

Efect of CSH‑PCE nanocomposites on early hydration of the ternary binder containing Portland cement, limestone, and calcined coal gangue

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Abstract

In this work, the impact of lab synthetic addition agent, CSH-PCE nanocomposites (CPNs), on the early hydration property of the ternary binder containing Portland cement, limestone, and calcined coal gangue was investigated. CPNs were added in partial substitution of Portland cement by mass at 0%, 0.5%, 1.0%, 1.5%, 2.0%, 2.5% and 3.0%. X-ray difraction (XRD), isothermal calorimetry, mercury intrusion porosimetry, and scanning electron microscopy were used to characterize the hydration and hydrates of the CPNs-modifed pastes systematically. The workability and compressive strength of this ternary system was also studied. The obtained results indicated that the use of CPNs continuously improved the workability of the ternary mortar. The compressive strength of the ternary mortar increased with CPNs additions until the threshold limits of 3.0% and 2.5% before and after 12 h, under which the strength values were even higher than the reference OPC mortar at each age. Isothermal calorimetry results indicated that CPNs promoted cement hydration and produced more hydrates, which were also verifed by the qualitative XRD analysis. This promotion efect leads to signifcant reduction in porosity as well as densifcation in microstructure within the ternary paste, ultimately resulting in enhanced early-age compressive strength. These fndings provide valuable insights for designing lower carbon footprint ternary blends incorporating calcined coal gangue and limestone while maintaining comparable early-age compressive strength to traditional cement.

Keywords CSH · Nanocomposites · Calcined coal gangue · Hydration · Strength

Introduction

It is well known that during the cement production, a large volume of natural resources needs to be consumed and vast amounts of greenhouse gas is emitted as well during its manufacture. Extensive research indicates that the utilization of supplementary cementitious materials (SCMs) represents one of the most efective strategies for mitigating the environmental impacts associated with the cement industry, thereby facilitating the transition toward a resource-efficient

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and low-carbon economy [\[1](#page-9-0), [2\]](#page-9-1). Furthermore, these materials greatly enhance the strength and durability of cementbased materials, contributing to their overall improvement [[3,](#page-9-2) [4](#page-9-3)]. The Coal gangue (CG) is a kind of by-product generated from the process coal mining and washing, constituting approximately 10–20% of coal production [\[5](#page-9-4)]. Over the past decades, CG has been commonly disposed in landflls or as ponds, causing signifcant environmental issues. CG contains SiO_2 , Al_2O_3 , CaO and Fe₂O₃ as its main chemical components, making it a potential candidate for SCM [[6\]](#page-9-5). Recent research indicates that unlike other SCMs used directly in cement-based materials, CG exhibits excellent pozzolanic activity after being calcined within an optimal temperature range [[7](#page-9-6)]. The calcined CG, abbreviated as CCG, owns amorphous mineral, metakaolin (MK) [[8\]](#page-9-7).

At present, limestone calcined clay cement (LC^3) is popular studied. It is a type of low carbon cement consisting of 50% cement clinker, 5% gypsum, 15% limestone, and 30% calcined clay [\[9](#page-9-8), [10\]](#page-9-9). Like calcined clay, CCG also contains

MK, therefore, the studies about the binders containing limestone, CCG and cement are increasing [\[11–](#page-9-10)[13\]](#page-10-0). The utilization of 10–20% CCG alone leads to a slightly reduction in the compressive strength of cement mortar at both 7 and 28 d [[11](#page-9-10)]. However, when the same amount of cement is replaced, the co-utilization of CCG and limestone enhances the strength of cement mortar at both 7 and 28 d [\[11,](#page-9-10) [12](#page-9-11)]. It should be noted that the compressive strength of cement mortar signifcantly decreases with the increasing content of CCG. The reactivity of CCG in the blended system is limited. However, when CCG is combined with limestone, the interaction between alumina phase and limestone leads to enhanced formation of carboaluminates and refnement of the pore structure in cement-based materials.

The incorporation CCG and limestone in a synergistic manner positively infuences the strength development of cement, starting from 7 d. However, this ternary system exhibits relatively low early compressive strength. For instance, at 1 d, the compressive strength of cement paste mixed with 20% CCG and 10% limestone was reduced by 46.6% compared to the reference OPC [[11\]](#page-9-10). Similar fndings were also reported in the work of Li et al. [[12](#page-9-11)], where using the same replacement level resulted in a reduction in compressive strength by 37.6% and 16.4% at 1 d and 3 d, respectively. CCG is a slow reacting materials at early curing time, the decrease in the cement content cannot be balanced by the CCG reactions. In addition, the participation of limestone in the reactions is also limited during this period. Therefore, it is imperative to address these detrimental efects since the strength and durability characteristics of concrete during early curing time are crucial in construction applications such as precast concrete.

