## ORIGINAL ARTICLE

# Causes of large-scale landslides in the Lesser Himalaya of central Nepal

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**Abstract** Geologically and tectonically active Himalayan Range is characterized by highly elevated mountains and deep river valleys. Because of steep mountain slopes, and dynamic geological conditions, large-scale landslides are very common in Lesser and Higher Himalayan zones of Nepal Himalaya. Slopes along the major highways of central Nepal namely Prithvi Highway, Narayangadh-Mugling Road and Tribhuvan Highway are considered in this study of large-scale landslides. Geologically, the highways in consideration pass through crushed and jointed Kathmandu Nappe affected by numerous faults and folds. The relict large-scale landslides have been contributing to debris flows and slides along the highways. Most of the slope failures are mainly bechanced in geological formations consisting phyllite, schist and gneiss. Laboratory test on the soil samples collected from the failure zones and field investigation suggested significant hydrothermal alteration in the area. The substantial hydrothermal alteration in the Lesser Himalaya during advancement of the Main Central Thrust (MCT) and thereby clay mineralization in sliding zones of large-scale landslide are the main causes of large-scale landslides in the highways of central Nepal. This research also suggests that large-scale landslides are the major cause of slope failure during monsoon in the Lesser Himalaya of Nepal. Similarly, hydrothermal alteration is also significant in failure zone of the large-scale landslides. For the sustainable road maintenance in Nepal, it is of utmost importance to study the nature of sliding zones of large-scale landslides along the highways and their role to cause debris flows and slides during monsoon period.

**Keywords** Himalaya · Landslides · Clay minerals · Hydrothermal alteration

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## Introduction

Nepal is a highly mountainous country located between China in the north and India in the south, east and west with an area of 147,181 sq. km (Fig. 1). It occupies about 800 km long central part of the Himalayan arc which has been formed by the collision of Indian and Eurasian plates. Nepal is mainly characterized by rugged topography, very high relief, variable climatic conditions, complex geological structures affected by active tectonic process and seismic activities. Topographic elevation changes from 60 m at the southern plains to 8,848 m at the Mt. Everest in the north within a horizontal distance of less than 200 km (Fig. 1). This kind of topography is prone to landslide and erosion in Nepal. The mountainous and hilly regions of the country occupy nearly 83% of the total area, whereas the



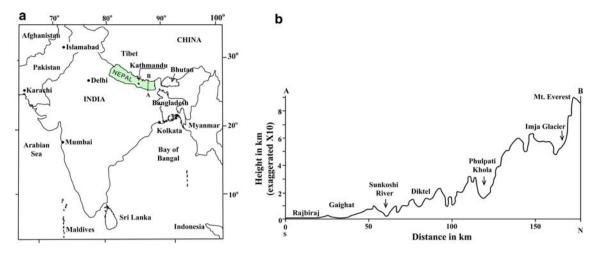
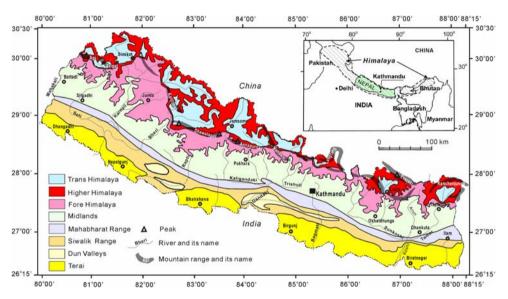


Fig. 1 a Location map of Nepal, b S-N topographic profile (*line AB* in a) showing vast difference in elevation within short distance, height of the Mt Everest is 8,848 m

Fig. 2 Digital elevation model (DEM) based geomorphologic map of Nepal (modified after Hagen 1969, Upreti 1999 and Dahal and Hasegawa 2008)



remaining 17% is flat land. The seasonal monsoon rains and intense but improper land use practices make the Nepalese Himalayas the most unstable landscapes in the world.

