



Structural shape optimization using Cartesian grids and automatic h -adaptive mesh projection

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Abstract

We present a novel approach to 3D structural shape optimization that leans on an Immersed Boundary Method. A boundary tracking strategy based on evaluating the intersections between a fixed Cartesian grid and the evolving geometry sorts elements as internal, external and intersected. The integration procedure used by the NURBS-Enhanced Finite Element Method accurately accounts for the nonconformity between the fixed embedding discretization and the evolving structural shape, avoiding the creation of a boundary-fitted mesh for each design iteration, yielding in very efficient mesh generation process. A Cartesian hierarchical data structure improves the efficiency of the analyzes, allowing for trivial data sharing between similar entities or for an optimal reordering of the matrices for the solution of the system of equations, among other benefits. Shape optimization requires the sufficiently accurate structural analysis of a large number of different designs, presenting the computational cost for each design as a critical issue. The information required to create 3D Cartesian h -adapted mesh for new geometries is projected from previously analyzed geometries using shape sensitivity results. Then, the refinement criterion permits one to directly build h -adapted mesh on the new designs with a specified and controlled error level. Several examples are presented to show how the techniques here proposed considerably improve the computational efficiency of the optimization process.

Keywords Cartesian grids · h -refinement · Shape optimization · NEFEM

1 Introduction

The structural shape optimization problem can be tackled as the minimization of a real function f , which depends on some variables and is subjected to several constraints. The generic form of such problem is:

$$\begin{aligned} & \text{minimize } f(\mathbf{a}) \\ & \text{where } \mathbf{a} = \{a_m\} \quad m = 1, \dots, q \\ & \text{verifying } g_n(\mathbf{a}) \leq 0 \quad n = 1, \dots, r \\ & \text{and } h_l(\mathbf{a}) = 0 \quad l = 1, \dots, s \end{aligned} \quad (1)$$

being f the objective function, a_m are the design variables, g_n are inequality constraints and the values h_l define equality constraints. The vector \mathbf{a} defines a specific

structural shape and the task consists in finding the \mathbf{a} values which define the optimum design.

The algorithms for the solution of (1) are, normally, iterative. Among the optimization algorithms we will mainly focus in this paper on the gradient-based algorithms because of their fast convergence to the optimal solution. These methods require the computation of the objective function, the constraints and their derivatives (sensitivities) with respect to the design variables for each geometry considered during the process. In addition, in every step of that process, it is necessary to evaluate the values, and their sensitivities, for f and \mathbf{g} . In this work, these calculations are done through Finite Element Analysis (FEA).

There are different approaches and many codes to solve the optimization problem. However, some problems in this context, identified long time ago (Braibant and Fleury 1984; Haftka and Grandhi 1986), still remain unsolved, like the incorporation of robust parametrization techniques for the definition of each design and the unwanted variations in the structural response due to mesh-dependency effects. Also, in many optimization processes based on the use of FEA,

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there is no control on the accuracy of the numerical solution. As a result, when the process comes to an end there is no guarantee on the practicability of the final outcome; sometimes analyzes with higher accuracy levels would expose that the final design is unfeasible, contravening one or more of the constraints imposed. The control of the error related with the Finite Element (FE) computation and its impact on the solution of the optimization problem was analyzed in Ródenas et al. (2011).

Two main concepts evolved to bypass these drawbacks. On the one hand, the design update procedure can be assigned to geometry model (Bennett and Botkin 1985; Chang and Choi 1992). In this case, the nodal coordinates manipulation within the FE discretization is avoided, thus removing impracticable patterns that cannot be defined by any combination of design variables. Another idea would be to improve the geometrical accuracy of the models by integrating CAD representations with the FEM solvers, as in the case of Isogeometric Analysis (IGA) (Hughes et al. 2005; Nguyen et al. 2015). However, in its finite element form, generating an analysis-suitable solid discretization is an open topic (Zhang et al. 2013; Escobar et al. 2014; Liu et al. 2014). Some works on IGA shape optimization can be found in Cho and Ha (2009), Ha et al. (2010), Qian (2010), Li and Qian (2011), and Lian et al. (2016).

On the other hand, the FE model could be updated through the optimization procedure to improve the accuracy of the numerical simulation results or to enhance the element quality (Kikuchi et al. 1986; Yao and Choi 1989; Riehl and Steinmann 2014). This may, for instance, take the form of adaptive mesh refinement based on error estimation in energy norm (Zienkiewicz and Zhu 1987) or goal oriented adaptivity (González-Estrada et al. 2014). However, when it comes to complicated geometries or to large shape changes, these strategies may still necessitate computationally expensive re-meshing algorithms.

From this perspective, so-called Immersed Boundary (IB) discretization techniques seem the most appropriate choice for structural shape optimization. The main notion behind these methods is to extend the structural analysis problem to an easy-to-mesh approximation domain that encloses the physical domain boundary. Then, it suffices to generate a discretization based on the approximation domain subdivision, rather than a geometry-conforming FE mesh. Moreover, when the structural component is allowed to evolve, the physical points move through the fixed discretization created from the embedding domain where there will be no mesh distortion. There are plenty of alternatives within the IB scope. Among many other names used to describe these techniques where the mesh does not match the domain's geometry, we have the Immersed Boundary Method (IBM) (Peskin 1977) and the Immersed Finite Element Method (IFEM) (Zhang et al. 2004).

These methods have been studied by a number of authors for very different problems including, of course, shape optimization (Haslinger and Jedelsky 1996; Kunisch and Peichl 1996; Kim and Chang 2005; Najafi et al. 2015; Riehl and Steinmann 2016).

Nevertheless, the attractive advantages of IB approaches come together with numerical challenges. Basically, the computational effort has moved from the use of expensive meshing algorithms toward the use of, for example, elaborated numerical integration schemes to be able to capture the mismatch between the geometrical domain boundary and the embedding FE mesh. All intersected elements have to be integrated properly in order to account for the volume fractions interior to the physical domain. The domain integration for these methods have been investigated in the literature.

The first intuition, is to homogenize the material properties within intersected elements based on the actual volume fraction of these elements covered by the domain. This approach is straight forward and could be computationally efficient but it provides low accuracy for the structural analysis (García-Ruiz and Steven 1999; Dunning et al. 2011).

A more selective approach is employed in the Finite Cell framework (Parvizian et al. 2007; Düster et al. 2008; Schilling and Ruess 2015). Therein, for all intersected elements, a number of integration points are provided employing hierarchical *octree* data structures (Meagher 1980; Jackins and Tanimoto 1980; Doctor and Torborg 1981) for their distribution. Then, only the integration points interior to the physical domain are considered in the respective integral contribution. However, regardless of the number of integration points, the exact representation of the geometry is not possible.

Still, the highest level of accuracy and optimal convergence rate, for the embedding domain structural analysis, is obtained only when proper integration schemes are used along the intersected elements. Very recently, several methodologies to perform high-order integration in embedded methods have emerged, such as the so-called 'smart octrees' tailored for Finite Cell approaches (Kudela et al. 2016) or techniques used where the geometry is defined implicitly by level sets (Fries and Omerović 2016). Even in the first of these approaches, where the isoparametric reoriented elements only provide an approximated FE description of the boundary, the exact geometry is not taken into account at the integration stage.

A step further to improve accuracy and retain the optimal convergence rate of the FE solution is to consider the exact geometry when integrating. In the embedded domain framework this can be realized by the use of a separate element-wise tetrahedralization that is used for integration purposes only. The present contribution is concerned with

the formulation and implementation of this last approach for structural shape optimization in an embedding domain setting. The methodology is based in the Cartesian grid FEM (cgFEM) successfully implemented in 2D (Nadal et al. 2013; Nadal 2014) and 3D (Marco et al. 2015, 2017b) problems. The Cartesian grid FEM relies on an explicit geometry description using parametric surfaces (i.e. NURBS or T-spline) and includes NURBS-enhanced integration techniques (Sevilla et al. 2011a, c; Marco et al. 2015) in order to consider exactly the boundary description of the embedded domain. Stabilization terms are added at the elements cut by the Dirichlet and Neumann boundaries to ensure the appropriate satisfaction of these boundary conditions to maintain the optimal convergence rate of the FE solution and to provide control of the variation of the solution in the vicinity of the boundary (Tur et al. 2015).

In order to calculate the sensitivities of the magnitudes that take part in the shape optimization analysis, a formulation for the derivation of design sensitivities in the discrete setting is used (Marco et al. 2017a). This formulation takes into account the derivatives of the stabilized formulation implemented for the imposition of boundary conditions (Tur et al. 2015).

