



# A novel Harris hawks' optimization and k-fold cross-validation predicting slope stability

Hossein Moayedi<sup>1,2</sup> · Abdolreza Osouli<sup>3</sup> · Hoang Nguyen<sup>4</sup> · Ahmad Safuan A. Rashid<sup>5</sup>

Received: 29 June 2019 / Accepted: 16 July 2019 / Published online: 30 July 2019  
© Springer-Verlag London Ltd., part of Springer Nature 2019

## Abstract

Stability of the soil slopes is one of the most challenging issues in civil engineering projects. Due to the complexity and non-linearity of this threat, utilizing simple predictive models does not satisfy the required accuracy in analysing the stability of the slopes. Hence, the main objective of this study is to introduce a novel metaheuristic optimization namely Harris hawks' optimization (HHO) for enhancing the accuracy of the conventional multilayer perceptron technique in predicting the factor of safety in the presence of rigid foundations. In this way, four slope stability conditioning factors, namely slope angle, the position of the rigid foundation, the strength of the soil, and applied surcharge are considered. Remarkably, the main contribution of this algorithm to the problem of slope stability lies in adjusting the computational weights of these conditioning factors. The results showed that using the HHO increases the prediction accuracy of the ANN for analysing slopes with unseen conditions. In this regard, it led to reducing the root mean square error and mean absolute error criteria by 20.47% and 26.97%, respectively. Moreover, the correlation between the actual values of the safety factor and the outputs of the HHO–ANN ( $R^2 = 0.9253$ ) was more significant than the ANN ( $R^2 = 0.8220$ ). Finally, an HHO-based predictive formula is also presented to be used for similar applications.

**Keywords** Metaheuristic algorithms · Harris hawks' optimization · Artificial intelligence · Stability performance

## 1 Introduction

Local slopes can highly affect adjacent engineering studies. In most civil engineering projects, the stability of the local slopes has been considered as a significant problem. Also, slope failures can cause various psychological damages, including property loss as well as human life in our world.

For instance, Iranian Landslide Working Party (2007) reported that 187 people were killed due to the destructive effects of slope failure [1]. The impregnation level, along with different intrinsic characteristics of the soil, can impact the slope failure likelihood [2, 3]. Various studies have been conducted to propose impressive modelling for slope stability issue. Traditional methods have many shortcomings, such as the requirement of utilizing laboratory equipment and also high complexity barricade them from being an appropriate solution [4]. Nevertheless, because of their constraint in studying a particular slope state, for example, soil properties, height, slope angle, groundwater level, etc., these solutions have not commonly been considered as a general solution. Various sorts of numerical solutions, finite element model (FEM), and limit equilibrium methods (LEMs) are extensively chosen for the slope stability issue [5–7]. For providing a trustworthy method for slope stability study, scholars have focused on the expansion of design charts [8]. However, this method also has some defects. Producing an impressive design chart needs a lot of time and cost. Also, indicating the accurate mechanical factors is a problematic duty [9, 10]. Hence, because of

✉ Hossein Moayedi  
hossein.moayedi@tdtu.edu.vn

<sup>1</sup> Department for Management of Science and Technology Development, Ton Duc Thang University, Ho Chi Minh City, Vietnam

<sup>2</sup> Faculty of Civil Engineering, Ton Duc Thang University, Ho Chi Minh City, Vietnam

<sup>3</sup> Southern Illinois University Edwardsville, Edwardsville, IL 62026, USA

<sup>4</sup> Institute of Research and Development, Duy Tan University, Da Nang 550000, Vietnam

<sup>5</sup> Centre of Tropical Geoenvironment (Geotropik), School of Civil Engineering, Faculty of Engineering, Universiti Teknologi Malaysia, Johor Bahru, Malaysia

efficiency, design charts commonly accompanied high precision, therefore the usage of artificial intelligence methods is more bolded [11, 12]. These methods can specify the non-linear relationship between the target factors as well as its key parameters, and this is an outstanding advantage of these approaches. Artificial neural network (ANN) commonly uses any determined number of hidden nodes [13, 14]. In geotechnical studies, different scholars stated that machine learning approaches such as support vector machine (SVM) and ANNs have proper efficiency [15–19]. The intricacy of the slope stability issue is obvious. What makes the problem even more complex and critical is creating different buildings in the presence of slopes that are showing a considerable value of loads used on a rigid footing. It is known that the interval of the slope's crest along with the value of surcharge is considered as two factors that can affect the stability of the target slope [20]. Because of this fact, scholars have motivated to show a relationship to compute the factor of safety of pure slopes and sometimes the slopes taking a static load [21–25]. Chakraborty and Goswami [26] predicted the factor of safety for around 200 slopes to distinct geometric and shear strength factors by taking into account the multiple linear regression (MLR) along with ANN algorithms. In their work, a comparison study has been conducted to compare calculated results to a FEM model. They have obtained a proper rate of precision obtained for both practical models. In addition, they found that ANN had better performance compared to MLR. Lie et al. [27] utilized the random forest (RF) along with regression tree in functional soil–landscape simulations to regionalize the depth of the failure level and density of soil bulk. Although looking for more reliable analysis of the stability of the slopes various hybrid evolutionary algorithms has been successfully employed in plenty of studies [28–31], this study presents a novel optimization technique named Harris hawks' optimization (HHO) incorporated with ANN to give a reliable approximation of the stability of soil slopes. Notably, the HHO is a recently proposed natural inspired metaheuristic algorithm, and the authors did not come across any previous study which applied this algorithm to the mentioned subject.

## 2 Methodology

### 2.1 Artificial neural network

The artificial neural network (ANN) is based on the interaction among the neurons in the biological neural apparatus. McCulloch and Pitts [32] proposed ANN for the first one. The algorithm of ANNs is generally utilized as approximators in a non-linear survey of input–output data [33–38]. These methods are commonly used for different

engineering issues because of their specific mathematical solution in optimization tasks [22, 39–47]. Basically, the ANN algorithm includes a group of computational relationships that are commonly worked with each other. Multilayer perceptron (MLP) is known as one of the most appropriate methods between different algorithms of ANNs that used for classification as well as regression issues. The whole structure of the MLP algorithm is presented in Fig. 1. As can be observed, this model consists of three different types of layers. In this method, the number of hidden layers usually changes; however, it just can have input and output layers. Scholars determined that MLPs possess one hidden layer in terms of efficiency [48].

The MLP is basically employed for detecting the mathematical relations among different factors with the taking into account of one and even more activation function(s). We consider  $W_1$  and  $W_2$  as the weight matrices layers in the hidden and output sections, respectively. After that, the mentioned method is adjusted as below:

$$f(X) = b_2 + W_2 \times (f_A(b_1 + W_1 \times X)), \quad (1)$$

where  $f_A$  stands for the activation function.  $b_1$  and  $b_2$  stand for the bias matrices related to the neurons located in the hidden and output layers, respectively.