Steep slopes are the key features of the Himalayan geomorphology. Rapid uplift from Miocene, which continues even today, has created local relief measurable in kilometres from river valleys to peaks. As a result, large-scale valley slope creeping (large-scale landslides) due to gravity has been prolonging from the early upliftment of mountains, which is a common feature. Thus, landslides may be regarded as one end of the spectrum of slope modification processes in the Nepal Himalaya and adjacent regions.

Landslides in the Himalaya are scale-dependent, from massive extent of whole mountain ranges (gravity tectonics) through failure of single peaks to very minor slope failures (Shroder and Bishop 1998; Shang et al. 2003). However,

only few studies have been done to understand the Nepal Himalayan landslide mechanisms and processes (e.g. Laban 1979; Selby 1988; Ives and Messerli 1981; Caine and Mool 1982; Wagner 1983; Heuberger et al. 1984; Burbank et al. 1996; Upreti and Dhital 1996; Gerrard 1994; Gerrard and Gardner 2000; Chalise and Khanal 2001) and there are still lack of research publications related to the mechanisms and processes of large-scale landsliding in the Lesser Himalaya. In this context, this paper deals with comprehensive information about the large-scale landslides observed in the major highways running through river valleys of central Nepal. The main objectives of this research include, (1) mapping of the large-scale landslides along the major highway of central Nepal, (2) investigating the failure zone of some of large scale landslides in terms of strength and clay mineralogy, and (3) discussing the role of hydrothermally altered clay in the origin of large-scale landslides.



#### Geological setting

Geomorphologically, Nepal is divided into eight units running east—west (Table 1; Fig. 2), namely, Terai, Churia Range, Dun Valley, Mahabharat Range, Midland, Fore Himalaya, Higher Himalaya, Inner and Trans Himalaya (Hagen 1969; Upreti 1999). Likewise, geologically and tectonically, Nepal is divided into five major tectonic zones, namely, Terai, Sub-Himalaya (Siwaliks), Lesser Himalaya, Higher Himalaya and Tibetan-Tethys Himalaya (Ganser 1964; Upreti 1999). These tectonic zones are separated by major thrusts and faults of the Himalaya, namely from north to south (Fig. 3), South Tibetan Detachment System (STDS), Main Central Thrust (MCT), Main Boundary Thrust (MBT) and Main Frontal Thrust (MFT).

According to Bilham et al. (1997), the Indian Plate is moving northwards with an average rate of 1.5–5 cm per year and nearly half of this horizontal slip is being accommodated within the Himalaya. As a result, the rocks of the Himalaya are moving upwards as well as horizontally towards south along the major thrusts (Upreti 2001).

The MCT was very active during the period between 25 and 19 million years ago, and the northern part of the Himalaya gained the present day height very quickly. Steep slopes and deeply eroded river valleys are the outcomes of such movement in the Higher Himalaya.

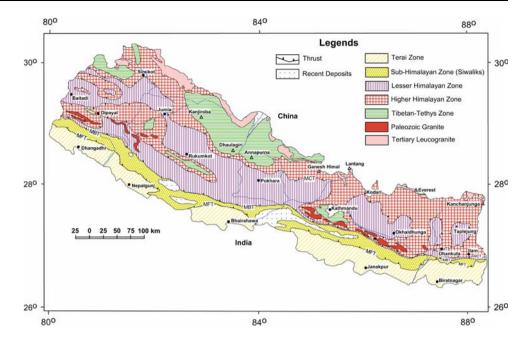
Many investigations have disclosed that movement along the MCT ceased by around 18 Ma (Copeland et al. 1991; Hodges et al. 1996). Such movement caused Higher Himalayan rocks to slide horizontally towards south about 120 km over the young Lesser Himalayan rocks. Thus, it is believed that once upon the geological time much of the Lesser Himalaya must have been covered by the rocks of the Higher Himalayan crystalline rocks like a bedspread. Evidences of such cover can be noticed very clearly in the geological map of Nepal (see Fig. 3). In eastern Nepal, MCT is still in south and only opens as window in the Arun River valley and Taplejung areas. But most of the parts of Higher Himalayan crystalline rocks are already eroded in other area. Although it is presumed that MCT became inactive and the movement along this thrust practically ceased by 18 Ma, it has been found that MCT was reactivated during a period 6–8 million years ago (Harrison et al.