In this paper we propose a structural shape optimization method that will benefit from the accuracy of cgFEM but also from the computational efficiency due to a data structure that allows sharing information between the different geometries analyzed during the procedure. Last, this work presents a strategy that provides an h -adapted mesh for every design without performing a complete adaptive procedure. It is based on the calculation of the sensitivities of the magnitudes present during the refinement step with respect to the design variables. This sensitivity analysis can be performed only once on a reference geometry, and then utilized to project the results of the analysis to other designs just before being analyzed. This procedure is useful for moderate shape modifications during the whole optimization process. Alternatively, the shape sensitivity analysis can be performed also for other geometries obtained during the optimization process if required. The projection of information allows to generate appropriate h -adapted meshes in one preprocess step which, compared to standard re-meshing operations, significantly reduces the computational cost of mesh generation. This method is inspired by a similar strategy that was developed and used in the context of gradient-based (Bugeda and Oliver 1993) and evolutionary (Bugeda et al. 2008) optimization methods, oriented to standard body-fitted FEM.

The paper is organized as follows: A brief review of basic features of the cgFEM methodology will be shown in Section 2, including how to take advantage of them in shape optimization. Section 3 will present the formulation used

for the structural and for the sensitivity analysis. Section 4 will be devoted to explain our strategy for h -adaptive mesh projection. Numerical results showing the behavior of the proposed technique will be presented in Section 6. This contribution ends with the conclusions in Section 7.

2 Cartesian grids for optimization

This work has been developed as a logical continuation of Marco et al. (2015) and Marco et al. (2017b) where a new FEA methodology called cgFEM was presented. This methodology was implemented in a FE code for the analysis of structural 3D components called FEAVox, where the main novelty was the ability to perform accurate numerical integration in non-conforming meshes independent of the geometry.

The foundations of mesh generation in cgFEM consists in defining an embedding domain Ω such that an open bounded domain Ω_{phys} fulfills $\Omega_{\text{phys}} \subset \Omega$. Let us assume that the embedding domain is a cube, although rectangular cuboids could also be considered. In Fig. 1 we can appreciate the embedding domain Ω interacting with Ω_{phys} .

The discretization of the embedding domain is based on a sequence of uniformly refined Cartesian meshes to mesh the domain Ω where the different levels of the Cartesian meshes are connected by predefined hierarchical relations. There are plenty of techniques that follow these principles, all of them based, in one way or another, on the *octree* concept (Meagher 1980; Jackins and Tanimoto 1980; Doctor and Torborg 1981).

The first step of the analysis consists of creating the analysis mesh taking a set of non-overlapped elements of different sizes from the different levels of the Cartesian grid pile (see Fig. 2). In order to enforce C^0 continuity between adjacent elements of different levels multipoint constraints (MPCs) (Abel and Shephard 1979; Farhat et al. 1998) are used.

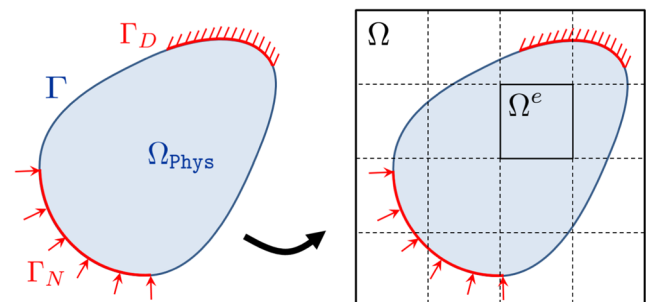


Fig. 1 Immersed Boundary Method environment. Domain Ω_{phys} within the embedding domain Ω

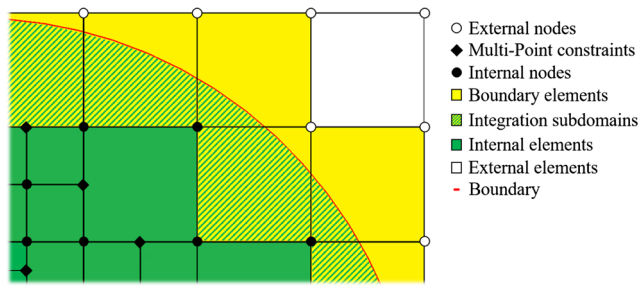


Fig. 2 Components of an immersed boundary environment

During the creation of the FE analysis mesh used to solve the boundary value problem we have to classify the elements of the Cartesian grid in three groups: boundary, internal and external elements. In order to do that, we need to know the relative position of the domain of interest with respect to the embedding domain. In Marco et al. (2015, 2017b) we proposed a robust procedure to find intersections, between the surfaces of the boundary and the axes of the Cartesian grids, based on ray-tracing techniques (Kajiya 1982; Toth 1985; Sweeney and Bartels 1986; Nishita et al. 1990; Barth and Stürzlinger 1993).

Internal elements are standard FE elements whose affinity with respect to the embedding domain Ω is exploited in order to save computational cost. Regarding the integration of intersected elements in cgFEM, we proposed a strategy in Marco et al. (2015) to perform the integration over the internal part of these elements. In order to achieve this, we generate a tetrahedralization into each boundary element. This submesh of tetrahedrons will be used only during the integration step. Numerical integration over the intersected elements is then accomplished by integrating over each subdomain of the tetrahedralization. In order to perform the integration over the subdomains, we proposed a NURBS-Enhanced integration strategy (Sevilla et al. 2011a, c)

that takes into account the exact geometry defined by the CAD model.

This methodology was designed to incorporate the exact boundary of the computational domain into body-fitted FE simulations and the advantages with respect to the classical FEM were demonstrated for a variety of problems, see Sevilla et al. (2011b).

Let Λ_{Phys} a tetrahedral face lying on the boundary parametrized by the NURBS S , and $\mathbf{v}_1, \mathbf{v}_2, \mathbf{v}_3$ its three vertices, see Fig. 3. A straight-sided triangle Λ_{Param} in the parametric space of the surface is defined by the parametric coordinates of the vertices $S^{-1}(\mathbf{v}_1), S^{-1}(\mathbf{v}_2)$, and $S^{-1}(\mathbf{v}_3)$. The transformation to a curved face, Λ_{Phys} , is defined as the image of the straight-sided triangle Λ_{Param} by the NURBS parametrization S ,

$$\Lambda_{\text{Phys}} := S(\Lambda_{\text{Param}}) \tag{2}$$

With this definition of curved faces, a curved tetrahedral subdomain with a face on a NURBS surface Υ_{Phys} corresponds to a convex linear combination of the curved NURBS face and the remaining internal vertex mapped as

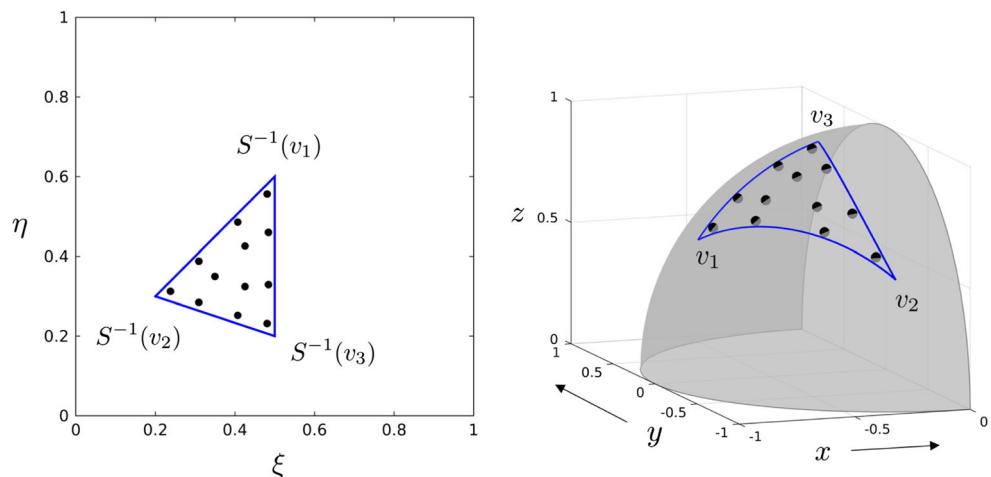
$$\Psi : \Lambda_{\text{Param}} \times [0, 1] \rightarrow \Upsilon_{\text{Phys}} \\ (\xi, \eta, \zeta) \mapsto \Psi(\xi, \eta, \zeta) := (1 - \zeta)S(\xi, \eta) + \zeta\mathbf{v}_4,$$

where \mathbf{v}_4 is the internal vertex of Υ_{Phys} .

A convenient property of this strategy are the ability to decouple the directions of the surface definition, Λ_{Param} in the mapping Ψ , with respect to the interior direction ζ .

Given these parametrizations, it is possible to perform the numerical integration over all the curved tetrahedral subdomains that form $\Omega_{\text{B}}^{\text{phys}}$. To this end, we consider tensor products of triangle quadratures for the curved faces and one-dimensional Gaussian quadratures for interior directions, see Fig. 4. Analogous steps have to be made to integrate subdomains with only one edge on the boundary (Sevilla et al. 2011a; Marco et al. 2015).