### 2.2 Harris hawks' optimization algorithm

The algorithm of Harris hawks' optimization (HHO) is inspired using the cooperative treatment along with the chasing manner of Harris' hawks that is first expanded by Heidari et al. [49]. This algorithm has been successfully used for various scientific applications [50, 51]. Hawks attempting to surprise their prey and from different paths swooped on them, cooperatively. In addition, Harris hawks have the ability to choose chase type according to the distinct patterns of prey flight. It has three base stages in HHO, including

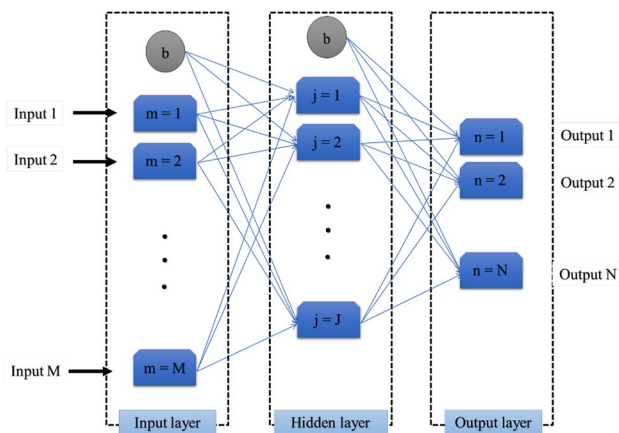


Fig. 1 The structure of an MLP neural network with one hidden layer

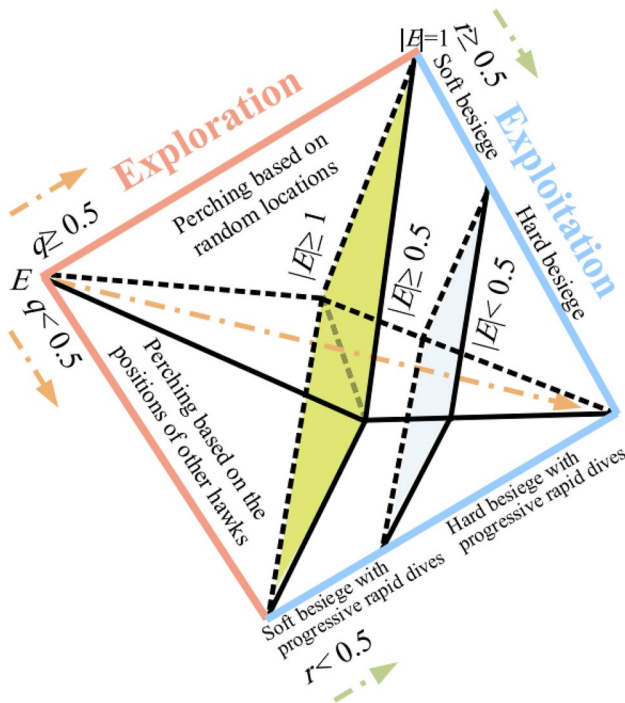


Fig. 2 Different phases of Harris hawks’ optimization (HHO) (after Heidari et al. [49])

amaze pounce, tracking the prey, and other different sorts of attacking strategies. Different phases of Harris hawks’ optimization (HHO) are shown in Fig. 2. The pseudo-code of HHO algorithm is also illustrated in Table 1. In a glance, the first stage is named “Exploration” and is modelled to mathematically wait, search, and discover the desired hunt. The second stage of this algorithm is transforming from exploration to exploitation, based on the external energy of a rabbit. Finally, in the third phase which is called “Exploitation”, considering the residual energy of the prey, hawks commonly take a soft and sometimes hard surround for hunting the rabbit from different directions.

2.2.1 Exploration

In each step, Harris’ hawks have been considered the best solutions. The iter + 1 (the Harris hawks’ position) is mathematically modelled by the following relation:

$$X(\text{iter} + 1) = \begin{cases} X_{\text{rand}}(\text{iter}) - r_1 |X_{\text{rand}}(\text{iter}) - 2r_2 X(\text{iter}) & \text{if } q \geq 0.5 \\ (X_{\text{rabit}}(\text{iter}) - X_m(\text{iter})) - r_3 (LB + r_4(UB - LB)) & \text{if } q < 0.5 \end{cases} \tag{2}$$

where iter means the present iteration,  $X_{\text{rand}}$  stands selected for hawk at the available population,  $r_i, i = 1, 2, 3, 4, \dots, q$  are random numbers that are between 0 and 1,  $X_{\text{rabit}}$  stands for

Table 1 Pseudo-code of the HHO algorithm (after Heidari et al. [49])

```

Inputs: The population size  $N$  and maximum number of iterations  $T$ 
Outputs: The location of rabbit and its fitness value
Initialize the random population  $X_i(i = 1, 2, \dots, N)$ 
while (stopping condition is not met) do
  Calculate the fitness values of hawks
  Set  $X_{\text{rabit}}$  as the location of rabbit (best location)
  for (each hawk ( $X_i$ )) do
    Update the initial energy  $E_0$  and jump strength  $J$ 
     $E_0 = 2 \cdot \text{rand}() - 1, J = 2(1 - \text{rand}())$ 
    Update the  $E$  using Eq. (4)
    if ( $|E| \geq 1$ ) then  $\triangleright$  Exploration phase
      Update the location vector using Eq. (2)
    if ( $|E| < 1$ ) then  $\triangleright$  Exploitation phase
      if ( $r \geq 0.5$  and  $|E| \geq 0.5$ ) then  $\triangleright$  Soft besiege
        Update the location vector using Eq. (5)
      else if ( $r \geq 0.5$  and  $|E| < 0.5$ ) then  $\triangleright$  Hard besiege
        Update the location vector using Eq. (7)
      else if ( $r < 0.5$  and  $|E| \geq 0.5$ ) then  $\triangleright$  Soft besiege with progressive rapid dives
        Update the location vector using Eq. (11)
      else if ( $r < 0.5$  and  $|E| < 0.5$ ) then  $\triangleright$  Hard besiege with progressive rapid dives
        Update the location vector using Eq. (12)
  Return  $X_{\text{rabit}}$ 
    
```

the rabbit position, and  $X_m$  is the mean position for hawks and that is computed as follows:

$$X_m(\text{iter}) = \frac{1}{N} \sum_{i=1}^N X_i(\text{iter}), \tag{3}$$

where  $X_i$  shows the every hawk place and  $N$  stands for the hawks size.

2.2.2 Transition from exploration to exploitation

The rabbit energy may be calculated by the below relation:

$$E = 2E_0 \left( 1 - \frac{\text{iter}}{T} \right), \tag{4}$$

where  $E$  is the external energy from rabbit and  $T$  stands for the maximum size about the iterations. In this relation,  $E$  stands for the energy of the rabbit, and  $E_0 \in (-1.1)$  shows the inlet energy for each step. HHO may determine the rabbit state based on the variation trend of  $E_0$ .

2.2.3 Exploitation

In this stage, for successful escape of the prey: if  $r < 0.5$ . If  $|E| \geq 0.5$  HHO takes soft surround and if  $|E| < 0.5$  the

HHO takes hard surround. To model the attacking stage, the algorithm of HHO used four distinct methods based on the escaping approaches of the prey as well as pursuing approaches of the Harris’ hawks: hard and soft surrounds, advanced rapid dives while soft surround, progressive rapid dives while hard surround. Particularly,  $|E| \geq 0.5$  means that the prey has enough energy for running out from the surround. Therefore, whether the rabbit runs out from the surround or not is based on two values of  $r$  and  $E$ .