It has been widely reported that many types of nano-materials are adopted to enhance the initial properties of cement-based materials, such as nano-SiO₂ [[14](#page-10-1), [15](#page-10-2)], nano-TiO₂ [\[16,](#page-10-3) [17\]](#page-10-4), nano-CaCO₃ [[18,](#page-10-5) [19](#page-10-6)], carbon nanotube/nanofiber [\[20](#page-10-7)[–23\]](#page-10-8), nano graphene [\[24,](#page-10-9) [25](#page-10-10)] and calcium silicate hydrate (CSH) seeds. Of the above nano-materials, CSH has recently become a research hotspot [[26,](#page-10-11) [27\]](#page-10-12). CSH is one of the main hydrates from silicate minerals in Portland cement and it is also the primary phase contributing to the strength development of cement-based materials. When CSH seeding is applied, the early hydration of cement is greatly accelerated and compressive strength was also signifcantly enhanced [[28,](#page-10-13) [29\]](#page-10-14). Several studies have reported that CSH seeding nanocomposites prepared by combining CSH and comb-like polymers, such as polycarboxylate superplasticizer (PCE), is more efective for enhancing cement hydration than pure CSH. This is because this type of nanocomposites are able to alleviate the agglomeration of CSH particles. Kanchanason et al. [[30](#page-10-15)] investigated the infuence of 2 mass% CSH-PCE nanocomposites on the compressive strength of cement blended with 35 mass% slag or calcined clay. It was found that this system exhibited a signifcant increase in strength, with an enhancement of approximately 240% at 16 h and a range of 25–95% from 1 to 28 d. Sun et al. [\[31](#page-10-16)] reported that the optimum addition of CSH-PCE nanocomposites in the blended cement with 20 mass% of metakaolin was 3 mass%, under which the compressive strength of this system was increased approximately 36%, 21% and 14% at 1, 3 and 7 d, respectively. The authors ascribed the increase in the strength to the enhancement of CSH-PCE nanocomposites on cement hydration and metakaolin reaction.

In this study, the efect of synthetic CSH-PCE nanocomposites (CPNs) on early hydration properties of the binders containing Portland cement, limestone, and calcined coal gangue are explored. The fuidity of fresh mortars was initially assessed, followed by compressive strength testing up to 7 d. Subsequently, the kinetics of hydration, phase assemblages, and microstructure were comprehensively analyzed using isothermal calorimetry, X-ray difraction (XRD), scanning electron microscopy (SEM), and mercury intrusion porosimetry (MIP).

Materials and method

Material

A commercial CEM II-42.5 Portland cement (OPC) was produced from Fushun Cement Factory (Liaoning, China). Coal gangue was collected from Huainan Coal Mine (Anhui, China). Calcined coal gangue (CCG) was obtained by calcining coal gangue at 800 °C for 2 h. Limestone with a purity of 85% was sourced from Zaozhuang (Shandong, China). The chemical composition of these materials determined by X-ray fuorescence analysis is presented in Table [1.](#page-1-0) Their particle size distributions determined by laser particle size analyzer are depicted in Fig. [1](#page-2-0). Additionally, dihydrate gypsum purchased from Taitan Chemical Reagent Co., Ltd

Table 1 Chemical composition of cement, limestone, and calcined coal gangue (mass%)

Oxides	Cement	Limestone	Calcined coal gangue
SiO ₂	20.94	5.2	64.3
Al_2O_3	4.85	2.4	28.8
Fe ₂ O ₃	3.44	1.3	3.1
CaO	64.02	55.6	0.5
MgO	1.70	2.7	0.5
SO_3	1.88	0.2	0.4
R_2O	0.50	0.63	1.5
f-CaO	0.50		
L.O.I	1.88	31.6	

Fig. 1 Particle size distributions (PSD) of OPC, limestone, and CCG

Fig. 2 Bound water content in the CCG-hydrated sample at diferent curing ages

Fig. 3 TEM images of CPNs

(Shanghai, China) was utilized to adjust the sulfate content in the ternary binders containing CCG. In addition, $R³$ test [[32\]](#page-10-17) was also adopted to evaluate the pozzolanic reactivity of CCG, which was expressed by the bound water content up to 7 d and the result is shown in Fig. [2](#page-2-1). It can be seen that the bound water content increased obviously from 3 d, indicating the starting reaction of CCG with CH during this period, and this reaction was more intense with increasing the curing age.