Table 1 Geomorphologic units of Nepal, modified after Hagen (1969), Upreti (1999) and Dahal (2006)

SN	Geomorphic units	Width (km)	Altitudes (m)	71		Main processes for landform development  River deposition, erosion and tectonic upliftment		
1	Terai (northern edge of the Gangetic Plain)	20–50	100–200					
2	Churia range (Siwaliks)	10–50	200-1,300	Sandstone, mudstone, shale and conglomerate.	Mid-Miocene to Pleistocene	Tectonic upliftment, erosion, and slope failure		
3	Dun Valleys	5–30	200–300	Valleys within the Churia Hills filled up by coarse to fine alluvial sediments	Recent	Tectonic upliftment, weathering, erosion, and slope failure		
4	Mahabharat Range	10–35	1,000–3,000	Schist, phyllite, gneiss, quartzite, granite and limestone belonging to the Lesser Himalayan Zone	Precambrian and Paleozoic and some Cenozoic	Tectonic upliftment, weathering, erosion, and slope failure		
5	Midlands	40–60	300–2,000	Schist, phyllite, gneiss, quartzite, granite and limestone geologically belonging to the Lesser Himalayan Zone	Precambrian and Paleozoic to mesozoic	Tectonic upliftment, weathering, erosion, and slope failure		
6	Fore Himalaya	20–70	2,000–5,000	Gneisses, schists, phyllites and marbles mostly belonging to the northern edge of the Lesser Himalayan Zone	Precambrian	Tectonic upliftment, weathering, erosion, and slope failure		
7	Higher Himalaya	10–60	>5,000	Gneisses, schists, migmatites and marbles belonging to the Higher Himalayan Zone	Precambrian	Tectonic upliftment, weathering, erosion (rivers and glaciers), and slope failure		
8	Inner and trans Himalaya	5–50	2,500–4,500	Gneisses, schists and marbles of the Higher Himalayan Zone and Tethyan sediments (limestones, shale, sandstone etc.) belonging to the Tibetan-Tethys Zone	Precambrian and Cambrian to Cretaceous	Tectonic upliftment, wind and glacial erosion, and slope degradation by rock disintegrations		



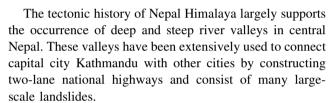
Fig. 3 Generalized geological map of Nepal (modified after Amatya and Jnawali 1994; Dahal 2006)



1997), and the Higher Himalaya again revitalized. Himalayas gain maximum topography around 17 Ma (Searle et al. 1997). As a result, normal fault system, STDS, developed as gravitational collapse structure, which is now considered as a tectonic boundary between the Tibetan-Tethys Zone and the Higher Himalayan Zone.

A thrust called MBT started in the south of the MCT about 10-11 million years ago and all the horizontal movement that had been occurring along the MCT shifted to the MBT (Meigs et al. 1995). This movement along the MBT effectively created high mountain range in the Lesser Himalaya known as the Mahabharat Range. The rivers flowing from the Higher Himalaya to south started to erode the elevated Mahabharat Range, resulting in development of deep and steep river valleys in the Lesser Himalayan Zone. The MBT is an active thrust system and the frontal part of the Lesser Himalaya is comparatively rising fast even today. But the MBT could not accommodate all strains due to northward displacement of the Indian Plate, which resulted in the development of another thrust MFT in the south at southern front of the Siwaliks. Nowadays, the MFT is more active than the MBT and is causing upliftment of the hills of Siwaliks.

Tertiary leucogranites are distributed around the highest peaks of the Himalaya along the border with China (see Fig. 3). They intrude the Higher Himalayan Gneiss and the Tibetan-Tethys Sediments. The radiometric age of the leucogranite is 9–24 million years, which indicates that the leucogranite intruded mainly in Miocene. The upheaval of Higher Himalaya, metamorphism, activity of the MCT and formation of the STDS occurred during the emplacement of the leucogranite (Deniel et al. 1987; Searle et al. 1997, 1999; Harrison et al. 1999; Upreti 1999).