**A**—soft surround:  $r \geq \frac{1}{2}$  and  $|E| \geq \frac{1}{2}$ .

We can use the following relation:

$$X(\text{iter} + 1) = \Delta X(\text{iter}) - E|JX_{\text{rabbit}}(\text{iter}) - X(\text{iter})|, \tag{5}$$

$$\Delta X(\text{iter}) = X_{\text{rabbit}}(\text{iter}) - X(\text{iter}), \tag{6}$$

where  $\Delta X$  stands for deference among the position vector of the prey,  $J = 2(I - r_s)$  stands for jump severity of the prey in the stage of escaping and  $r_s \in (0,1)$  shows a random number.

**B**—hard surround:  $r \geq \frac{1}{2}$  and  $|E| < \frac{1}{2}$ .

We can use the following formula for showing the present positions:

$$X(\text{iter} + 1) = X_{\text{rabbit}}(\text{iter}) - E|\Delta X(\text{iter})|. \tag{7}$$

**C**—advanced rapid dives while soft surround:  $r < \frac{1}{2}$  and  $|E| \geq \frac{1}{2}$ .

As stated for soft surround, previously, hawks find the next purpose using the below relation:

$$Y = X_{\text{rabbit}}(\text{iter}) - E|JX_{\text{rabbit}}(\text{iter}) - X(\text{iter})|. \tag{8}$$

The hawks can dive as the below relation:

$$Z = Y + S \times \text{LF}(D), \tag{9}$$

where  $D$  stands for the issue dimension and  $S_{1 \times D}$  shows a random vector along with the levy flight. We can calculate LF as follows:

$$\text{LF}(D) = 0.01 \times \frac{\mu \times \sigma}{|\vartheta|^{\frac{1}{\beta}}}, \sigma = \left( \frac{\Gamma(1 + \beta) \times \sin\left(\frac{\pi\beta}{2}\right)}{\Gamma\left(\frac{1+\beta}{2}\right) \times \beta \times 2^{\left(\frac{\beta-1}{2}\right)}} \right) \cdot \beta = 1.5, \tag{10}$$

where  $\mu$  and  $\vartheta$  stand for random amounts among in the range of 0–1. Hence, for updating the hawks’ locations, the final approach can be shown as follows:

$$X(\text{iter} + 1) = \begin{cases} Y & \text{if } F(Y) < F(X(\text{iter})) \\ Z & \text{if } F(Z) < F(X(\text{iter})) \end{cases} \tag{11}$$

**D**—advanced rapid dives while hard surround.

$$r < \frac{1}{2} \text{ and } |E| < \frac{1}{2}.$$

In the present paper, the hawks were considered being near the rabbit. The behaviour of them can be modelled as follows:

$$X(\text{iter} + 1) = \begin{cases} Y & \text{if } F(Y) < F(X(\text{iter})) \\ Z & \text{if } F(Z) < F(X(\text{iter})) \end{cases} \tag{12}$$

$Y$  and  $Z$  should be calculated as follows:

$$Y = X_{\text{rabbit}}(\text{iter}) - E|JX_{\text{rabbit}}(\text{iter}) - X(\text{iter})|, \tag{13}$$

$$Y = X_{\text{rabbit}}(\text{iter}) - E|JX_{\text{rabbit}}(\text{iter}) - X(\text{iter})|, \tag{14}$$

$$Z = Y + S \times \text{LF}(D), \tag{15}$$

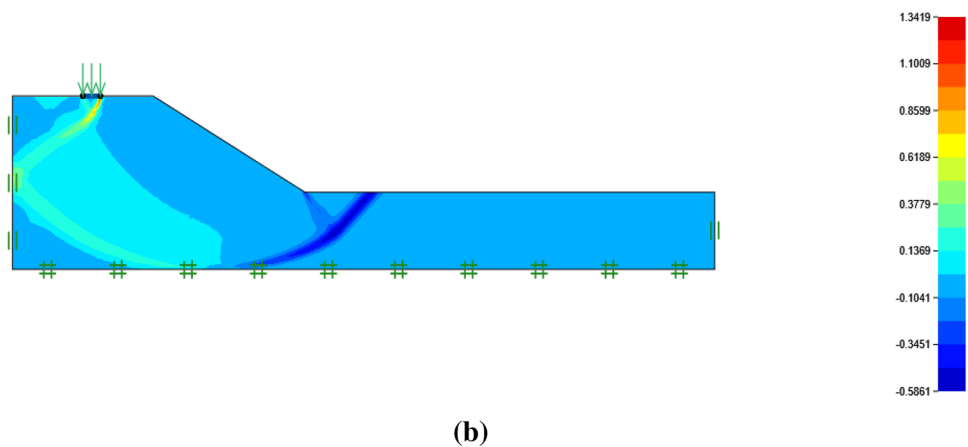
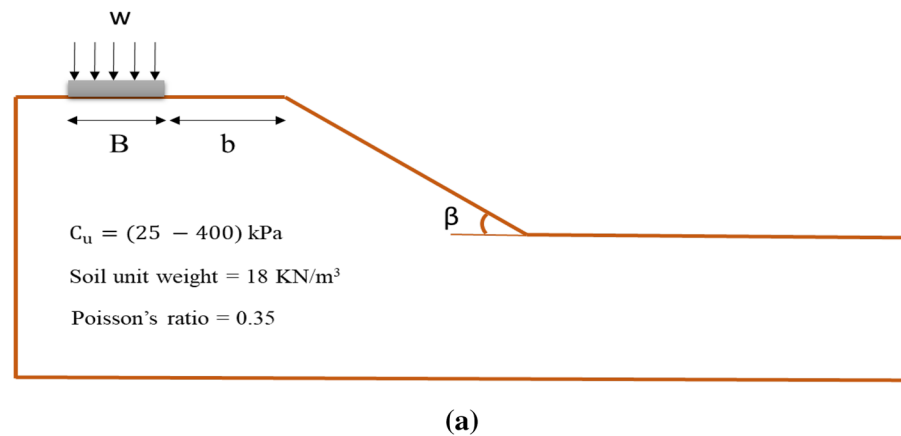
in which  $X_m(\text{iter})$  shows  $\frac{1}{N} \sum_{i=1}^N X_i(\text{iter})$  [52].

### 3 Data collection and methodology

We can use a single-layer slope to obtain a reliable database. In this method, we should assume a purely cohesive soil, having only undrained cohesive strength ( $C_u$ ), creates the body of this slope. The basic parameters that can have some influences on the strength of the slope versus the failure (i.e., the factor of safety) are the magnitude of the surcharge on the footing enched onto the slope ( $w$ ), setback distance ratio ( $b/B$ ), and slope angle ( $\beta$ ). Figure 3a shows these parameters. In this study, the Optum G<sub>2</sub> software was used for computing the factor of safety. In most cases, the safety factor is a typical method to show the geotechnical stability as well as deformation in slopes [53] (see Fig. 3b). In this regard, various geometries of the slope angle ( $\beta$ ) along with different rigid foundation ( $b/B$ ) (i.e., around 630 possible cases) are drawn and then evaluated in Optum G2 for calculating the factor of safety. Other parameters were also considered into simulation including cohesive strength of the soil ( $C_u$ ) and applied surcharge ( $w$ ). The mechanical factors, including the ratio of Poisson, internal friction angle, and soil unit weight, were specified 0.35, 0°, and 18 kN/m<sup>3</sup>, respectively. Moreover, modulus of Young ( $E$ ) differed for every amount of  $C_u$ . It is adjusted to be 1000, 2000, 3500, 5000, 9000, 15,000 and 30,000 kPa for amount of  $C_u$  25, 50, 75, 100, 200, 300 and 400 kPa, respectively.