The solid content of CPNs in the prepared solution is about 20% and the Ca/Si ratio in CPNs was 1.21. In addition, the Na content in CPNs could also be ignored since the excess Na has been fltered and washed away during the preparation process. The morphology of the sample was examined using transmission electron microscopy (TEM), as depicted in Fig. [3](#page-2-2). As observed, CPNs particles with nanoscale was successfully prepared, and their average particles sizes were within 20 nm.

Mixtures design

The detailed proportions of the mortar mixtures are presented in Table [2.](#page-3-0) In this experiment, the reference ternary binder was obtained by mixing 69.6 mass% of cement, 20 mass% of calcined coal gangue, 10 mass% of limestone and 0.4 mass% of gypsum. The dosage of CPNs, a suspension with 20% solid content, was set from 0 to 3.0 mass% of binder (binder = cement + LC + CCG + gypsum) with a step of 0.5 mass%.

Preparation of paste and mortar samples

The paste samples were prepared as follows: Raw materials were weighed according to Table [2](#page-3-0), followed by mixing at a low speed for 60 s and then at a high speed for 30 s using NJ-160B blender. The fresh mixtures were poured into plastic bottles with sizes of Φ 10 mm × 50 mm, then the pastes and bottles were manually vibrated for 10 s to compact the pastes. Finally, the bottles flled with pastes were tightly

Table 2 Mixture proportions of studied systems (100 g basis)

sealed with plastic cling flm and placed in a standard curing room (20 °C, humidity above 95%). At the designated curing time, the pastes were carefully extracted from the bottles, fragmented into small pieces, and subsequently immersed in ethanol to stop hydration. Following a 7 d immersion period, a portion of these fragments was fnely ground into powders passing through 75 µm sieve before being subjected to XRD analysis after drying for at least 48 h at 40 °C in a vacuum oven. The remaining fragments were directly dried under identical conditions for MIP and SEM analysis.

As for the mortar samples preparation, a fxed standard sand to binder ratio of 3 was used. All materials were weighed according to Table [2](#page-3-0) followed by mixing at a low speed for 120 s and high speed for 40 s using JJ-5 blender. After conducting the fuidity measurement, the fresh mortars were carefully poured into steel molds measuring $40 \text{ mm} \times 40 \text{ mm} \times 160 \text{ mm}$ and subsequently subjected to a controlled vibration of 60 cycles on the dedicated vibrating table. Subsequently, the mortar samples were put in the same curing environment as the paste samples until for the compressive strength test.

Methods

Fluidity measurement of mortar samples was conducted using the mini-slump flow testing conforming to the Chinese standard GB/T 8077-2012. The average of three tests was reported as the fnal fuidity.

The compressive strength of mortar samples was determined in accordance with the Chinese standard GB/T 8077- 2012, following a standardized procedure. The average of six tests was reported as the fnal compressive strength.

The hydration heat evolution of paste samples was monitored using a TAM Air 8 isothermal calorimeter. The experiment was conducted at a constant temperature of 20 °C, and the data were recorded for up to 24 h.

A Rigaku SmartLab 3000A X-ray difractometer with copper radiation $(\lambda = 0.154 \text{ nm})$ was used to analyze the

phase assemblages of samples with curing age. The instrument operated at an applied voltage of 45 kV and current of 15 mA, while data collection occurred from an angular range of 5° to 65° (2 θ) with a scanning rate of 5° /min.

Pore structures analysis in the samples was performed using a PoreMaster GT60 mercury intrusion porosimeter, which enabled probing pore sizes ranging from 0.0035 to $400 \mu m$.

Morphological observations on the samples were carried out using a Nova NanoSEM450 scanning electron microscope (SEM) equipped with a W tungsten flament operating at an accelerating voltage of 15 kV. Prior to testing, the samples were coated with a thin layer of gold for improved conductivity.

Results and discussion

Fluidity

Figure [4](#page-4-0) shows fuidity of the diferent mortars. As shown in Fig. [4,](#page-4-0) the fuidity of CCG20LS10 mortar was 10.4% lower than that of the reference OPC mortar. The reason for this is that CCG and limestone possess smaller particle sizes (Fig. [3,](#page-2-2) with a larger specific surface area), necessitating a greater amount of water to fully cover the surface of these particles. For the same amount of water, less free water could be acted as the lubrication layer and the friction between these particles was increased, resulting in a decreased fuidity [\[33,](#page-10-18) [34\]](#page-10-19).