Fig. 3 Curved faces parametrization



(a) Parametric space of a NURBS.

(b) NURBS in the physical space.

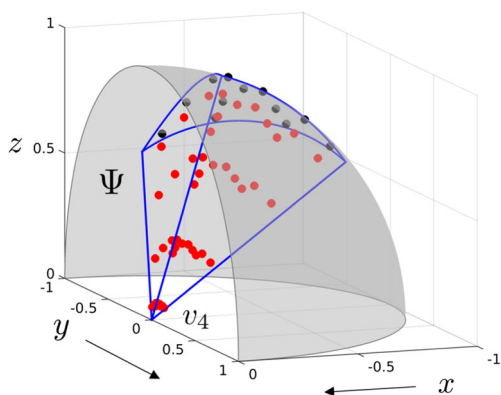


Fig. 4 Curved subdomain parametrization

2.1 Data sharing

It is worth noting that considering hierarchical relationships within the data structure suggests an automatic improvement of the mesh refinement thus positively affecting the efficiency of the FE implementation. Nodal coordinates, mesh topology, hierarchical relations, neighborhood patterns, and other geometric information are algorithmically evaluated when required.

In addition, all internal elements share the same stiffness matrix, for constant material properties and linear elasticity problems, which is only calculated once on a reference element. Then, a scale factor related to the mesh level is used to adapt the stiffness to the actual element size. As we can imagine, this implies a major increase in efficiency of the generation of the numerical model. Figure 5a shows a cross section of a model of a quarter of a cylinder, Fig. 5b presents a coarse analysis mesh and Fig. 5c shows the mesh obtained after its *h*-adaptive refinement. For both meshes we only have to evaluate one element for the domain colored in green, which represents all the internal elements.

On the other hand, each boundary element is trimmed differently, so each of these elements will require a particular evaluation of the element matrices. Contrasting with many *h*-adaptive FE codes, where the previous meshes are discarded and new ones are created, the use of Cartesian grids together with the hierarchical data structure allows recycling calculations performed in previous meshes.

In this context, the so-called *vertical data sharing* by means of which the matrices of elements present in different meshes of the same *h*-adaptive process will not be re-evaluated. Figure 5c represents the resulting mesh of a *h*-adaptive process where the blue colored elements represent elements evaluated in previous meshes that do not require to be re-evaluated. Hence, the only element matrices to be evaluated for the analysis of the mesh shown in Fig. 5c correspond to the yellow elements.

Within the context of the traditional FEM, each geometry requires a different body-fitted mesh, therefore, the elements of different geometries are, generally, completely different and unrelated. This situation makes very difficult to enable an efficient exchange of information between different geometries. However, cgFEM provides a framework to define an operation, called *horizontal data sharing*, to further improve the computational efficiency of the optimization process by allowing the transfer of information between elements of different geometries. This data sharing just requires all geometries to be defined using the same embedding domain to ensure that the Cartesian grid pile is the same for all geometries, making the inter-geometries data transfer possible.

The different components of a parametric definition of the boundary of the models to be analyzed can be subdivided into three types:

1. Fixed part. This is the part of the boundary that remains fixed in all the geometries (such as the internal curve of the cylinder represented in Fig. 6a).
2. Moving part with fixed intersection pattern. These surfaces or curves can be changed by the optimization algorithm, but those changes do not imply a modification of the intersection with the surrounding elements. In Fig. 6a we can see two curves type 2 corresponding with planes of symmetry of the model.
3. Free moving part. This part of the geometry could freely change during the optimization analysis without a predictable pattern. The outer curve in Fig. 6a is a good example of this kind of entity.

The *horizontal data sharing* consists of reusing, on the one hand, the computations attached to the elements

Fig. 5 Vertical data sharing example. a Model of a quarter of a cylinder. b Coarse mesh *i*. c *h*-adapted mesh *i* + 1

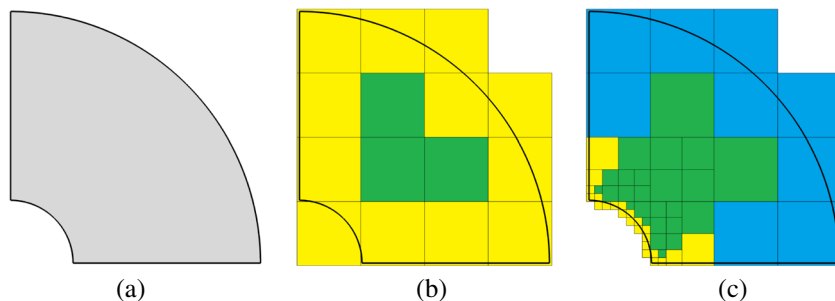
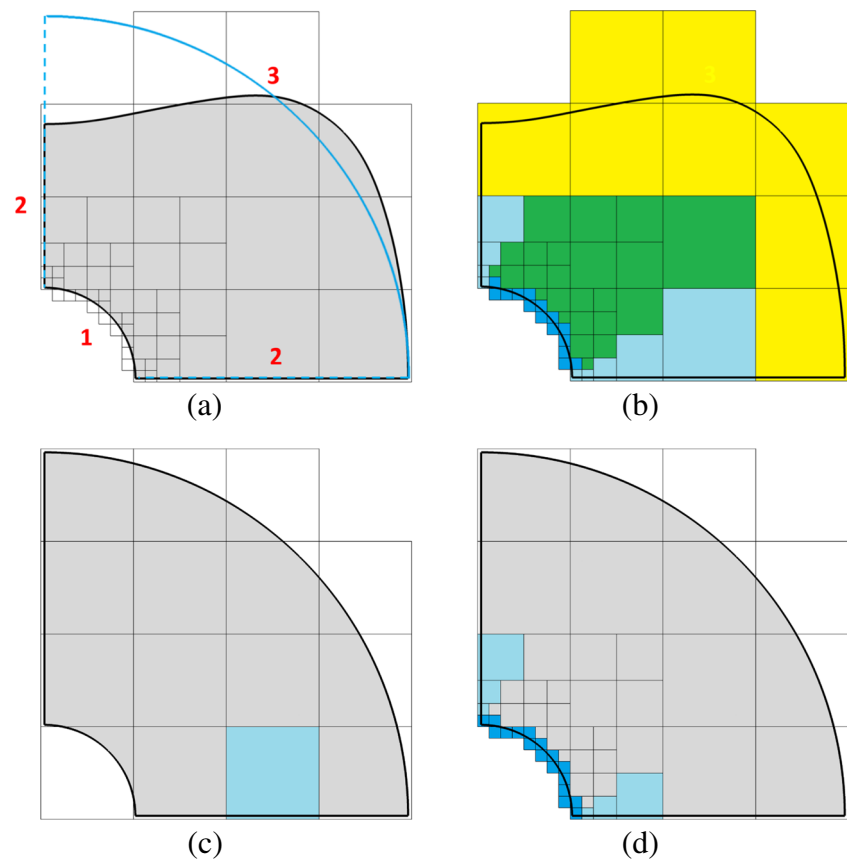


Fig. 6 Horizontal data sharing example. **a** Different type of entities. **b** Individual $j + 1$. **c** Individual j , mesh i . **d** Individual j , mesh $i + 1$



intersected by the fixed part of the boundary in the different geometries analyzed during the optimization process. For instance, in Fig. 6b we can see a h -adapted mesh of a geometry, $j + 1$, that represents a perturbation of the original model, j , shown in Fig. 5a. In Fig. 6c and d we can verify how the blue elements used in the mesh in Fig. 6b are inherited from separated meshes of a different individual evaluated previously. Dark blue represents the elements intersected by entities type 1 and light blue the elements intersected by type 2 entities. As in the *vertical sharing*, the green elements will be evaluated as explained before and the yellow elements will be the only elements evaluated for this individual. Observe that the *horizontal data sharing* implies a significant reduction of calculations. For example, the number of elements that need integration (yellow elements) for the mesh shown in Fig. 6b is considerably lower than the total number of elements in the mesh.

2.2 Nested domain reordering

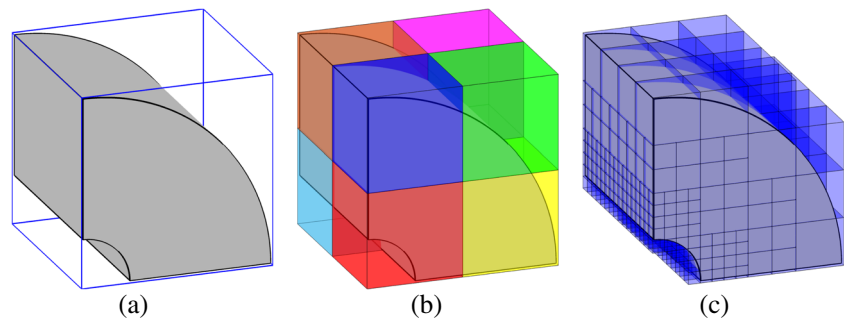
Solving large sparse linear systems is the most time-consuming computation in shape optimization using FEM.