An example of the utilized dataset is shown in Table 2. In this table, we illustrated the examples of the relation between the slope safety factor and its effective parameters. As can be observed, when  $C_u$  has high value, the slope ensures more stability. The factors of  $\beta$  (5°, 30°, 45°, 60°, and 75°), as well as  $w$  (50, 100, and 150 KN/m<sup>2</sup>), have been considered as adversely proportionate for the FOS. By increasing the values of  $\beta$  and  $w$ , the slope is more likely to be failing. The factor of safety cannot illustrate any considerable sensibility to the  $b/B$  ratio variations (0, 1, 2, 3, 4, and 5). Also, Table 2 shows that the safety factor does not specify any considerable sensitivity for the  $b/B$  ratio variations of 0, 1, 2, 3, 4, and 5.

**Fig. 3** A graphical view of the designed slope in (a) the schematic view and (b) results of the horizontal strain diagram obtained from the Optum G2 (for  $b/B=3$ ,  $C_u=75$  kPa,  $\beta=30^\circ$ , and  $w=50$  KN/m<sup>2</sup>)



We have randomly divided the dataset into training and testing sub-classes that have the respective amounts of 0.8 (504 instances) and 0.2 (126 instances). It is important to note that the training instances are utilized for training the ANN and HHO-ANN models. The performance of these methods has been verified using the testing database. Also, *k*-fold cross-validation procedure is utilized to mitigate the bias caused by the random selection of the data [54–56] (see Fig. 4).

## 4 Results and discussion

### 4.1 Implementation and optimization

As stated previously, the main objective of this research is to present a new optimization of the artificial neural network, namely Harris hawks’ optimization, for the stability analysis of soil slopes by predicting the FOS. To this end, four slope stability conditioning factors, namely slope angle, the position of the rigid foundation, the strength of the soil, and the magnitude of the surcharge are considered to create the required dataset. After dividing the data into the training and testing parts,

utilizing the programming language of MATLAB v.2014, the proposed ANN and HHO-ANN models were designed. Based on the authors’ experience, as well as a trial and error process, an MLP neural network with six hidden computational units in the middle layer was developed. In this sense, lots of theoretical attempts have revealed the efficiency of the MLP tool with one hidden layer [57, 58]. Notably, the activation function of “Tansig” was used to activate the calculations of these neurons. This function is expressed as follows:

$$\text{Tan sig}(x) = \frac{2}{1 + e^{-2x}} - 1. \tag{16}$$

After determining the optimal structure of the ANN, the HHO algorithm was coupled with it. It is worth noting that the main aim of such optimization algorithms in incorporation with intelligent tools (e.g., ANFIS and ANN) is to find the most appropriate values for their computational parameters. In the case of MLP we used in this study, the HHO performs to find the solution for a mathematically defined problem which contains the weights and biases of the neurons. Ten different structures of HHO-ANN networks were tested based on the population size. In this sense, the population size was considered to vary from 50 to 500 with 50

**Table 2** Example of the input and output datasets used for training and validating the applied models

No.	$C^u$	$\beta$	$b/B$	$w$	FOS	No.	$C^u$	$\beta$	$b/B$	$w$	FOS	No.	$C^u$	$\beta$	$b/B$	$w$	FOS
1	50	15	0	100	2.35	26	50	60	0	100	1.485	51	200	45	2	150	6.579
2	50	15	0	150	1.587	27	50	60	0	150	1.022	52	200	45	3	50	10.6
3	50	15	1	50	3.611	28	50	60	1	50	2.391	53	200	45	3	100	8.455
4	50	15	1	100	2.616	29	50	60	1	100	1.829	54	200	45	3	150	6.773
5	50	15	1	150	1.756	30	50	60	1	150	1.355	55	200	45	4	50	10.73
6	50	15	2	50	3.575	31	50	60	2	50	2.361	56	200	45	4	100	8.678
7	50	15	2	100	2.669	32	50	60	2	100	1.841	57	200	45	4	150	7.021
8	50	15	2	150	1.793	33	50	60	2	150	1.469	58	200	45	5	50	10.65
9	50	15	3	50	3.555	34	50	60	3	50	2.39	59	200	45	5	100	8.995
10	50	15	3	100	2.678	35	50	60	3	100	1.893	60	200	45	5	150	7.066
11	50	15	3	150	1.794	36	50	60	3	150	1.528	61	200	60	0	50	9.775
12	50	15	4	50	3.556	37	50	60	4	50	2.443	62	200	60	0	100	5.858
13	50	15	4	100	2.683	38	50	60	4	100	1.975	63	200	60	0	150	4.035
14	50	15	4	150	1.797	39	50	60	4	150	1.592	64	200	60	1	50	9.442
15	50	15	5	50	3.57	40	50	60	5	50	2.535	65	200	60	1	100	7.238
16	50	15	5	100	2.686	41	50	60	5	100	2.053	66	200	60	1	150	5.337
17	50	15	5	150	1.79	42	50	60	5	150	1.661	67	200	60	2	50	9.302
18	50	30	0	50	3.107	43	50	75	0	50	2.139	68	200	60	2	100	7.289
19	50	30	0	100	2.034	44	50	75	0	100	1.221	69	200	60	2	150	5.783
20	50	30	0	150	1.391	45	50	75	0	150	0.8478	70	200	60	3	50	9.397
21	50	30	1	50	3.094	46	50	75	1	50	2.086	71	200	60	3	100	7.471
22	50	30	1	100	2.425	47	50	75	1	100	1.595	72	200	60	3	150	6.018
23	50	30	1	150	1.656	48	50	75	1	150	1.177	73	200	60	4	50	9.631
24	50	30	2	50	3.065	49	50	75	2	50	2.055	74	200	60	4	100	7.788
25	50	30	2	100	2.435	50	50	75	2	100	1.604	75	200	60	4	150	6.268

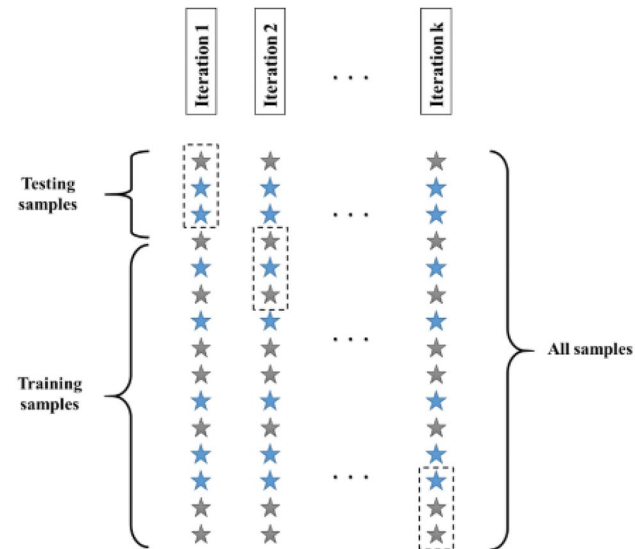
intervals. Each model performed within 1000 repetitions when meaning square error was defined as the objective function (Table 3). Figure 5 shows the obtained convergence

curves. According to this chart, the HHO–ANN having population size = 90 outperformed other tested models. It finally achieved the MSE = 2.469635486 in 4129 s. Remarkably, the majority of the reduction of the MSE occurred in the first 100 iterations.