It can be seen that adding CPNs efficiently increased the fuidity of CCG20LS10 mortar, especially at a higher amount of CPNs. For example, when the mixing amount of CPNs was 0.5%, 1.0%, 1.5%, 2.0%, 2.5% and 3.0%, the mortar fuidity increased approximately up to 161, 185, 266, 279, 289 and 293 mm, respectively, increased by 3.8%, 19.6%, 71.6%, 79.7%, 84.8% and 89.0% compared with CCG20LS10. This is associated with the superplasticizer introduced by CPNs that can reduce water demand while

Fig. 4 The effect of CPNs on fluidity of the CCG20LS10 mortar

Fig. 5 Compressive strength of the specimens at diferent curing ages

maintaining adequate workability [\[35](#page-10-20)]. This observation is also completely diferent from other types of nano-particles, where higher amount of nano-particles resulted in much worse fluidity than the control mortars [[24,](#page-10-9) [36](#page-10-21)].

Compressive strength

The compressive strength development for specimens at curing ages of 6 h, 12 h, 1 d, 3 d and 7 d is illustrated in Fig. [5.](#page-4-1) As displayed in Fig. [5](#page-4-1), the specimen of CCG20LS10 experienced a much lower compressive strength than the reference OPC before 3 d. This is due to the much lower reactivity of CCG and limestone than that of cement at early ages, which resulted in precipitating less hydrates and thereby the slower progress in the compressive strength development. Such decrease in the compressive strength was less pronounced with increasing age. At the age of 7 d, the compressive strength of CCG20LS10 specimen was even about 1.3% higher than the reference OPC. Such a fast strength gain for CCG20LS10 specimen was associated with the starting reaction of CCG that produced more hydrates such as C–A–S–H and carboaluminates that greatly enhanced the corresponding compressive strength.

As expected, CPNs addition exhibited a directly positive efect on the compressive strength of CCG20LS10. The compressive strength of CCG20LS10 with CPNs was observed to increase as the content of CPNs increased before 12 h. The highest increase is observed in C3.0, where a signifcant enhancement in compressive strength is observed at 6 and 12 h, respectively, compared to CCG20LS10. The increase in the compressive strength due to the incorporation of CPNs can be attributed to their seeding efect, which greatly promotes cement hydration and facilitates the production of additional hydrates. In addition, CPNs particles possibly acted as fllers to fll the space in specimens, resulting in denser microstructures.

Similarly, compared with CCG20LS10, the increase in the compressive strength at 1, 3 and 7 d was observed to be 9.3–63.6%, 6.9–21.9% and 1.5–6.6% for the specimens with adding ranging from 0.5 to 3.0 mass%, respectively. It was observed from Fig. [4](#page-4-0) that the maximum compressive strength of the specimen was obtained with CPNs at an amount of 2.5 mass% from 1 d onwards. The compressive strength, however, exhibited a decline upon further increase in the amount of CPNs up to 3.0 mass%, suggesting that an excessive presence of CPNs did not contribute favorably to the development of compressive strength in CCG20LS10.

Isothermal calorimetry

Figure [6](#page-5-0) shows the isothermal calorimetry results of selected pastes up to 24 h. As shown in Fig. [6a](#page-5-0), four hydration stages including initial reactions, induction period, acceleration period and deceleration period can be clearly seen in all pastes. Compared with CCG20LS10, the addition of CPNs shortened the induction period, which decreased continuously from around 2.9 to 1.8 h when CPNs additions increased from 0 to 3.0%. The acceleration period exhibits an exothermic peak, which is attributed to the hydration of Alite and the precipitation of CSH and calcium hydroxide (CH) [\[37\]](#page-10-22). It is visible that CPNs fastened the appearance of this peak and increased the corresponding peak intensity. For example, the hydration peak in the acceleration period occurred at around 13.2 h and its intensity was 0.0022 W g^{-1} . When 1%, 2% and 3% of CPNs was added, the occurrence of alite hydration peak shifted to approximately 5.4 h and the peak intensities increased up to 0.00265, 0.0034 and 0.00395

Fig. 6 Heat fow (**a**) and cumulative heat released (**b**) of the selected pastes normalized by the mass of selected pastes

 $W g^{-1}$, respectively. This acceleration is associated with the nucleation sites efect of CPNs that enhances cement hydration, especially at higher CPNs additions.