In this contribution we are interested in direct solvers. Matrix reordering plays an important role on the performance of these solvers. In fact, it is common to reorder

the system matrix before proceeding to its factorization as it can increase the sparsity of the factorization, making the overall process faster and reducing the storage cost. Finding the optimal ordering is usually not possible although heuristic methods can be used to obtain good reorderings at a reasonable computational cost.

This section is intended to show how the hierarchical data structure inherent to the Cartesian grids, thus directly related to the mesh topology, can be used to directly obtain a reordering of the system matrix that speeds up the Cholesky factorization process. The Nested Domain Decomposition (NDD) technique is a domain decomposition technique specially tailored for h -adaptive FE analysis codes with refinement based on element subdivision. The technique simply consists in recursively subdividing the domain of the problem using the hierarchical structure of the mesh. This technique was first described in Ródenas et al. (2005) and applied in an implementation of a FEM that used geometry-conforming meshes. Later, NDD was adapted to a Cartesian grid environment in 2D (Nadal et al. 2013). In this paper, we will use a 3D generalization of NDD. The technique consist in subdividing the domain of the problem considering that each element of a uniform grid of the lowest levels of the Cartesian grid pile (normally the Level-1 grid, with $2 \times 2 \times 2$ elements). Then, the degrees of

Fig. 7 Nested Domain Decomposition environment. **a** Body within the embedding domain $\Omega_{\text{phys}} \subset \Omega$. **b** Level 1 subdivision. **c** Example of 3D mesh to be reordered



freedom of the nodes of the mesh to be analyzed falling into a subdomain will be allocated together in the stiffness matrix. The nodes falling on the interface of the subdomains will not be reordered and will simply be moved to the end of the matrix producing the typical arrowhead type structure of the domain decomposition techniques. This idea is then recursively applied into each original subdomain producing a nested arrowhead-type structure. This reordering will provide a considerable reduction of the computational cost associated to the resolution of the system of equations.

Figures 7, 8 and 9 graphically show the process. The embedding domain, Fig. 7a, is subdivided into 8 subdomains or regions as shown in Fig. 7b. Each subdomain is represented in a different color. We can easily identify those subdomains with the elements of the first refinement level. Thus, the nested reordering in cgFEM will be made up by grouping the nodes according to the corresponding element in the hierarchical structure. Figure 7c shows an example of an analysis mesh where we are going to apply the nested reordering.

For the sake of clarity we will use a 2D representation of the process. Figure 8a shows the domain subdivision considering the Level-1 grid. The nodes are subdivided into 9 different categories. Only 5 of these categories are shown in the 2D representation of Fig. 8. The colored ones indicate the nodes falling into each one of the elements of the Level-1 grid. Black nodes are those falling on the interface between the Level-1 elements. The stiffness matrix will be reordered, grouping all nodes of the same color, as shown in Fig. 9b. This grouping creates an arrowhead-type structure made up of blocks. It can be noticed that the blocks on the

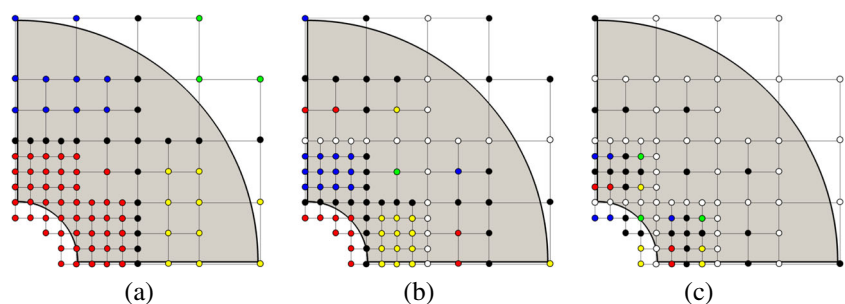
diagonal (two of them clearly shown shadowed in blue and red) show a structure similar to the structure of the original non-reordered stiffness matrix shown in Fig. 9a.

Level-2 reordering, Fig. 8b, indicates that each of the Level-1 subdomains is again reordered in the same way. For instance, the red subdomain in Fig. 8a is subdivided into 8 subdomains (only 4 are shown in 2D) separated by their interface, represented in black, as shown in Fig. 8b. Interfaces of previous levels are represented by white nodes. The same process is followed for the next levels, using the elements of the corresponding level of the hierarchical structure.

In the process, each node of the mesh is given a code with as many digits as levels of the Cartesian grid pile used. The i -th digit of the code contains the subdomain number (1 to 8) of the node considering the Level- i grid, or 9 if the node is on the interface between the Level- i subdomains, as in Fig. 8a to c for levels 1 to 3. Once the code of each node has been obtained a simple ‘alphabetical’ reordering of the codes provides the NDD reordering of the nodes. The degrees of freedom of the matrix will then be reordered considering nodal reordering.

The result of the NDD reordering generates the nested arrowhead-type structure of the stiffness matrix represented in Fig. 9c. This nested structure could also be used to define efficient domain decomposition solvers or iterative solvers, as in Ródenas et al. (2007a) where we showed initial implementations of these types of solvers. However, in this contribution we have just used this technique to reorder the system of equations to improve the performance of the Cholesky factorization.

Fig. 8 Nested Domain Decomposition scheme. **a** Level-1 decomposition, **b** Level-2 decomposition and **c** Level-3 decomposition



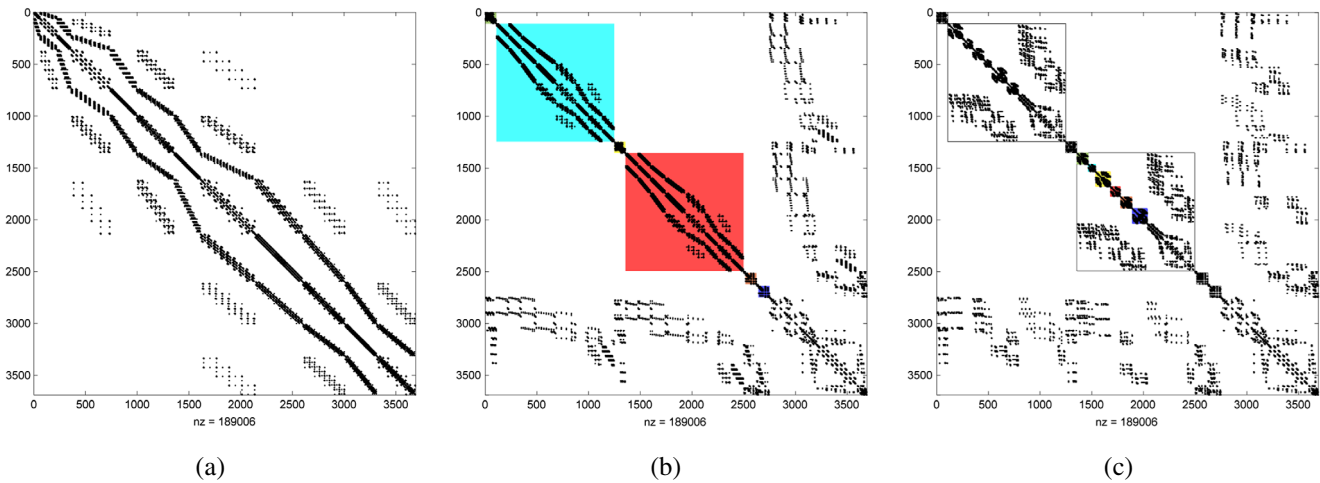


Fig. 9 Nested Domain Decomposition output. **a** Original stiffness matrix, **b** level 1 decomposition reordering and **c** last reordering (level 4)

In the section devoted to numerical examples we will show the performance of this method in comparison with other common procedures.