### 4.2 Performance assessment

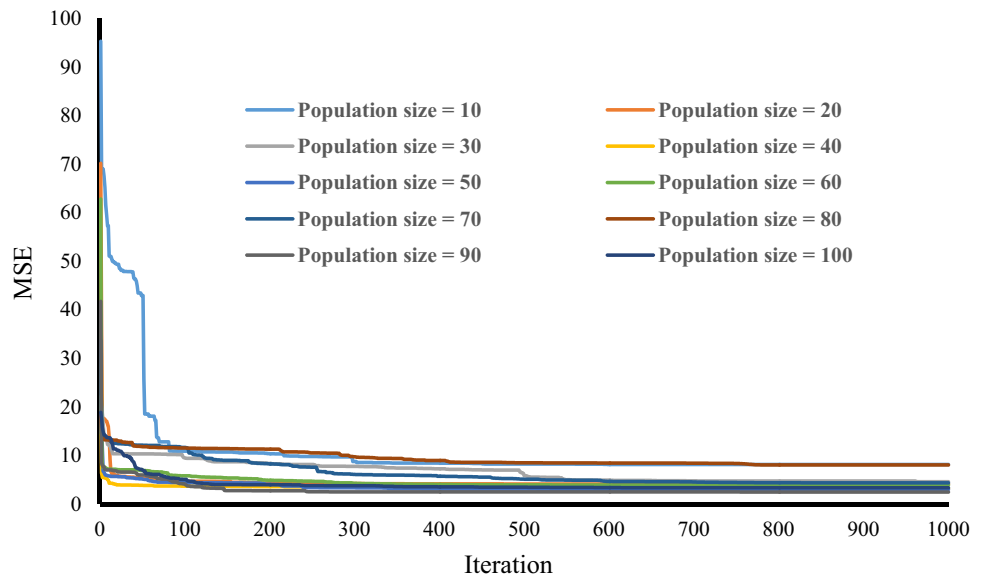
The outputs (i.e., the predicted FOS) of the ANN and HHO–ANN models were extracted and compared with the actual values to evaluate their prediction capability. Two error criteria of root mean square error (RMSE) and mean absolute error (MAE) are used to measure the prediction error. Moreover, the correlation between the observed and predicted FOSs is measured by the coefficient of determination ( $R^2$ ). These indices are expressed as follows:

$$R^2 = 1 - \frac{\sum_{i=1}^N (Y_{i_{\text{predicted}}} - Y_{i_{\text{observed}}})^2}{\sum_{i=1}^N (Y_{i_{\text{observed}}} - Y_{i_{\text{observed}}})^2} \tag{17}$$



**Fig. 4** The  $k$ -fold cross-validation process, taking training and testing samples

**Fig. 5** The convergence curves of tested HHO–ANN networks



**Table 3** Optimized weight and biases of the ANN model

Neuron ( <i>i</i> )	$Z_i = \text{Tansig}(W_{i1} \times C_u + W_{i2} \times \beta + W_{i3} \times b/B + W_{i4} \times w + b_i)$				
	$W_{i1}$	$W_{i2}$	$W_{i3}$	$W_{i4}$	$b_i$
1	0.4783	1.1539	1.0187	1.4842	-2.1911
2	0.9375	-1.3227	0.8848	1.1788	-1.3147
3	1.6702	-0.6027	-1.1291	-0.6109	-0.4382
4	0.9165	-1.8969	-0.5821	-0.1542	0.4382
5	1.7448	0.9500	0.3645	0.8494	1.3147
6	1.5837	1.4772	-0.1290	0.3066	2.1911

$$\text{MAE} = \frac{1}{N} \sum_{i=1}^N |Y_{i_{\text{observed}}} - Y_{i_{\text{predicted}}}|, \tag{18}$$

$$\text{RMSE} = \sqrt{\frac{1}{N} \sum_{i=1}^N [(Y_{i_{\text{observed}}} - Y_{i_{\text{predicted}}})]^2}, \tag{19}$$

where  $Y_{i_{\text{predicted}}}$  and  $Y_{i_{\text{observed}}}$  stand for the predicted and actual FOSs, respectively. The term  $N$  symbolizes the number of samples and  $\bar{Y}_{\text{observed}}$  denotes the average value of the observed FOS.

Figure 6 illustrates the results of the ANN and HHO–ANN models. In these figures, the error (i.e., the difference between the actual and predicted) and histogram of the errors are also presented. Based on the results, applying the HHO algorithm has helped the ANN to have a better analysis of the relationship between the FOS and its conditioning factors. In this sense, the training RMSE was