#### The study area

Three highways of central Nepal, namely, Prithivi Highway (H4), Tribhuvan Highway (H2) and Narayangadh-Mugling Road (H5) were selected for this study (Fig. 4), because the present trend of prioritizing natural disasters in Nepal shows that landslide occurrences along the highways, especially the above-mentioned three routes are given much importance. It is also due to greater economic loss as well as a larger number of people affected, especially due to traffic disruption, during an event of roadside failure.

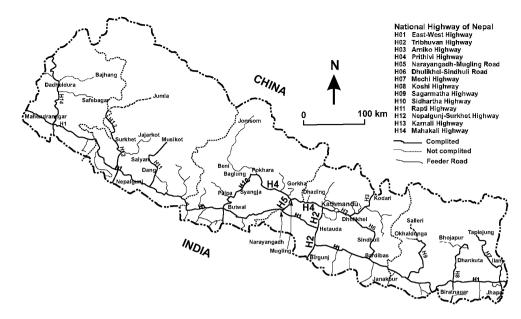
## Methodology

## Landslide mapping

Identification of large-scale creep on a steep mountain valley is a tedious work. To make it easy to recognize landslide spots along the selected road network, landslide inventory mapping was carried out with the help of aerial photo interpretation (API) and plotting the landslide shape and size on the topographic map (1:25,000 scale) based on the judgment from aerial photographs. During identification of



**Fig. 4** Major road network in Nepal and highways H2, H4 and H5 are selected for study



landslides, priority was given to already existing instabilities such as old landslides, colluvial deposits, and alluvial fans. The scale of aerial photographs used was about 1:50,000 and they were 12 years old. So, it was necessary to make field verification, especially for the failures that could have been missed in stereoscope viewing. For the field verification, six field visits were made from year 2003 to 2007. Moreover, the effect of vegetation cover is often high enough to hide slope failures measuring only a few hundred square meters. So, to avoid possibilities of mismatch between deskwork and field existence of landslides, the first field visit program was conducted along Prithvi Highway (Nagdhunga-Pokhara section) in July 2003; the second visit was made along Prithvi Highway (Nagdhunga-Pokhara section) and Narayangadh-Mugling Highway in November 2003; the third and fourth visits were carried out for Prithvi Highway (Naubise-Mugling section), Narayangadh-Mugling Road, and Tribhuvan Highway (Hetauda-Naubise Section) in November 2004 and November 2005. The fifth and sixth visits to for Prithvi Highway (Naubise-Mugling section) and Narayangadh-Mugling Road were carried out in June 2006 and December 2007. The activities carried out during the field visits included visual inspection, experience-based estimation of the failure type and associated geology, existence of vegetation cover and groundwater hydrology as well as examination of failure zones in uphill or downhill section of the roads.

### Soil sampling and laboratory test

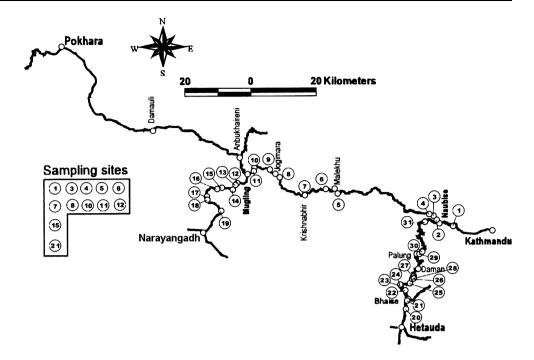
During field verification, failure zone materials were observed in some landslides along the Prithivi Highway, Narayangadh-Mugling Road and Tribhuvan Highway. It was noticed that the failure zone soils consist of significant amount of clay mineralization. Considerable amount of coarse rock fragments (granule to pebble size) were also present in failure zone. As a result, study of stress-strain behaviour and hydrological characteristics (Dai et al. 1999; Frattini et al. 2004; Shakoor and Smithmyer 2005; Yilmaz and Karacan 2002; Yilmaz and Yildirim 2006) of failure zone soil are not possible in undisturbed soil sample. So, disturbed soil samples were collected from the failure zones for laboratory tests in order to interpret residual shear strength parameters and clay mineral composition. Moreover, in some locations, non-clayey materials such as sand and gravel were also noticed in the failure zones. Although 31 sites were taken into consideration as problematic large scale landslide sites, only 12 sites (Fig. 5) were selected for the soil sampling because of availability of clay materials.