Figure [6b](#page-5-0) shows the measured cumulative heat released of the pastes. The total hydration heat of CCG20LS10 at 6, 12, and 24 h exhibited a reduction of 16.0%, 20.0%, and 18.8% compared to the reference OPC. This is because CCG and limestone exhibit much lower reactivity at early ages than OPC, and the heat generated from the reactions relating to these two minerals are far from balancing the decreased hydration heat resulting from the decreased cement content. When CPNs was added, the total heat released of CCG20LS10 increased significantly. As shown in Fig. [6b](#page-5-0), the total heat released of CCG20LS10 with 1%, 2% and 3% of CPNs increased by 207.3%, 289.9% and 368.7% at 6 h, 72.6%, 110.3% and 135.3% at 12 h, 9.4%, 45.1% and 59.2% at 24 h, respectively. It should be pointed out that CCG20LS10 with CPNs achieved even higher total hydration heat than that of the reference OPC (except C1.0 at 24 h). This observation was also well consistent with the compressive strength results.

X‑ray difraction analysis

Figure [7](#page-6-0) displays the XRD patterns of CCG20LS10 samples with and without CPNs after diferent curing ages of hydration. In addition, XRD pattern of OPC is also given for better comparison. As displayed in Fig. [7,](#page-6-0) ettringite and CH respectively based on the peaks at 9.1° and 18.0° are the main crystalline hydrates of cement at all curing ages. In the samples of CCG20LS10 with or without CPNs, in addition to ettringite and CH, only the crystalline hydrate of hemicarboaluminate was formed (Fig. [7](#page-6-0)d–e).

It is well known that CH is precipitated from dicalcium silicate (C_2S) and tricalcium silicate (C_3S) hydration. In the ternary system with CCG and limestone, the pozzolanic reactivity of CCG and the synergistic efect between CCG and limestone would react with CH to precipitate C–A–S–H and carboaluminates (Hc in Fig. [5\)](#page-4-1), respectively. Therefore, the CH, overlapped C_2S and C_3S , and Hc peaks respectively centered at 18.0°, 32.5° and 11.7° were selected for further comparison. The overlapped C_2S and C_3S peak intensity for the paste with CPNs was obviously lower than that of the CCG20LS10 paste, especially at higher CPNs addition, indicating that the addition of CPNs enhanced cement hydration. Meanwhile, compared with CCG20LS10, the intensity of CH peak in C1.0 at 6 and 12 h, C2.0 at 6 h was increased accordingly. However, in other cases, the intensity of this peak in CPNs-added pastes was decreased with increasing the amount of CPNs. This is possibly due to the enhanced reaction of CCG that consumed more CH. In addition, the Hc peak started to appear in CPNs-added CCG20LS10 sample from 3 d, indicating the addition of CPNs also enhanced the synergistic efect between CCG and limestone that precipitated more Hc. The qualitative XRD analysis revealed that the incorporation of CPNs not only enhanced cement hydration but also facilitated the reaction of CCG in CCG20LS10. More hydrates would produced in this process, which well supported the higher compressive strength development of the CPNs-added CCG20LS10 mortar.

Pore structure analysis

Figure [8](#page-7-0) shows the porosity of OPC, CCG20LS10 samples with 0% and 2.5% of CPNs after different curing ages of

Fig. 7 XRD patterns of hydrated cement pastes at 6 h (**a**), 12 h (**b**), 1 d (**c**), 3 d (**d**) and 7 d (**e**)

Fig. 8 Total porosity of selected systems at 12 h (**a**), 1 d (**b**), 3 d (**c**) and 7 d (**d**)

hydration. As the curing age increased, the porosity of each system decreased due to the continuous reactions, including cement hydration and the reaction of CCG.

The replacement of cement by CCG and limestone led to an increased porosity at all curing ages, as depicted in Fig. [8.](#page-7-0) For instance, compared to the reference OPC, the system of CCG20LS10 exhibited approximately 24.4%, 20.1%, 27.4%, and 27.9% increases in porosity at 12 h, 1 d, 3 d, and 7 d, respectively. This can be attributed to a higher water-to-cement ratio and lower gel-to-space ratio when cement was partially substituted with CCG and limestone, which is analogous to the scenario where fy ash was incorporated for partial cement replacement.