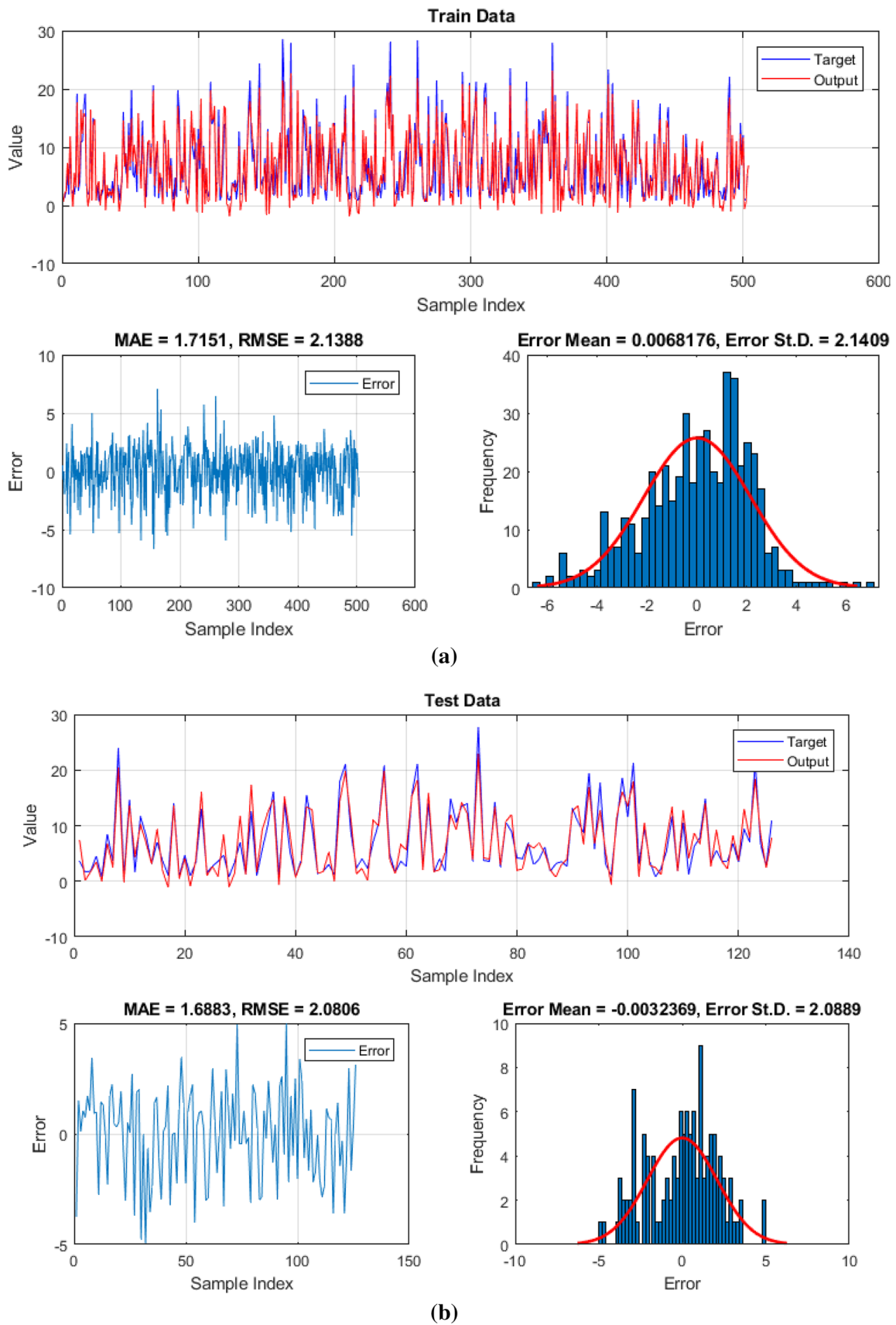
decreased by 26.52% (from 2.1388 to 1.5715). As for the MAE, the HHO reduced this error criterion by 32.31% (from 1.7151 to 1.1610). Furthermore, the obtained values of  $R^2$  (0.8778 vs. 0.9339) show more consistency for the outputs of the HHO–ANN. About the testing phase, it can be deduced that using the HHO increases the generalization power (i.e., predicting the unseen samples) of the ANN. More clearly, the testing RMSE and MAE fell by 20.47% (from 2.0806 to 1.6546) and 26.97% (from 1.6883 to 1.2330), respectively. Besides, the correlation analysis between the testing outputs of the ANN and HHO–ANN show that the  $R^2$  increases from 0.8220 to 0.9253.

### 4.3 Presenting the HHO-based predictive formula

Overall, it was found that the weights and biases which were suggested by the HHO algorithm can predict the FOS more efficiently than those found in the non-optimized ANN. Hence, in this part of the study, it was aimed to extract the FOS predictive formula from the HHO–ANN model. Notably, the calculated accuracy criteria indicate that it can estimate the FOS accurately, by taking into consideration four slope stability influential factors, namely slope angle, the position of the rigid foundation, strength of the soil, and applied surcharge. Equation 20 denotes the HHO–ANN formula:

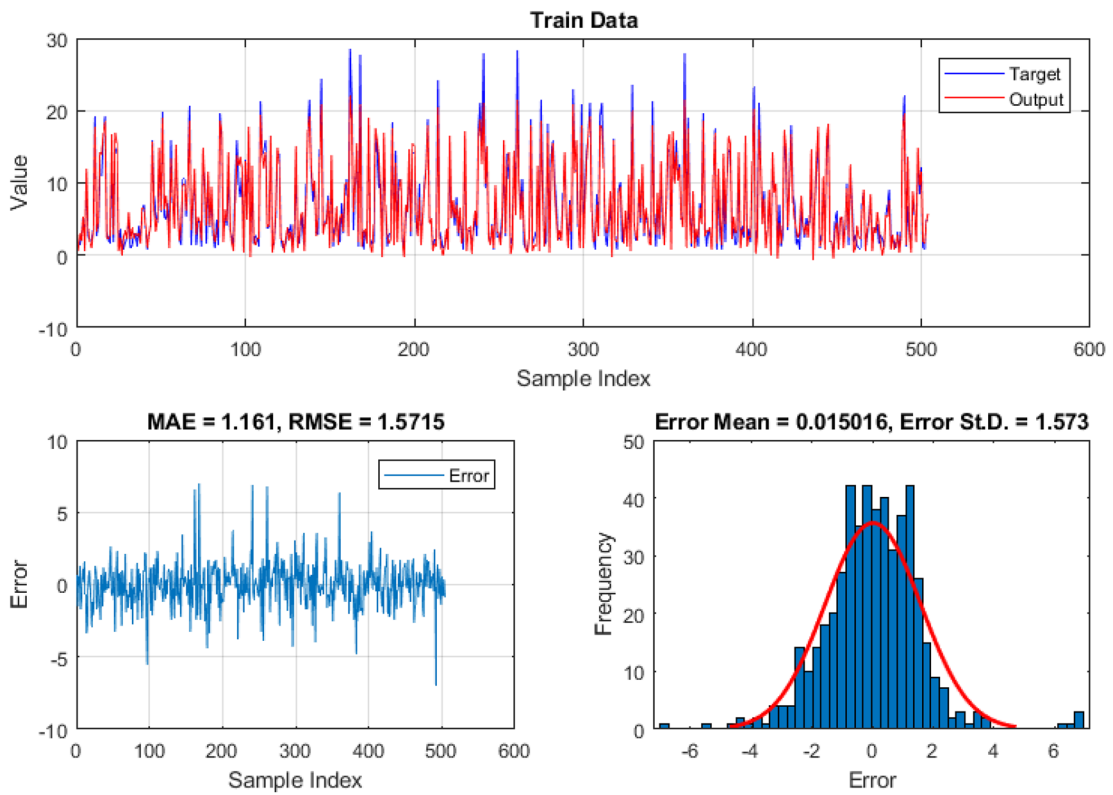
$$\begin{aligned} \text{FOS}_{\text{HHO-ANN}} = & -0.7312 \times Z_1 - 0.9610 \times Z_2 - 0.7498 \times Z_3 \\ & -0.5534 \times Z_4 - 0.1017 \times Z_5 \\ & +0.0691 \times Z_6 + 0.9808, \end{aligned} \tag{20}$$

where  $Z_1, Z_2, \dots, Z_6$  are calculated as shown in Table 3.