Strain-controlled and constant normal stress type direct ring shear apparatus (Bishop et al. 1971) was used to measure shear strength of clayey soils under three effective pressures (1, 2, and 3 kgf/cm²; i.e. 98.07, 196.14 and 294.21 kN/m², respectively). The results obtained from the ring shear tests were interpreted in terms of stress–strain curves and strength envelopes. A total of ten soil samples from the landslides along the highways were used for ring shear test.

The method for the detection of clay minerals in this study is based on X-ray diffraction patterns in (1) powder method, (2) oriented aggregate method, and (3) ethylene glycol treatment method of clay mineralogical analysis. However, the latter two methods were carried out only to confirm the minerals detected in powder method, which often results in an overall representation of minerals in a sample.



Fig. 5 Vulnerable landslide spots and soil sampling sites



Review of the models of Himalayan tectonics

Observations in the clay-rich zones below the thick rock successions distinctly suggested that there were specific roles of fluid movements along the fractures of rockmass. Thus, in this research, special attentions were given to the available previous study about mechanism and model of the MCT, MBT and Tertiary volcanism in Himalaya.

#### Findings of mapping and laboratory test

Relation between large-scale landslide and rainfall triggered landslide

Basic measurements (length, breadth, slope angle and so forth) of failed slopes, study of geological characteristics (rock types, attitudes of joints, foliation and bedding and so forth) of sites and soil sampling were carried out during field visits, and it was noticed that mainly geological formations having phyllite, schist and gneiss are having the instability problem. In total, 31 vulnerable sites were identified after inventory mapping. The inventory maps of all three selected highway stretches were prepared in GIS (ArcGIS 9.0) environment. The inventory maps suggested a quite surprising relation between landslide and road alignment. Almost whole Narayangadh-Mugling Road section was found to be run through the landslide terrains (Fig. 6). Similarly, in 2003, a huge landslide disaster occurred in this road (Dahal et al. 2006; Dahal and Hasegawa 2008) and caused damages to many bridges and hundreds of meters of road stretch. Almost all rainfalltriggered landslides that occurred in 2003 were observed in large-scale landslide material.

Clay minerals of sliding surface material

From the X-ray diffraction patterns of the tested soil samples, it is clearly understood that the main constituent minerals of the collected samples (Table 2) are chlorites, illites, muscovite, actinolite, feldspar and quartz with occasional appearance of smectite, talc and metahalloysite. In Sample 1 of the Prithivi Highway at Khatripauwa village, some traces of montmorillonite (smectite) were observed and confirmed by conducting oriented aggregate analysis and ethylene glycol treatment analysis (Fig. 7). Metasandstone with alternate bands of slates is the main unit in the site. A marked presence and significantly high intensity of X-rays corresponding to illites (along with muscovite) in almost all samples indicates that the landslide materials consist of a higher percentage of illites. High content of quartz in soil samples also gives a clear significance of high value of peak and residual shear resistance (>30°).

