the precipitation changes in the future have accounted for worth-noting percentages of contribution on the increases in both  $NO<sub>3</sub>$ and  $PO<sub>4</sub><sup>3-</sup>$  levels, fluctuating from more than 5% to nearly 7%. This can be explained that urban runofs and especially agricultural runofs with quite high input of inorganic nitrogenous and phosphorus fertilizers and manures (Kumar et al. [2010,](#page-35-19) [2019;](#page-35-20) Minh et al. [2020](#page-35-21)) in the study area are also considered remarkable contributors of  $NO_3^-$  and  $PO<sub>4</sub><sup>3–</sup> concentrations to the Hau River.$ 

Under the negative impacts from inevitable drivers (population and industrial growths) and pressure (climate change) on the Hau River water quality in the future, the WEAP model was used to evaluate the efectiveness of potentially structural measures in the study area. In three scenarios with measures, building WWTPs (with the total treatment capacities of nearly 1.118 and 1.49 million  $m^3$  per day) and RWTPs (0.775 million  $m^3$  per day) with advanced treatment technology UASB-SBR for treating 75% and 100% wastewater discharged from residential and industrial zones as well as the polluted Hau River water in the future was considered. These water treatment plants' efectiveness is simulated, assessed, and also illustrated in Figs. [10](#page-22-0), [11,](#page-24-0) and [12.](#page-25-0)

In both scenarios with mitigation measures (WM75 and WM100), as shown from these fgures, the application of WWTPs led to the considerable decline in the average BOD, TC,  $NO_3^-$ , and  $PO_4^{3-}$  levels in the wet season of 2030, by 35.28% (for WM75)/47.68% (WM100), 35.65% (MW75)/48.39% (WM100), 33.80% (MW75)/46.09% (WM100), and 29.10% (MW75)/38.03% (WM100) respectively compared to those in the BAU scenario. Noticeably, the simulated values of four these parameters' levels in the wet season at all stations will meet the desired water quality of class A (BOD≤6 mg/l, TC ≤ 5,000 CFU/100 ml,  $NO_3^-$  ≤ 5 mg/l, and  $PO_4^{3-}$  ≤ 0.2 mg/l) (MONRE [2015](#page-35-15)) of the NTRSWQ except for BOD levels at Cai Sau station. The average BOD concentrations at this station for WM75 and WM100 were only 8.90 and 6.91 mg/ml, which were just over that in class A (Figs. [10a](#page-22-0) and [11b](#page-24-0) and [c\)](#page-24-0). Regarding water quality in the dry season, however, the simulated concentrations of both BOD, TC, and  $PO<sub>4</sub><sup>3–</sup>$  for both scenarios WM75 and WM100 indicated that the application of WWTPs has given high efectiveness for river water quality improvement at the upstream stations only. At the remaining stations in lower areas, including Tra Noc, Binh Thuy, Khai Luong, and Cai Sau; TC,  $PO_4^{3-}$ , and especially BOD levels were still much higher compared to class A, ranging from 6,041 to 11,266 CFU/100 ml, from 0.13 to 0.42 mg/l, and from 8.02 to 52.48 mg/l, respectively (Figs. [10d,](#page-22-0) [10f,](#page-22-0) [10b](#page-22-0), [12b,](#page-25-0) and [12c\)](#page-25-0). In the WM100, however, TC levels in the dry season at most stations along the Hau River were quite low and meet the desired level of class A (Fig. [10d](#page-22-0)). It is also noteworthy that the study area's  $NO<sub>3</sub><sup>-</sup>$  pollution in the dry season is not a concerning matter in 2030.