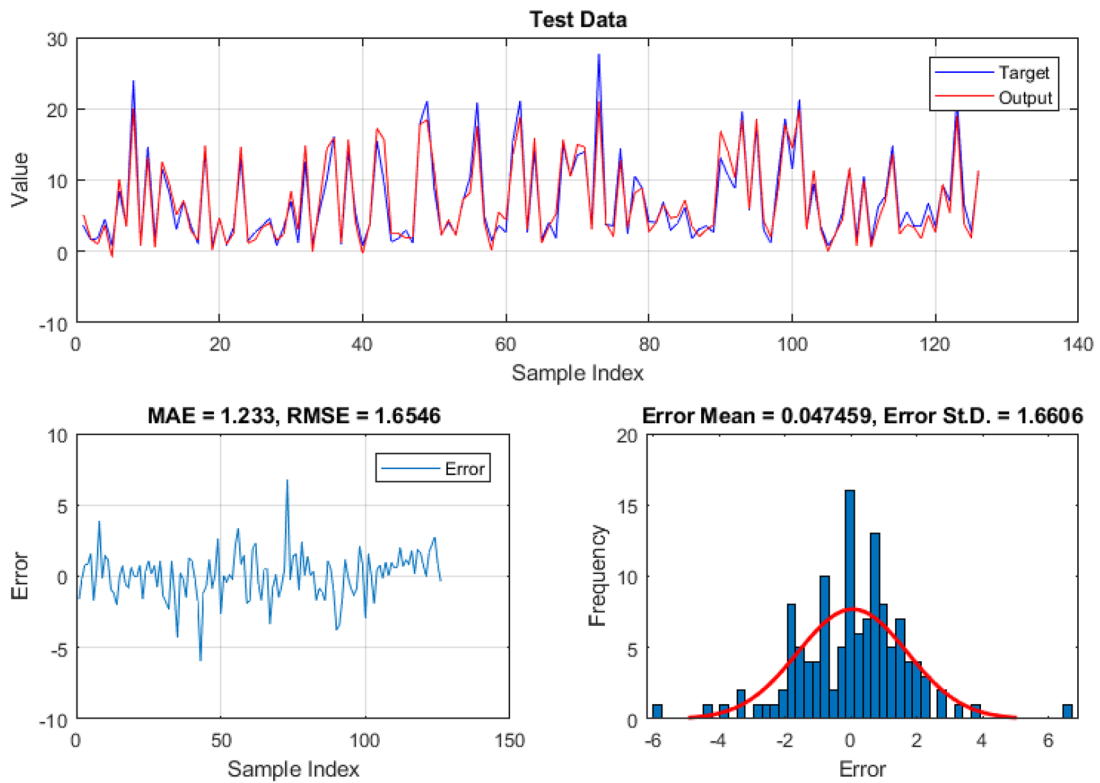


**Fig. 6** The prediction results of the (a and b) ANN and (c and d) HHO-ANN models, respectively, for the training and testing samples





(c)



(d)

Fig. 6 (continued)

## 5 Conclusion

The complexity of environmental threats has driven scholars to employ evolutionary evaluative methods for dealing with them. The stability of the soil slopes is a crucial civil engineering issue which needs nonlinear analysis. In this paper, Harris hawks' optimization was used as a novel hybrid metaheuristic technique for optimizing the performance of the artificial neural network in predicting FOS of the soil slope. In other words, the HHO was used to overcome the computational drawbacks of the ANN, through finding the best-fitted structure. Based on the results of the sensitivity analysis, the HHO–ANN with population size = 90 outperforms others. Moreover, the findings showed that synthesizing the HHO algorithm can effectively help the ANN to have more consistent learning and predicting of the slope failure pattern. Lastly, conducting comparative studies for comparing the potential of the used HHO algorithm with other well-known optimization techniques is a good idea for future works to determine the most appropriate technique for solving the mentioned problem.

## Compliance with ethical standards

**Conflict of interest** The authors declare no conflict of interest.

## References

- Pourghasemi HR, Pradhan B, Gokceoglu C (2012) Application of fuzzy logic and analytical hierarchy process (AHP) to landslide susceptibility mapping at Haraz watershed, Iran. *Nat Hazards* 63:965–996
- Latifi N, Rashid ASA, Siddiqua S, Majid MZA (2016) Strength measurement and textural characteristics of tropical residual soil stabilised with liquid polymer. *Measurement* 91:46–54
- Moayedi H, Huat BB, Kazemian S, Asadi A (2010) Optimization of tension absorption of geosynthetics through reinforced slope. *Electron J Geotech Eng B* 15:1–12
- Marto A, Latifi N, Janbaz M, Kholghifard M, Khari M, Alimohammadi P, Banadaki AD (2012) Foundation size effect on modulus of subgrade reaction on sandy soils. *Electron J Geotech Eng* 17:2523–2530
- Moayedi H, Huat BK, Kazemian S, Asadi A (2010) Optimization of shear behavior of reinforcement through the reinforced slope. *Electron J Geotech Eng* 15:93–104
- Raftari M, Kassim KA, Rashid ASA, Moayedi H (2013) Settlement of shallow foundations near reinforced slopes. *Electron J Geotech Eng* 18:797–808
- Nazir R, Ghareh S, Mosallanezhad M, Moayedi H (2016) The influence of rainfall intensity on soil loss mass from cellular confined slopes. *Measurement* 81:13–25
- Javankhoshdel S, Bathurst RJ (2014) Simplified probabilistic slope stability design charts for cohesive and cohesive-frictional ( $c - \phi$ ) soils. *Can Geotech J* 51:1033–1045
- Duncan JM (1996) State of the art: limit equilibrium and finite-element analysis of slopes. *J Geotech Eng* 122:577–596
- Kang F, Xu B, Li J, Zhao S (2017) Slope stability evaluation using Gaussian processes with various covariance functions. *Appl Soft Comput* 60:387–396
- Trigila A, Iadanza C, Esposito C, Scarascia-Mugnozza G (2015) Comparison of logistic regression and random forests techniques for shallow landslide susceptibility assessment in Giampilieri (NE Sicily, Italy). *Geomorphology* 249:119–136
- Hervás-Martínez C, Gutiérrez PA, Peñá-Barragán JM, Jurado-Expósito M, López-Granados F (2010) A logistic radial basis function regression method for discrimination of cover crops in olive orchards. *Expert Syst Appl* 37:8432–8444
- Basarir H, Kumral M, Karpuz C, Tutluoglu L (2010) Geostatistical modeling of spatial variability of SPT data for a borax stockpile site. *Eng Geol* 114:154–163
- Secci R, Foddis ML, Mazzella A, Montisci A, Uras G (2015) Artificial neural networks and kriging method for slope geomechanical characterization, engineering, geology for society and territory-volume 2. Springer, Switzerland, pp 1357–1361
- Zhang Y, Dai M, Ju Z (2015) Preliminary discussion regarding SVM kernel function selection in the twofold rock slope prediction model. *J Comput Civil Eng* 30:04015031
- Moayedi H, Nazir R, Mosallanezhad M, Noor RBM, Khalilpour M (2018) Lateral deflection of piles in a multilayer soil medium. Case study: the terengganu seaside platform. *Measurement* 123:185–192
- Mosallanezhad M, Moayedi H (2017) Developing hybrid artificial neural network model for predicting uplift resistance of screw piles. *Arab J Geosci* 10:479
- Gao W, Karbasi M, Hasanipanah M, Zhang X, Guo J (2018) Developing GPR model for forecasting the rock fragmentation in surface mines. *Eng Comput* 34:339–345
- Gao W, Karbasi M, Derakhsh AM, Jalili A (2019) Development of a novel soft-computing framework for the simulation aims: a case study. *Eng Comput* 35:315–322
- Youssef AM, Pradhan B, Al-Harathi SG (2015) Assessment of rock slope stability and structurally controlled failures along Samma escarpment road, Asir Region (Saudi Arabia). *Arab J Geosci* 8:6835–6852
- Acharyya R, Dey A (2018) Assessment of bearing capacity for strip footing located near sloping surface considering ANN model. *Neural Comput Appl* 31:1–14
- Moayedi H, Hayati S (2018) Modelling and optimization of ultimate bearing capacity of strip footing near a slope by soft computing methods. *Appl Soft Comput* 66:208–219
- Singh J, Banka H, Verma AK (2019) A BBO-based algorithm for slope stability analysis by locating critical failure surface. *Neural Comput Appl* 31:1–18
- Jellali B, Frikha W (2017) Constrained particle swarm optimization algorithm applied to slope stability. *Int J Geomech* 17:06017022
- Pei H, Zhang S, Borana L, Zhao Y, Yin J (2019) Slope stability analysis based on real-time displacement measurements. *Measurement* 131:686–693
- Chakraborty A, Goswami D (2017) Prediction of slope stability using multiple linear regression (MLR) and artificial neural network (ANN). *Arab J Geosci* 10:385
- Ließ M, Glaser B, Huwe B (2011) Functional soil-landscape modelling to estimate slope stability in a steep Andean mountain forest region. *Geomorphology* 132:287–299
- Gao W, Raftari M, Rashid ASA, Mu'azu MA, Jusoh WAW (2019) A predictive model based on an optimized ANN combined with ICA for predicting the stability of slopes. *Eng Comput* 35:1–20