Shear strength of sliding surface material

The shear strength results indicate that there is no significant difference in peak strength and residual strength of almost all tested samples because all shear tests were conducted on remolded saturated samples under normally consolidated conditions, which often exhibit insignificant drop in strength in post peak state of shear. Figure 8 shows a comparison of angles of peak and residual shear resistances. Although the number of samples collected was not



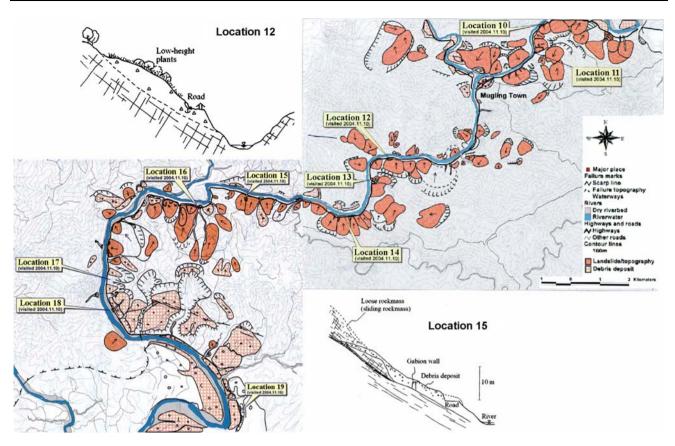


Fig. 6 Example of landslide inventory map, some section of Prithivi Highway and Narayangadh-Mugling road

Table 2 Relative occurrences of clay minerals in some of investigated sites

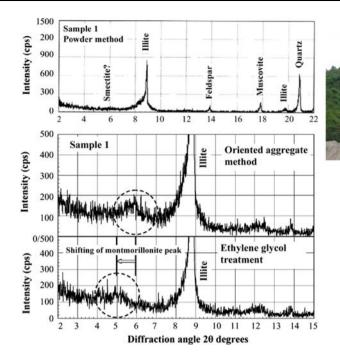
Loc.	Occurence	Minerals							
		Quartz	Albite	K-felspars	Chlorite	Illite	Actinolite	Smectite	Muscovite
No. 1	Weathered metasandtone	++	+	_	_	+++	_	+	+
No. 6	Weathered landslide materials	+++	_	_	++	++++	_	_	+++
No. 10	Weathered landslide materials	++	_	_	++	+++	_	_	++
No. 15	Phyllite (bedrock)	++++	+++	_	+++	++	_	_	_
	Sliding zone materials (micro-breccia)	++++	+++	_	+++	++	_	_	_
	Coarse breccia	++++	++	_	+++	+++	_	_	_
No. 16	Amphobolite (bedrock)	+++	+++	_	++++	++	++++	_	_
	Sliding zone materials (micro-breccia)	+++	++	_	+++++	+	++++	_	_
	Coarse breccia	+++	+++	_	++++	++	++++	_	_
No. 25	Granite (bedrock)	+++++	+++	+++	_	+++	+++	_	_
	Granite powder	++++	+++	+++	_	+++	+++	_	_
						Abundant +++++ + + Poor			Poor

significant enough to give an interpretation for the range of internal friction angles representing all landslide and slope failure soils along the highways, it is believed that the range marked in the figure more or less represents a greater part of their shear strength parameters. The marked ranges show that the peak angle of shear resistance varies between  $22^{\circ}$  and  $36^{\circ}$ , whereas the residual angle varies between  $21^{\circ}$ 

and 34°. This indicates that the landslides and failure zone materials of the study area do not possess a general average value of angle of shearing resistance, which is necessary for stability of slopes of study area, having average slope angle of 30°. Likewise, this site consists of slate and green phyllite and shows relatively less value of residual shear resistance than that of quartzite, limestone and granite.



**Fig. 7** X-ray diffraction of landslide materials from location 1 (see Fig. 5) of the Prithivi Highway



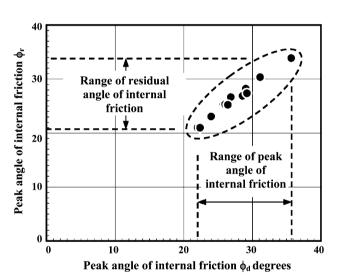


Fig. 8 Comparison of peak and residual angles of internal friction for representative soil samples along the Prithivi Highway, Narayangadh-Mugling Road, and Tribhuvan Highway

## Hydrothermal alteration and landsliding

Hydrothermal alteration is a process in which mineralogical changes take place as a result of interaction of rocks with hot water solutions, often known as hydrothermal fluids. These fluids carry various metallic ions in solution, either from a nearby igneous source, or from leaching out of some nearby rocks. Hydrothermal alteration is common in a wide variety of geological environments, including thrust and batholith zones. The study of sheared and crushed zones in various geological units suggested that there were significant hydrothermal alterations in the Himalayan rockmass.