Based on simulated outcomes, it can be seen that the BOD, TC,  $NO_3^-$ , and  $PO_4^{3-}$  pollution of the Hau River water in the study area was signifcantly improved in the wet season by adopting WWTPs with advanced technology treatment of UASB-SBR. In the dry season, however, the pollution of BOD and  $PO<sub>4</sub><sup>3-</sup>$  had not been completely resolved in t[h](#page-22-0)is river downstream areas (Fig.  $10b$  and h). The key reasons responsible for this issue are that (i) the residual concentrations of pollutants after wastewater treatment in the upstream area will be enhanced in the downstream regions by accumulative efects, (ii) being located at the downstream regions of the study area, the most populous and industrialized districts, namely O Mon, Binh Thuy, Ninh Kieu, Cai Rang, and Binh Minh, hence release a higher amount of wastewater, and (iii) the Hau River streamfow is very low in the dry months. Therefore, in the optimistic scenario (WM\_Opt.), the application of RWTPs with the optimal and possible lowest treatment capacity has been proposed with the aim of thoroughly solving the water quality pollution of the entire Hau River in general and its downstream areas in particular. As shown in Figs. [10](#page-22-0) and [11](#page-24-0), thanks to the application of five RWTPs (with a total treatment capacity of  $775,000 \text{ m}^3$ per day) at key locations along the Hau River, the BOD and  $PO_4^{3-}$  levels at downstream stations have significantly decreased. Indeed, the BOD and  $PO<sub>4</sub><sup>3-</sup>$  levels at Tra Noc, Binh Thuy, Khai Luong, and Cai Sau stations in the dry season have only ranged from 5.04 to 6.08 mg/l and from 0.08 to 0.21 mg/l, respectively (Figs.  $10b$ ,  $10b$ , and  $12d$ ). It is also noteworthy that the water quality improvement in the dry season is much higher than that in the wet season. This is because the diference between the total treatment capacity of proposed plants and the Hau River discharge in the dry season is much lower compared to that in the wet period.

#### **4.4 Merits and demerits of proposed measures**

Although the application of WWPTs has proven its efectiveness on river water pollution mitigation, it cannot be considered as a standalone solution for improving water quality to reach a desirable value of BOD and  $PO<sub>4</sub><sup>3−</sup>$  under class A without an additional support of RWTPs. Reason for selection and application of RWTPs within the WM\_Opt. scenario proposed for achieving the desired SWQ of the city are appended below. First of all, based on the simulated results from W100, it was found that even if 100% of future wastewater was collected and treated, the Hau River water with the BOD and  $PO_4^{3-}$  pollution in the dry season could not be thoroughly resolved in downstream areas. Also, treatment efficiency of the applied WWTPs under W100 scenarios was equipped with most cutting-edge one technology. So, there is no scope of either increasing the capacity of WWPTs or their treatment efficiency. Apart of rapid industrial urbanization, the issues remain here is the accumulation of pollutants in river downstream area, especially in the dry months, which is a concerning driver leading to this river water quality degradation. An Giang, an upstream city (Fig. [1\)](#page-4-0), is also one of the prominent agro-industrial cities in Vietnam; hence, the quality of river water fowing from this city to the study area can also be considered under the wastewater management and treatment plans. Moreover, according to the current and future master plans, the cities' governments are investing in environmental projects to improve the future river water quality by mainly using structural measures, especially applying more advanced wastewater treatment plants. The national budget and international grants are the main fnancial sources of these projects. With the aforementioned facts, considering the existing gap to achieve the desired 2030 water quality with WM100, RWTPs at serious water pollution locations along the Hau River were considered to apply in WM\_Opt.

However, implementation of proposed structural measures might have some negative issues as well like fnancial burden for building supporting structures like pipelines, land appropriation in urbanized centers, and disturbances to the local ecosystems. For example, the estimated costs for investing and building water treatment plants in proposed scenarios were relatively high, ranging from 822.48 to 1,750.04 million USD. Therefore, the local authorities should not consider the application of these plants as the sole solution for the study area. Hence, proposing a hybrid solution, where along with building these WWTPs, other options especially green infrastructures or nature-based solutions, viz. constructed wetlands, rain gardens, vegetated swales and median strips, bioswales, porous pavement, and enhancement of riparian bufers and foodplains should be considered for managing industrial wastewater, which can be more environmentally sustainable and cost-efective. Nevertheless, doing technical, economical, and socio-ecological evaluation of these green infrastructures is beyond the scope of this study. Because as mentioned before, this study aims to (i) assess the operational efficiency of water treatment plants proposed in current master plans of the cities to enhance the future Hau River water and thereby (ii) support policymakers in reviewing these plans as well as proposing more environmentally resilience approaches. Therefore, there would be the research gap with the application of these green measures for future studies.