3 Structural and shape sensitivity analyzes

Let us consider a bounded domain $\Omega_{\text{phys}} \in \mathbb{R}^3$. The contour can be separated into two non-overlapping parts, Γ_N and Γ_D , where Neumann and Dirichlet conditions are respectively imposed. The objective is to find the displacement field $\mathbf{u} \in \mathcal{U}$ that satisfy the internal equilibrium equation in the domain and the Neumann and Dirichlet boundary conditions, which can be formulated as follows:

$$\begin{aligned} \nabla \sigma(\mathbf{u}) + \mathbf{t}_v &= 0 & \text{in } \Omega_{\text{phys}} \\ \sigma(\mathbf{u}) \mathbf{n} &= \mathbf{t}_s & \text{on } \Gamma_N \\ \mathbf{u} &= \mathbf{g} & \text{on } \Gamma_D \\ \epsilon(\mathbf{u}) &= \mathbf{D} \sigma(\mathbf{u}) \end{aligned} \quad (3)$$

In the above expression displacements \mathbf{u} belong to $\mathcal{U} \equiv [H^1(\Omega)]^d$, $\mathbf{t}_v \in [L^2(\Omega)]^d$ are the volume forces, $\mathbf{t}_s \in [L^2(\Omega)]^d$ are the tractions imposed on the Neumann boundary, \mathbf{g} are the displacements imposed on the Dirichlet boundary and \mathbf{n} is the unit vector normal to the surface. In linear elasticity, the strain tensor is defined from the displacement field by

$$\epsilon(\mathbf{u}) = (\nabla \mathbf{u} + \nabla^T \mathbf{u})/2 \quad (4)$$

The constitutive equation, that relates the stresses with the strains by means of the tensor \mathbf{D} , can be expressed in the case of isotropic materials using two constants, the Young's

modulus E and the Poisson's ratio ν . This relationship can be written as

$$\epsilon = (\sigma - \nu (\text{tr}(\sigma) \mathbf{I} - \sigma)) / E \quad (5)$$

The next property concerning the constitutive equation will be used below.

Proposition 1 *The scalar product of the tractions can be bounded by the energy per unit volume with a constant C_E , which depends on the material properties, as*

$$\|(\mathbf{u})\|^2 \leq C_E ((\mathbf{u}) : \epsilon(\mathbf{u})) \quad \text{with } C_E = \frac{E}{1 - 2\nu} \quad (6)$$

The weak form of the elastic problem allows different approaches to imposing the Dirichlet boundary conditions. The most usual procedure is to impose a constraint in the space of virtual displacement \mathcal{V} . The equilibrium between the virtual work of the elastic forces and the virtual work of the external forces applied is written as:

$$a(\mathbf{u}, \mathbf{v}) = c(\mathbf{v}) \quad \forall \mathbf{v} \in \mathcal{V} \quad (7)$$

where

$$\begin{aligned} a(\mathbf{u}, \mathbf{v}) &= \int_{\Omega_{\text{phys}}} \sigma(\mathbf{u}) : \epsilon(\mathbf{v}) \, d\Omega \\ c(\mathbf{v}) &= \int_{\Omega_{\text{phys}}} \mathbf{v} \cdot \mathbf{t}_v \, d\Omega + \int_{\Gamma_N} \mathbf{v} \cdot \mathbf{t}_s \, d\Gamma \end{aligned} \quad (8)$$

This method is very simple and effective in the context of the body-fitted FEM. However, this method is not valid for Cartesian meshes, since it is very difficult to get a null field on the Dirichlet boundary when the contour of the geometry does not match with the faces of the elements.

For this reason it seems more appropriate to use another formulation, for instance, to define the elastic problem as a minimization problem with constraints. This means finding the displacement field \mathbf{u} that minimizes the total potential

energy, subject to the constraints imposed by Dirichlet boundary conditions:

$$\min \left(\frac{1}{2}a(\mathbf{v}, \mathbf{v}) - c(\mathbf{v}) \right) \quad \text{with } \mathbf{v} = \mathbf{g} \text{ in } \Gamma_D \quad (9)$$

One approach to solve this minimization problem is to use the Lagrange multipliers method. Besides the displacement field, a new field of Lagrange multipliers λ associated with the reaction forces is added. The Lagrange multipliers belong to the Hilbert space $\mathcal{M} = [H^{-\frac{1}{2}}(\Gamma_D)]^d$. Formally, the problem is to find the saddle point $[\mathbf{u}, \lambda] \in \mathcal{U} \times \mathcal{M}$ of the following Lagrangian

$$\mathcal{L}(\mathbf{v}, \mu) = \frac{1}{2}a(\mathbf{v}, \mathbf{v}) + b(\mu, \mathbf{v} - \mathbf{g}) - c(\mathbf{v}) \quad \text{with } b(\mu, \mathbf{u}) = \int_{\Gamma_D} \mu \cdot \mathbf{u} \, d\Gamma \quad (10)$$

The spaces of the finite element solution are denoted as $\mathcal{U}^h \subset \mathcal{U}$ for displacements and $\mathcal{M}^h \subset \mathcal{M}$ for multipliers. Substituting the FE fields in (10) and optimizing the Lagrangian we obtain the following system of equations:

$$\begin{aligned} a(\mathbf{u}^h, \mathbf{v}^h) + b(\lambda^h, \mathbf{v}^h) &= c(\mathbf{v}^h) \quad \forall \mathbf{v}^h \in \mathcal{U}^h \\ a(\mu^h, \mathbf{u}^h) &= b(\mu^h, \mathbf{g}) \quad \forall \mu^h \in \mathcal{M}^h \end{aligned} \quad (11)$$

where \mathbf{v}^h and μ^h are the variations of the displacement and multiplier fields and $[\mathbf{u}^h, \lambda^h]$ is the solution.

In practice, the natural choices of the Lagrange multiplier field do not satisfy *InfSup* condition because they introduce too many constraints. To alleviate this situation, stabilized methods impose additional conditions on the Lagrange multipliers without modifying the problem solution, at least in the limit, when the element size approaches zero, in order to have more freedom to choose the Lagrange multiplier field.

In this contribution we use an approach derived from the following Lagrangian:

$$\mathcal{L}_S(\mathbf{v}^h, \mu^h) = \mathcal{L}(\mathbf{v}^h, \mu^h) - \frac{1}{2} \frac{h}{k} \int_{\Gamma_D} \left\| \mu^h + \mathbf{T}(\hat{\mathbf{u}}^h) \right\|^2 \, d\Gamma \quad (12)$$

where $\mathbf{T}(\hat{\mathbf{u}}^h)$ is the smoothed traction that depends on the FE solution computed from a previous iteration, $\hat{\mathbf{u}}^h$. The penalty constant can be defined again as $k = \kappa C_E$.

In our formulation we use the recovered tractions on Γ_D evaluated from the recovered stress field σ^* (Ródenas et al. 2007b) to stabilize, solving the problem iteratively by updating the stress field value (Tur et al. 2014, 2015), $\sigma^*(\hat{\mathbf{u}}^h)$ the FE recovered stress field being calculated for an FE solution from a previous iteration $\hat{\mathbf{u}}^h$. The traction on the boundary is defined as $\mathbf{T}(\hat{\mathbf{u}}^h) = \sigma^*(\hat{\mathbf{u}}^h) \cdot \mathbf{n}$ where \mathbf{n} is the unit vector normal to the boundary.

The proposed formulation can be simplified by eliminating the Lagrange multipliers and obtaining a modified penalty method: Find the displacement field $\mathbf{u}^h \in \mathcal{U}^h$ such that

$$\begin{aligned} a(\mathbf{u}^h, \mathbf{v}^h) + \frac{k}{h} \int_{\Gamma_D} \mathbf{u}^h \cdot \mathbf{v}^h \, d\Gamma = \\ c(\mathbf{v}^h) + \frac{k}{h} \int_{\Gamma_D} \mathbf{g} \cdot \mathbf{v}^h + \int_{\Gamma_D} \mathbf{T}(\hat{\mathbf{u}}^h) \cdot \mathbf{v}^h \, d\Gamma \quad \forall \mathbf{v}^h \in \mathcal{U}^h \end{aligned} \quad (13)$$

The second term on the left hand side of (13) is a penalty term with a constant k/h . The last term on the right hand side is the virtual work of reaction forces. This term acts as a correction of the penalty term and is necessary for the FE solution to converge to the exact solution when the mesh is refined.

The problem (13) can be written in matrix form as:

$$\bigcup_{e=1}^{n_e} (\mathbf{k}^e + \mathbf{k}_D^e) \{ \mathbf{u}^e \} = \bigcup_{e=1}^{n_e} (\mathbf{f}_q^e + \mathbf{f}_g^e + \mathbf{f}_s^e) \quad (14)$$

The stiffness matrix of each element is computed by

$$\mathbf{k}^e = \int_{\Omega_{\text{Phys}}^e} \mathbf{B}^T \mathbf{D} \mathbf{B} |\mathbf{J}| \, d\Omega \quad (15)$$

where

Ω_{Phys}^e is the domain in local element coordinates, \mathbf{B} is the nodal strains-displacements matrix, \mathbf{D} is the elasticity tensor, $|\mathbf{J}|$ is the determinant of the matrix \mathbf{J} , which represents the Jacobian matrix of transformation of the global coordinates (x, y, z) to the local element coordinates (ξ, η, τ) .

The vector \mathbf{f}_q is the standard FE vector due to point forces, volumetric forces, forces distributed over the Neumann surface of the element, evaluated assembling the contribution \mathbf{f}_q^e of every element e on the domain:

$$\mathbf{f}_q^e = \int_{\Gamma_N^e} \mathbf{N}^T \mathbf{t} |\mathbf{J}| \, d\Gamma + \int_{\Omega^e} \mathbf{N}^T \mathbf{b} |\mathbf{J}| \, d\Omega \quad (16)$$

where vectors \mathbf{t} and \mathbf{b} correspond to the surface and body loads, respectively.