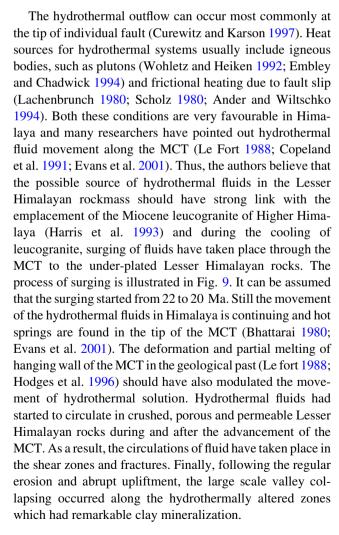
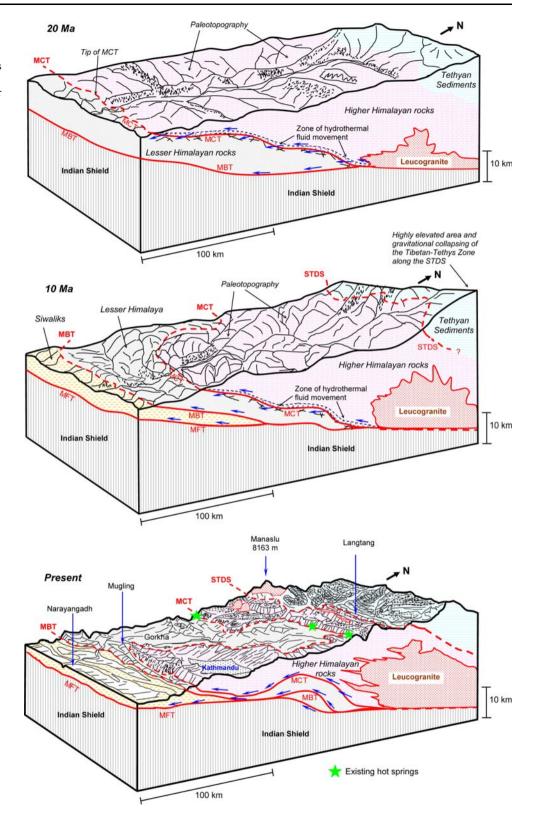




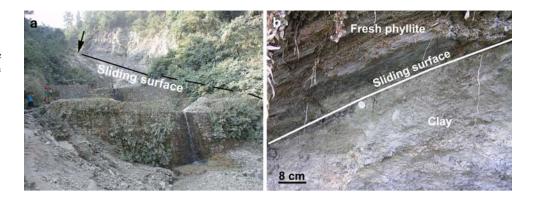
Fig. 9 Conceptual model of surging of hydrothermal solution in central Nepal Himalaya. The surging of fluids in geological past attributed to the clay mineralization in Lesser Himalayan rockmass. Geology and topography adopted from Hagen (1969), Brunel (1986), Le fort (1988) and Schelling (1992)



Hydrothermally altered zones or clay mineralizations along fractures were well observed in sliding zones of large-scale landslides at various locations and illustrations of such zones are provided in Fig. 10 (location 15) and Fig. 11 (location 16). At location 15 (see Fig. 6), the sliding surface of the large-scale landslide consists of more



**Fig. 10** Large-scale landslide at Location 15 along Narayangadh-Mugling Road and a close view of slip surface and altered clay zone. *Arrow* in **a** shows the location of **b** 



than 1-m-thick gray clay layer (Fig. 10). The overlaying rock is fresh phyllite, but it has abundant open joints and fracture. The boundary between sliding surface and overlaying phyllite is very sharp and phyllite does not have shear structure. These observations indicated that the sliding surface which resembles fault gouge was formed by hydrothermal alteration. Similarly, at location 16 (see Fig. 6), the sliding surface of the large-scale landslide consists of more than 1-m-thick greenish grey clay and breccia layer (Fig. 11). The overlaying rock is amphibolite and it is also fresh as in location 15.