29. Moayedi H, Mosallanezhad M, Mehrabi M, Safuan ARA, Biswa-jeet P (2019) Modification of landslide susceptibility mapping using optimized PSO-ANN technique. *Eng Comput* 35:967–984
30. Yuan C, Moayedi H (2019) The performance of six neural-evolutionary classification techniques combined with multi-layer perception in two-layered cohesive slope stability analysis and failure recognition. *Eng Comput* 36:1–10
31. Nguyen H, Mehrabi M, Kalantar B, Moayedi H, MaM Abdullahi (2019) Potential of hybrid evolutionary approaches for assessment of geo-hazard landslide susceptibility mapping. *Geomat Na Hazards Risk* 10:1667–1693
32. McCulloch WS, Pitts W (1943) A logical calculus of the ideas immanent in nervous activity. *Bull Math Biophys* 5:115–133
33. Committee AT (2000) Artificial neural networks in hydrology. II: hydrologic applications. *J Hydrol Eng* 5:124–137
34. Alnaqi AA, Moayedi H, Shahsavari A, Nguyen TK (2019) Prediction of energetic performance of a building integrated photovoltaic/thermal system through artificial neural network and hybrid particle swarm optimization models. *Energy Convers Manag* 183:137–148
35. Xi W, Li G, Moayedi H, Nguyen H (2019) A particle-based optimization of artificial neural network for earthquake-induced landslide assessment in Ludian county, China. *Geomat Nat Hazards Risk* 10:1750–1771
36. Yuan C, Moayedi H (2019) Evaluation and comparison of the advanced metaheuristic and conventional machine learning methods for the prediction of landslide occurrence. *Eng Comput* 36:1–11
37. Wang B, Moayedi H, Nguyen H, Foong LK, Rashid ASA (2019) Feasibility of a novel predictive technique based on artificial neural network optimized with particle swarm optimization estimating pullout bearing capacity of helical piles. *Eng Comput* 36:1–10
38. Liu L, Moayedi H, Rashid ASA, Rahman SSA, Nguyen H (2019) Optimizing an ANN model with genetic algorithm (GA) predicting load-settlement behaviours of eco-friendly raft-pile foundation (ERP) system. *Eng Comput* 35:1–13
39. Moayedi H, Huat BB, Mohammad Ali TA, Asadi A, Moayedi F, Mokheri M (2011) Preventing landslides in times of rain-fall: case study and FEM analyses. *Disaster Prevent Manag Int J* 20:115–124
40. Moayedi H, Rezaei A (2017) An artificial neural network approach for under-reamed piles subjected to uplift forces in dry sand. *Neural Comput Appl* 31:327–336
41. Moayedi H, Hayati S (2018) Applicability of a CPT-based neural network solution in predicting load-settlement responses of bored pile. *Int J Geomech* 18:06018009
42. Seyedashraf O, Mehrabi M, Akhtari AA (2018) Novel approach for dam break flow modeling using computational intelligence. *J Hydrol* 559:1028–1038
43. Gao W, Dimitrov D, Abdo H (2018) Tight independent set neighborhood union condition for fractional critical deleted graphs and ID deleted graphs. *Discrete Contin Dyn Syst S* 12:711–721
44. Gao W, Guirao JLG, Abdel-Aty M, Xi W (2019) An independent set degree condition for fractional critical deleted graphs. *Discrete Contin Dyn Syst S* 12:877–886
45. Gao W, Guirao JLG, Basavanagoud B, Wu J (2018) Partial multi-dividing ontology learning algorithm. *Inf Sci* 467:35–58
46. Gao W, Wang W, Dimitrov D, Wang Y (2018) Nano properties analysis via fourth multiplicative ABC indicator calculating. *Arab J Chem* 11:793–801
47. Gao W, Wu H, Siddiqui MK, Baig AQ (2018) Study of biological networks using graph theory. *Saudi J Biol Sci* 25:1212–1219
48. Kavzoglu T, Mather PM (2003) The use of backpropagating artificial neural networks in land cover classification. *Int J Remote Sens* 24:4907–4938
49. Heidari AA, Mirjalili S, Faris H, Aljarah I, Mafarja M, Chen H (2019) Harris Hawks optimization: algorithm and applications. *Future Gener Comput Syst* 97:849–872
50. Bao X, Jia H, Lang C (2019) A novel hybrid harris hawks optimization for color image multilevel thresholding segmentation. *IEEE Access* 7:76529–76546
51. Jia H, Lang C, Oliva D, Song W, Peng X (2019) Dynamic harris hawks optimization with mutation mechanism for satellite image segmentation. *Remote Sens* 11:1421
52. Du P, Wang J, Hao Y, Niu T, Yang W (2019) A novel hybrid model based on multi-objective Harris hawks optimization algorithm for daily PM<sub>2.5</sub> and PM<sub>10</sub> forecasting. *arXiv preprint arXiv:1905.13550*
53. Krabbenhoft K, Lyamin A, Krabbenhoft J (2015) Optum computational engineering (Optum G2). Available on: [www.optumce.com](http://www.optumce.com). Accessed 2018
54. Allayear SM, Sarker K, Ara SJF (2018) Prediction model for prevalence of type-2 diabetes complications with ANN approach combining with K-fold cross validation and K-means clustering. *Advances in information and communication networks: proceedings of the 2018 future of information and communication conference (FICC)*, San Francisco, United States
55. Nandi GC, Agarwal P, Gupta P, Singh A (2018) Deep learning based intelligent robot grasping strategy. *14th International conference on control and automation (ICCA)*, Anchorage, AK, USA
56. Abas M, Zubir N, Ismail N, Yassin I, Ali N, Rahiman M, Saiful N, Taib M (2017) Agarwood oil quality classifier using machine learning. *J Fund Appl Sci* 9:62–76
57. Cybenko G (1989) Approximation by superpositions of a sigmoidal function. *Math Control Signals Syst* 2:303–314
58. Hornik K, Stinchcombe M, White H (1989) Multilayer feed-forward networks are universal approximators. *Neural Netw* 2:359–366

**Publisher's Note** Springer Nature remains neutral with regard to jurisdictional claims in published maps and institutional affiliations.