The global stiffness matrix is obtained by the contribution of the classical stiffness matrix of each element \mathbf{k}^e and a stabilization term \mathbf{k}_D^e for all the boundary elements containing the Dirichlet boundary.

The stabilization term is computed as:

$$\mathbf{k}_D^e = \int_{\Gamma_D^e} \frac{\kappa^*}{h} \mathbf{C}^T \mathbf{C} |\mathbf{J}| \, d\Gamma \quad (17)$$

where

Γ_D^e is the portion of the Dirichlet boundary within the element,

κ^* is the penalty constant, being $\kappa^* = \kappa \cdot C_E$ and $\kappa > 0$, h is the element size,

\mathbf{C} is the matrix of finite element interpolation if Dirichlet conditions are applied on the three displacement components x , y and z .

$$\mathbf{C} = \mathbf{N} = \begin{bmatrix} N_1 & 0 & 0 & N_2 & 0 & 0 & N_3 & 0 & 0 & \dots & N_{nnod} & 0 & 0 \\ 0 & N_1 & 0 & 0 & N_2 & 0 & 0 & N_3 & 0 & \dots & 0 & N_{nnod} & 0 \\ 0 & 0 & N_1 & 0 & 0 & N_2 & 0 & 0 & N_3 & \dots & 0 & 0 & N_{nnod} \end{bmatrix}$$

with $nnod$ as the number of nodes per element. Otherwise $\mathbf{C} = \mathbf{S}\mathbf{N}$, where $\mathbf{S}_{ii} = \sum_d \delta_{id}$ would be a diagonal matrix, d is the direction in which Dirichlet boundary conditions are applied and δ is the Dirac delta function.

On the other side of the equation, the equivalent force vector \mathbf{f} is evaluated by adding the contribution of the standard FE vector of equivalent forces on nodes \mathbf{f}_q , the stabilization term of the Dirichlet boundary \mathbf{f}_g and the stabilizing stress component \mathbf{f}_s .

The vector \mathbf{f}_g is due to the non-homogeneous Dirichlet condition $\mathbf{u}^h = \mathbf{g}$ on Γ_D and it is evaluated assembling the contribution of every element on the Dirichlet boundary:

$$\mathbf{f}_g^e = \int_{\Gamma_D^e} \frac{\kappa^*}{h} \mathbf{C}^T g |\mathbf{J}| d\Gamma \quad (18)$$

Finally, \mathbf{f}_s is the stabilizing term which depends on the stress field. As mentioned above, in our formulation we use the recovered tractions on Γ_D evaluated from the recovered stress field $\sigma^*(\hat{\mathbf{u}}^h)$. The traction on the boundary is defined as $\mathbf{T}(\hat{\mathbf{u}}^h) = \sigma^*(\hat{\mathbf{u}}^h) \cdot \mathbf{n}$ where \mathbf{n} is the unit vector normal to the boundary, then

$$\mathbf{f}_s^e = \int_{\Gamma_D^e} \mathbf{C}^T \mathbf{T}(\hat{\mathbf{u}}^h) |\mathbf{J}| d\Gamma \quad (19)$$

Regarding the structural sensitivity analysis, the differentiation of the previous system of equations is needed, including the components due to the imposition of the boundary conditions. In this work we use a formulation presented in Marco et al. (2017a) that is based on the analytical discrete method (Kibsgaard 1992; Poldneff et al. 1993; Pandey and Bakshi 1999; Moita et al. 2000) consisting in obtaining analytical expressions of the sensitivities of the external forces and stiffness matrix. A detailed review and comparison of the different sensitivity analysis approaches can be found in van Keulen et al. (2005).

The derivative of (14) with respect to any design variable a_m allows to obtain the sensitivity of the calculation

$$\begin{aligned} \bigcup_{e=1}^{n_e} \left(\left(\frac{\partial \mathbf{k}^e}{\partial a_m} + \frac{\partial \mathbf{k}_D^e}{\partial a_m} \right) \{ \mathbf{u}^e \} + (\mathbf{k}^e + \mathbf{k}_D^e) \left\{ \frac{\partial \mathbf{u}^e}{\partial a_m} \right\} \right) \\ = \bigcup_{e=1}^{n_e} \left(\frac{\partial \mathbf{f}_q^e}{\partial a_m} + \frac{\partial \mathbf{f}_g^e}{\partial a_m} + \frac{\partial \mathbf{f}_s^e}{\partial a_m} \right) \quad (20) \end{aligned}$$

First, starting with \mathbf{K} and considering that the derivative of material properties matrix, \mathbf{D} , with respect to design variables is zero

$$\begin{aligned} \frac{\partial \mathbf{k}^e}{\partial a_m} = \int_{\Omega_{\text{phys}}^e} \left[\frac{\partial \mathbf{B}^T}{\partial a_m} \mathbf{D} \mathbf{B} + \mathbf{B}^T \mathbf{D} \frac{\partial \mathbf{B}}{\partial a_m} \right] |\mathbf{J}| d\Omega \\ + \int_{\Omega_{\text{phys}}^e} \left[\mathbf{B}^T \mathbf{D} \mathbf{B} \frac{\partial |\mathbf{J}|}{\partial a_m} \right] d\Omega \quad (21) \end{aligned}$$

where

$\frac{\partial |\mathbf{J}|}{\partial a_m}$, $\frac{\partial \mathbf{B}}{\partial a_m}$ are the sensitivities of $|\mathbf{J}|$ and \mathbf{B} with respect to the design variable a_m , which are functions of the velocity field, \mathbf{V}_m , that represents the partial derivatives of the location of material points, \mathbf{P} , with respect to the design variables: $\mathbf{V}_m = \frac{\partial \mathbf{P}}{\partial a_m}$ (see Marco et al. (2017a)).

The derivatives of the terms introduced by the stabilization method will be

$$\begin{aligned} \frac{\partial \mathbf{k}_D^e}{\partial a_m} &= \int_{\Gamma_D^e} \frac{\kappa^*}{h} \mathbf{C}^T \mathbf{C} \frac{\partial |\mathbf{J}|}{\partial a_m} d\Gamma \\ \frac{\partial \mathbf{f}_g^e}{\partial a_m} &= \int_{\Gamma_D^e} \frac{\kappa^*}{h} \mathbf{C}^T g \frac{\partial |\mathbf{J}|}{\partial a_m} d\Gamma \\ \frac{\partial \mathbf{f}_s^e}{\partial a_m} &= \int_{\Gamma_D^e} \left[\mathbf{C}^T \frac{\partial \mathbf{T}(\hat{\mathbf{u}}^h)}{\partial a_m} |\mathbf{J}| + \mathbf{C}^T \mathbf{T}(\hat{\mathbf{u}}^h) \frac{\partial |\mathbf{J}|}{\partial a_m} \right] d\Gamma \quad (22) \end{aligned}$$

As mentioned above, the traction on the boundary is defined as $\mathbf{T}(\hat{\mathbf{u}}^h) = \sigma^*(\hat{\mathbf{u}}^h) \cdot \mathbf{n}$ where \mathbf{n} is the unit vector normal to the boundary and its derivative can be written as

$$\frac{\partial \mathbf{T}(\hat{\mathbf{u}}^h)}{\partial a_m} = \frac{\partial \sigma^*}{\partial a_m} \mathbf{n} + \sigma^* \frac{\partial \mathbf{n}}{\partial a_m} \quad (23)$$

3.1 Objective function and constraints

Although we will consider that the objective function is the volume, other magnitudes could also be considered. This volume can be obtained adding the volume of each finite element present in the mesh, computed as:

$$V = \bigcup_{e=1}^{n_e} V_e = \bigcup_{e=1}^{n_e} \int_{\Omega_{\text{phys}}^e} d\Omega = \bigcup_{e=1}^{n_e} \int_{\Omega_{\text{phys}}^e} |\mathbf{J}| d\xi d\eta d\tau \quad (24)$$

Applying the techniques discussed above to differentiate the components of the system of equations we obtain

$$\frac{\partial V_e}{\partial a_m} = \int_{\Omega_{\text{phys}}^e} \frac{|\mathbf{J}|}{\partial a_m} d\xi d\eta d\tau \quad (25)$$

where $\frac{|\mathbf{J}|}{\partial a_m}$ can be evaluated as:

$$\begin{aligned} \frac{|\mathbf{J}|}{\partial a_m} = |\mathbf{J}| \cdot \text{trace} \left(\mathbf{J}^{-1} \frac{\mathbf{J}}{\partial a_m} \right) \quad \text{with} \quad \frac{\mathbf{J}}{\partial a_m} \\ = \sum_i^{nnod} \left\{ \begin{matrix} N_{i,\xi} \\ N_{i,\eta} \\ N_{i,\tau} \end{matrix} \right\} \frac{\partial}{\partial a_m} \{ x_i, y_i, z_i \} \quad (26) \end{aligned}$$

In the last expression, all the components are evaluated as part of the shape sensitivity analysis. The derivatives of the nodal coordinates with respect to the design variables are the so-called velocity field whose definition will be given in Section 4.