The addition of CPNs greatly improved the pore structure reflected by the significant decrease in the porosity at effect of CPNs that accelerated cement hydration, which produced more hydrates and efficiently decreased the porosity of CCG20LS10. In addition, the particle sizes of CPNs is much finer than that of cement, CCG and limestone particles, which can fill the pores and helped to decrease the porosity. It should be pointed out that as for the CPNs added system, it also exhibited about 29.6%, 41.4%, 55.2% and 57.1% decrease of porosity than the reference OPC at 12 h, 1 d, 3 d and 7 d, respectively. This observation also well explains the compressive strength results, where the mortar with adding 2.5% CPNs yielded significant higher compressive strength than both mortars of CCG20LS10 and OPC at each curing age.

each curing age. This is likely associated with the seeding

Morphology observation

Morphology observation was performed on the samples of OPC, CCG20LS10 and C3.0 at 1 (Fig. [9\)](#page-8-0) and 7 d (Fig. [10](#page-8-1)). It can be clearly seen that the typical hydrates such as CSH gel, AFt and CH were precipitated in hydrated OPC at 1 d (Fig. [9](#page-8-0)a), which is well consistent with the XRD observations (Fig. [7\)](#page-6-0). In addition, many pores left by the free water were also observed. As for the CCG20LS10 paste, hydrates were easier to be observed (Fig. [9](#page-8-0)b), being associated with the fller efect of CCG and limestone that promoted cement hydration. However, its microstructure was less denser compared with OPC, which was refected by more pores and disconnection of hydrates (Fig. [9](#page-8-0)e). This is

Fig. 9 SEM images of the selected samples at 1 d

Fig. 10 SEM images of the selected samples at 7 d

due to the much lower reactivity of CCG and limestone than that of cement at early stage. On the contrary, the microstructure of C3.0 was dense and contained a few pores and fne hydrates, mainly due to the seeding efect of CPNs that has great contribution of early-age cement hydration. Understandably, C3.0 displays comparable strength to OPC but much higher than CCG20LS10 (Fig. [5\)](#page-4-1).

After 7 d of hydration (Fig. [10\)](#page-8-1), the microstructure of each system became denser due to the continuous reactions with forming more hydrates. The pozzolanic reaction of CCG and the synergistic interaction between CCG and limestone were found to be highly signifcant during this period, thereby facilitating the microstructural development of CCG20LS10 paste. This well explains the similar microstructure between OPC and CCG20LS10. From Fig. [10](#page-8-1)f, the hydrates in C3.0 were tightly connected with few pores, contributing to a much denser microstructure in comparison with OPC and CCG20LS10, evidenced by the MIP results in Fig. [8.](#page-7-0) This is associated with the acceleration efect of CPNs on the reactions in the sample of CCG20LS10 that produced more hydrates and compacted the microstructure, which well explains the highest compressive of C3.0 achieved at 7 d (Fig. [5](#page-4-1)).

Conclusions

The present study investigates the impact of synthetic CSH-PCE nanocomposites on the early hydration process of ternary binders comprising Portland cement, limestone, and calcined coal gangue. The fuidity, compressive strength, hydration heat, phase assemblages, and microstructure changes of the blends containing CSH-PCE nanocomposites were evaluated. Based on the aforementioned experimental fndings, the following conclusions can be drawn:

- 1. The workability of the ternary mortar continuously increased with increasing the CPNs content up to 3.0%, which is due to the superplasticizer introduced by CPNs that can reduce water demand while maintaining adequate workability.
- 2. The incorporation of CPNs signifcantly enhanced the compressive strength of the ternary mortar at all curing ages. The highest compressive strength of the ternary mortar was achieved with a 3.0% dosage of CPNs at 6 and 12 h, and with a 2.5% dosage at 1, 3, and 7 d. Remarkably, even compared to the reference OPC mortar, the compressive strength of the ternary mortar surpassed it from as early as 6 h up to 7 d.
- 3. Isothermal calorimetry and qualitative XRD analysis revealed that the addition of CPNs not only enhanced cement hydration, but also facilitated the reaction of CCG. resulting in increased formation of hydrates in the

ternary paste. These hydrates contributed to a signifcant reduction of porosity and densifcation of microstructure (evidenced by MIP and SEM observations), well supporting the compressive strength development.

Author contributions Haoran Zhai: Conceptualization; Ying Liu, Qinghui Yang, Yuantao Wang, Shufeng Liu, Yuanyuan Huang, Delu Zou, Xueyan Fan: Investigation; Yongling Ding: Writing.

Declarations

Conflict of interest The authors declare that they have no known competing fnancial interests or personal relationships that could have appeared to infuence the work reported in this paper.

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