In addition, based on the estimated contribution rates to the water pollution, industrial growth was attributed as the biggest driver of the future Hau River water pollution compared to the other inevitable drivers, i.e., population growth and climate change. Thus, stricter but technically feasible policies and regulations

should be issued to restrict both effluent concentration and volume of wastewater discharged from industrial plants into river bodies. These policies and regulations can focus on main requirements ranging from promoting decentralized WWTPs, adopting industrial symbiosis and sustainability-oriented hybrid solutions, and applying onsite green technologies (e.g., absorbent gardens, green rooftops, roadside plantings, storm water tree trenches) for wastewater treatment and sustainable resource management. Furthermore, concepts of circular economy, sustainable cleaner production, and Industry 4.0 standards should also be considered by the study area's government in the future. Indeed, circular economy practices are being considered one of the most important factors for increasing sustainability performance in manufacturing, followed by the practices of cleaner production and Industry 4.0. Supply chain traceability and information, reuse and recycling infrastructure, natural and clean environment, etc. are globally being the potential practices identified for manufacturing organizations in particular and cities, in general, aiming to enhance sustainability. However, before prioritizing and practicing these concepts at the local level, a robust scientific understanding about their economic and environmental benefits must be there.

### **5 Conclusions and recommendations**

This study presents an overall picture of the current and future water quality in the wet and dry seasons of the Hau River using the scenario-based analysis to identify key drivers responsible for water pollution and to aware the local policymakers. It is evident that BOD, TC, and  $PO_4^{3-}$  concentrations in 2018 are high in the river water, especially in the dry period, as compared to the desired level in class A of NTRSWQ. Under the BAU scenario with the anticipated population growth, rapid industrialization, and rainfall change, the river water quality will be further deteriorated, especially in the dry season and in downstream areas. Noticeably, industrial development is considered the main driver compared to the other ones, i.e., population growth and climate change. In the scenario with measures, the implementation of WWTPs in the residential and industrial areas can potentially deal with the water pollution problem in the wet season in the Hau River by 2030. However, water quality, especially BOD and  $PO<sub>4</sub><sup>3-</sup>$  pollution, will still be a matter of concern when looking into the downstream water quality in the dry period. In order to combat this issue, the additional combination of RWTPs for treating river water at a number of key locations of pollution was successfully applied. However, given the relatively high investment costs for the application of water treatment plants, the cities' governments should consider and propose more other solutions to the future water pollution in the Hau River. Besides, the monitoring and assessing programs on non-point pollution sources in the study area should be also developed with the aim of achieving a more comprehensive assessment of future river water pollution.

### <span id="page-30-1"></span>**Appendix 1**

The study area's schematic map (river network, catchments, water demand sites, STEP: WWTPs' and RWTPs' sites, monitoring stations, rainfall runoff and wastewater discharging direction, etc.) development in WEAP interface using ArcGIS tool STEP<sub>2</sub> Required data (river cross sections and streamflow, water quality, hydrometeorological parameters, WWTPs and RWTPs, etc.) input for model set up Hydrological (runoff, streamflow, etc.) and river water quality (BOD, TC, NO<sub>3</sub>. STEP<sub>3</sub> and  $PO<sub>4</sub><sup>3</sup>$ ) simulations utilizing the Rainfall Runoff Method (simplified

coefficient method) and Streetere-Phelps Model, respectively ┙┚

- Model calibration and validation by comparing between simulated and observed STEP 4 streamflow and river water quality parameters (BOD, TC,  $NO_3^-$  and  $PO_4^{3-}$ )
- Measures' effectiveness simulation and evaluation in the future scenarios **FEP** (Business As Usual and With measures), thereby proposing optimal measures for the cities' water quality management

<span id="page-30-0"></span>**Fig. 13** Flowchart for the research methodology

![](_page_30_Figure_8.jpeg)

<span id="page-30-2"></span>**Fig. 14** The study area's comparative water demands of residents and industry between 2018 and 2030

![](_page_31_Picture_616.jpeg)

**13 Appendix 2**<br>Sprimger the treatment characteristics of existing and new WWTPs and RWTPs located in the study area<br>Table 6 The treatment characteristics of existing and new WWTPs and RWTPs located in the study area  $\overline{a}$ ı

<span id="page-31-0"></span>**Appendix 2**

<span id="page-31-1"></span>**Appendix 2** 

![](_page_32_Picture_511.jpeg)

<span id="page-33-9"></span>![](_page_33_Picture_257.jpeg)

<span id="page-33-10"></span>**Table 8** The implementation progress of WRM projects in the study area in recent years

Project	<b>Quantity</b>	Capacity (m <sup>3</sup> /day)/ technology	Cost (million) USD)	Planned year	year	Targeted Finished Delayed year	gap	Main reasons
Domestic <b>WWTP</b>		30/UASB-TF	21.03	2003	2006	2017	11 years	Financial shortage, changes in tech- nology, policy barriers, etc
Industrial <b>WWTPs</b>		70/UASB-TF	56.79	2008	2010	2016	6 years	

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