In our study, the constraints are expressed in terms of stresses. To evaluate the stresses we consider the general expression for the calculation of stresses in continuous isoparametric elements

$$\boldsymbol{\sigma} = \mathbf{DB}\mathbf{u}_h^e \tag{27}$$

\mathbf{u}_h^e being the vector of nodal displacements of element e . Taking the derivative with respect to the design variable a_m , it yields

$$\frac{\partial \boldsymbol{\sigma}}{\partial a_m} = \mathbf{DB} \frac{\partial \mathbf{u}^e}{\partial a_m} + \mathbf{D} \frac{\partial \mathbf{B}}{\partial a_m} \mathbf{u}^e \tag{28}$$

where all terms on the right can be evaluated using the development of the preceding sections. Once we have evaluated both $\boldsymbol{\sigma}$ and $\frac{\partial \boldsymbol{\sigma}}{\partial a_m}$ we can apply the construction of the smoothing field based on a recovery technique shown in (Ródenas et al. 2007b).

3.2 Error estimator

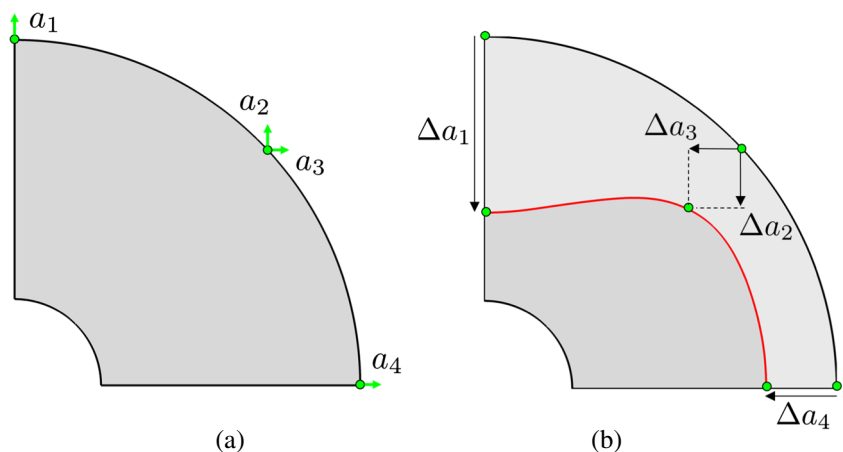
The error associated with the FE discretization is evaluated in this work using the Zienkiewicz and Zhu (1987) error estimator as:

$$\|\mathbf{e}_{es}\|^2 = \int_{\Omega_{\text{Phys}}} (\boldsymbol{\sigma}_h - \boldsymbol{\sigma}^*)^T \mathbf{D}^{-1} (\boldsymbol{\sigma}_h - \boldsymbol{\sigma}^*) d\Omega \tag{29}$$

where $\boldsymbol{\sigma}^*$ is a smoothed continuous stress field obtained by the recovery technique (Ródenas et al. 2007b; Nadal et al. 2013). The resulting expression for the sensitivity analysis of the error estimator already presented in Bugeda and Oliver (1993):

$$\frac{\partial \|\mathbf{e}_{es}\|^2}{\partial a_m} = \int_{\Omega_{\text{Phys}}} (\boldsymbol{\sigma}_h - \boldsymbol{\sigma}^*)^T \mathbf{D}^{-1} \left(2 \left(\frac{\partial (\boldsymbol{\sigma}_h - \boldsymbol{\sigma}^*)}{\partial a_m} \right) + \frac{(\boldsymbol{\sigma}_h - \boldsymbol{\sigma}^*)}{|\mathbf{J}|} \frac{\partial |\mathbf{J}|}{\partial a_m} \right) |\mathbf{J}| d\Omega \tag{30}$$

Fig. 10 Design evolution during optimization. **a** Reference design (j). **b** Perturbed design ($j + 1$)



where $\frac{\partial (\boldsymbol{\sigma}_h - \boldsymbol{\sigma}^*)}{\partial a_m}$ will be approximated as follows:

$$\frac{\partial (\boldsymbol{\sigma}_h - \boldsymbol{\sigma}^*)}{\partial a_m} = \frac{\partial \boldsymbol{\sigma}_h}{\partial a_m} - \left(\frac{\partial \boldsymbol{\sigma}}{\partial a_m} \right)^* \tag{31}$$

being $\left(\frac{\partial \boldsymbol{\sigma}}{\partial a_m} \right)^*$ obtained through the same recovery procedure applied previously to $\boldsymbol{\sigma}^*$.

Equation (30) was also derived in Fuenmayor et al. (1997) for the definition of an estimator for the discretization error in shape sensitivity analysis. In order to use an h -refinement strategy, it will also be necessary to compute the energy norm and its sensitivity with respect to each design variable. This can be evaluated considering:

$$\|\mathbf{u}_{es}\|^2 \approx \mathbf{u}^T \mathbf{K} \mathbf{u} + \|\mathbf{e}_{es}\|^2 \tag{32}$$

$$\frac{\partial \|\mathbf{u}_{es}\|^2}{\partial a_m} \approx \frac{\partial \mathbf{u}^T}{\partial a_m} \mathbf{K} \mathbf{u} + \mathbf{u}^T \frac{\partial \mathbf{K}}{\partial a_m} \mathbf{u} + \mathbf{u}^T \mathbf{K} \frac{\partial \mathbf{u}}{\partial a_m} + \frac{\partial \|\mathbf{e}_{es}\|^2}{\partial a_m} \tag{33}$$

4 Automatic h -adaptive mesh projection

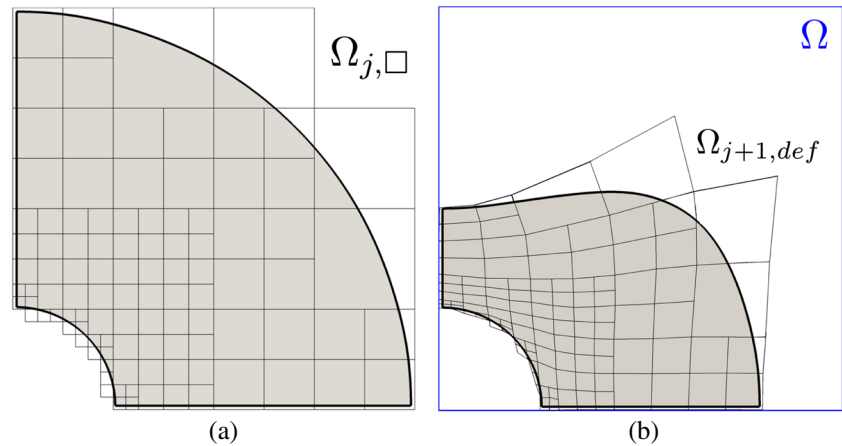
In this contribution we use a gradient-based algorithm (Nocedal and Wright 2006) which uses first-order sensitivities of the objective function and constraints to evaluate the solution of (1). Using this information and the values of the design variables for the j -th geometry obtained during the iterative process (\mathbf{a}^j), see Fig. 10a, the algorithm generates the modified values of \mathbf{a}^j defining an improved design (\mathbf{a}^{j+1}) using

$$\mathbf{a}^{j+1} = \mathbf{a}^j + \alpha \mathbf{S}(\mathbf{a})^j \tag{34}$$

where $\mathbf{S}(\mathbf{a})^j$ is the search direction vector and α is the step-length parameter.

After the definition of the ($j + 1$)-th geometry to be analyzed, see Fig. 10b, it is necessary to construct the

Fig. 11 Mesh projection procedure. **a** Cartesian reference analysis mesh, $\Omega_{j,\square}$. **b** Projected (non-Cartesian) mesh on geometry $j + 1$



new analysis mesh. There have been previous developments about this using standard body-fitted FE meshes (Bugeda and Oliver 1993; Bugeda et al. 2008). In these references the information required to define a new mesh was projected from one geometry to another making use of the following expression:

$$M_{j+1} \approx M_j + \sum \left(\frac{\partial M_j}{\partial a_m} \right) \cdot \Delta a_m \quad (35)$$

where M represents any magnitude that has to be projected from geometry j to geometry $j + 1$. The generation of an h -adapted mesh used in these references was based on the use of a mesh optimality criterion, in these cases the criterion used was the minimization of the number of elements in the mesh to be created that would produce the prescribed estimated error in energy norm. This criterion is equivalent to the equidistribution of the error in energy norm on the elements of the new mesh (Fuenmayor and Oliver 1996). In the following, we use a 3D generalization of this criterion presented in Marco et al. (2017b).