The rockmass of amphibolite also has abundant open joints and fractures. The boundary between sliding surface and overlaying amphibolite is very sharp and clay and breccia layer does not have shear structure (Fig. 11). This emplacement also indicates that the sliding surface which has many similarities to fault breccia was previously formed by hydrothermal alteration.

The clay mineralization in the failure zones noticed in this study and the existence of many hot water springs along the vicinity of MCT (Bhattarai 1980) are the main supporting evidences to the hydrothermal alteration in the Lesser Himalayan Zone. Strength of the clay materials estimated from ring shear is also supporting the overlain rockmass creeping along the zone of clay mineralization.

Typical large-scale landslide terrain of the Lesser Himalaya is illustrated in Fig. 12. The landslide terrain of

Nepal usually has three stage of landslide activation. The first stage (Fig. 12) corresponds to the large scale failure along the zone of clay mineralization whereas small scale slope failures are found in same huge landslide body as III stage of landslide incidence. In the roadside slope of Nepal, mostly III stage landslides were mitigated from low cost mitigation measures and were still noticed to be ineffective because of disturbed mass owing to the stage I and II landslides. The schematic model illustrated in Fig. 12 is perfectly observed in field and examples from locations 6 and 10 are given in Figs. 13a, b.

## Concluding remarks

Landslide mapping of selected highways (H2, H4, and H5) in Nepal shows that more than 50% of the road alignment passes through large-scale landslide areas. Thus, most of the stretches have severe landslides and debris flow problems. The results of X-ray diffraction analysis of soils revealed that large-scale landslides along the highways have significant clay mineralization in sliding zones. The substantial hydrothermal alteration in the Lesser Himalaya during and after the advancement of the MCT and thereby clay mineralization in sliding zones of large-scale landslide are the main causes of large-scale landslides in the highways of central Nepal.

Fig. 11 Large-scale landslide at Location 16 along Narayangadh-Mugling Road and close view of slip surface and altered clay zone. *Arrow* in a shows the location of b







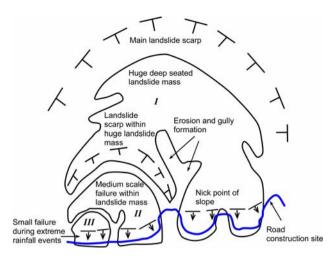
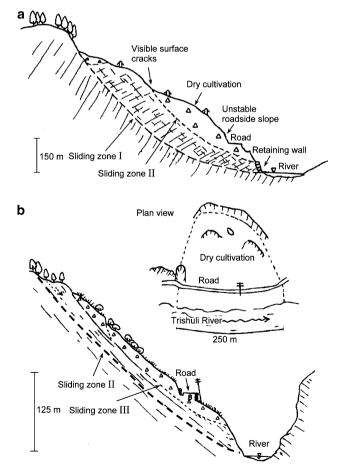


Fig. 12 Schematic model of valley creeping type landslide terrain in mountains of central Nepal. The road is found to be constructed on toe of large-scale landslides having clay mineralization in failure zone as shown in Fig. 10 and 11. Stage III is associated with the monsoon rainfall related landsliding process



**Fig. 13 a** Longitudinal section of valley creeping landslide at location 6 (see Fig. 5) and **b** Both plan and section view of landslide terrain and valley collapsing features at location 10 (see Fig. 5)

Similarly, the alignment investigation of roads in the Lesser Himalaya of Nepal should be inevitably accommodated with the wide range of surface and subsurface exploration techniques. The selected highways in this study are the main routes to the capital city of Kathmandu from the rest of the country, and the investigation suggested that for the sustainable road maintenance, it is of utmost importance to study the nature of sliding zones of large-scale landslides along the highways and their role to cause debris flows and slides during monsoon period. Likewise, the large-scale landslides also need to be considered during construction of roadside cut slopes, tunnels and mitigation measures.

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