Let's assume that $\Omega_{j,def}$ is mesh n of an h -adaptive analysis that corresponds to the geometry $j + 1$ and we want

to evaluate mesh $n + 1$ (the new mesh) of the h -adaptive sequence, then:

$$h_{e,n}^{n+1} \approx h_e^n \left[\frac{1}{M^n} \right]^{1/2(p+1)} \left[\frac{\|\mathbf{e}^{n+1}\|}{\|\mathbf{e}^n\|} \right]^{\frac{d}{2p^2+pd}} \left[\frac{\|\mathbf{e}^{n+1}\|}{\|\mathbf{e}^n\|} \right]^{\frac{2}{2p+d}} \quad (36)$$

where

h_e^n is the size of the element e of the mesh n ,
 $h_{e,n}^{n+1}$ is the new element size of the mesh $n + 1$ obtained by the subdivision of element e in the mesh n ,
 M^n is the number of elements in the mesh n ,
 $\|\mathbf{e}^{n+1}\|$ is the global error in energy norm of the mesh $n + 1$,
 $\|\mathbf{e}^n\|$ is the global error in energy norm of the mesh n ,
 $\|\mathbf{e}^n\|_e$ is the error of the element e of the mesh n ,
 p is the polynomial degree of the shape functions used,
 d is the dimension of the problem (2 for 2D, 3 for 3D problems).

To use this expression we have to replace $\|\mathbf{e}^n\|_e$ in (36) by the projection given in (39), evaluate $\|\mathbf{e}^n\|$ as the

Fig. 12 Mesh projection procedure. **a** Perturbed integration points. **b** Projected Cartesian mesh, $\Omega_{j+1,\square}$

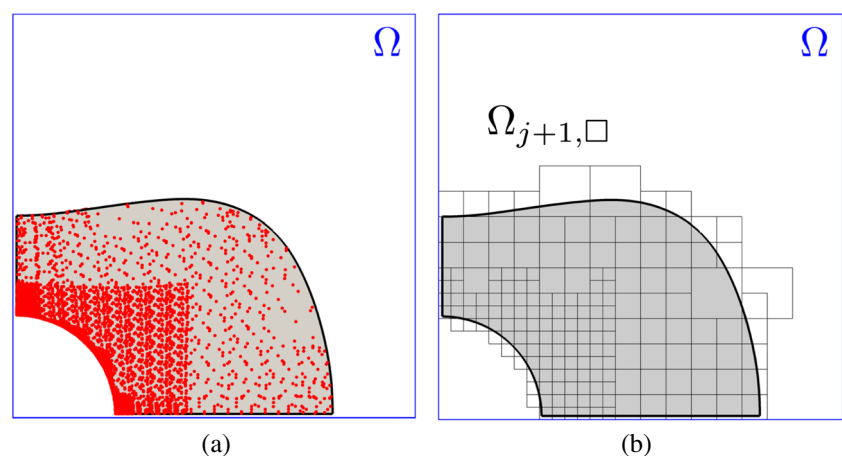
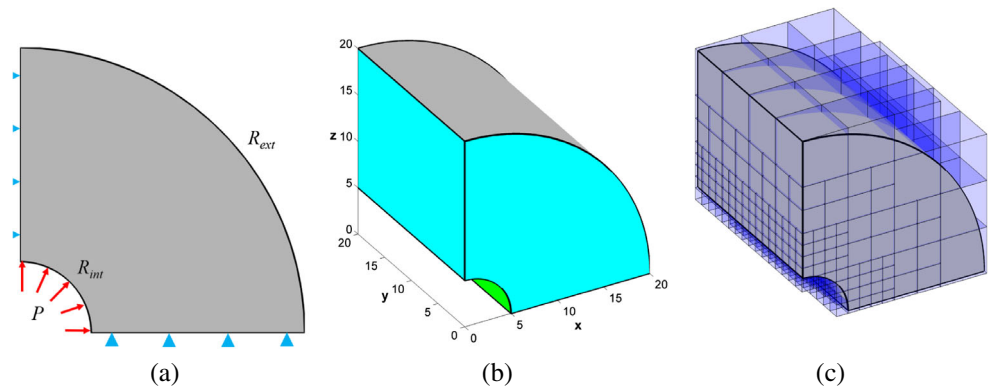


Fig. 13 Model of a cylinder under internal pressure. **a** Front view with boundary conditions. **b** 3D model representation. **c** Example of analysis mesh



summation of all the projected errors in elements from (39), and evaluate $\|\mathbf{e}^{n+1}\|$ as

$$\|\mathbf{e}^{n+1}\| = \frac{\gamma}{100} \|\mathbf{u}_{es}^{j+1}\| \tag{37}$$

where γ is the prescribed percentage of relative error in energy norm and $\|\mathbf{u}_{es}^{j+1}\|$ is the global projected energy norm.

Hence, once a new design has been defined, the projection starts with the previous analysis mesh, defined as $\Omega_{j,\square}$ in Fig. 11a, using the previously computed coordinate sensitivities. The projected position \mathbf{r}^{j+1} for each node of the mesh is given by:

$$\mathbf{r}^{j+1} = \mathbf{r}^j + \sum_m (a_m^{j+1} - a_m^j) \left(\frac{\partial \mathbf{r}^j}{\partial a_m^j} \right) \tag{38}$$

Remark 1 The velocity field at nodes, $\frac{\partial \mathbf{r}}{\partial a_m}$, can be defined using different strategies. As the location of the nodes of the Cartesian grid is always maintained we could simply consider that $\frac{\partial \mathbf{r}}{\partial a_m} = 0$ in the internal nodes, thus $\frac{\partial \mathbf{r}}{\partial a_m} \neq 0$ only on the varying portion of the boundary. However, this velocity field is not suitable for projection purposes since we need information also in the internal elements to be able to properly deform the mesh. To overcome this, we design a velocity field based on the physical approach (Belegundu et al. 1991) were we solve a FE problem in the entire domain imposing, as displacements, the velocity field calculated on the boundary. The resulting displacement field is then interpreted as the required velocity field. This kind of procedure is more expensive than the first one, but this

computational cost can be alleviated by using a coarse FE mesh for projecting the information.

Likewise, the estimated error in energy norm and the estimated energy norm at each element required in (36) can also be estimated by projection using the expressions

$$\|\mathbf{e}_{es}\|_{e,j+1}^2 \approx \|\mathbf{e}_{es}\|_{e,j}^2 + \sum_m (a_m^{j+1} - a_m^j) \frac{\partial \|\mathbf{e}_{es}\|_e^2}{\partial a_m} \tag{39}$$

$$\|\mathbf{u}_{es}\|_{e,j+1}^2 \approx \|\mathbf{u}_{es}\|_{e,j}^2 + \sum_m (a_m^{j+1} - a_m^j) \frac{\partial \|\mathbf{u}_{es}\|_e^2}{\partial a_m} \tag{40}$$

These projections give an approximation to the values of the estimated error in energy norm and the energy norm that would be obtained if the next design were computed with the previous Cartesian mesh $\Omega_{j,\square}$ projected to the new geometry, represented as $\Omega_{j+1,def}$ in Fig. 11b.

As in a standard remeshing procedure, we have an h -adapted mesh for geometry $j + 1$ and, thanks to the extrapolation procedure, the values of energy norm and its estimated error at each element. Hence, without any further computation on geometry $j + 1$, the projected estimated error and energy norm allow us to estimate the quality of the results that would be obtained through the FE analysis of geometry $j + 1$ with a mesh (Fig. 11b) equivalent to the one used in the previous design j (Fig. 11a). If the target error prescribed for the FE analysis is lower than the projected error of the $(j + 1)$ -th geometry, the mesh must be h -refined using (36).

Up to this point, the mesh projection presented is comparable to the strategies used for body-fitted meshes (Bugeda

Fig. 14 2D view of 3D uniform meshes with different element size

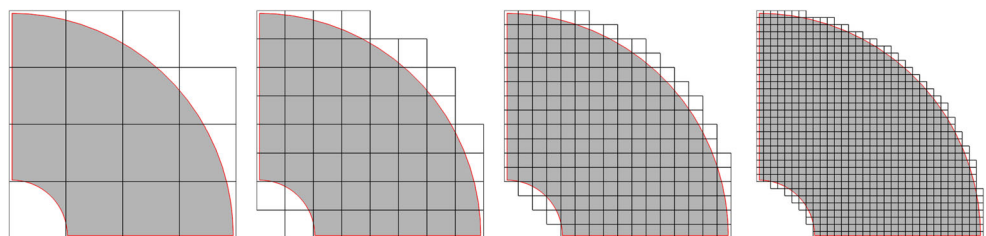
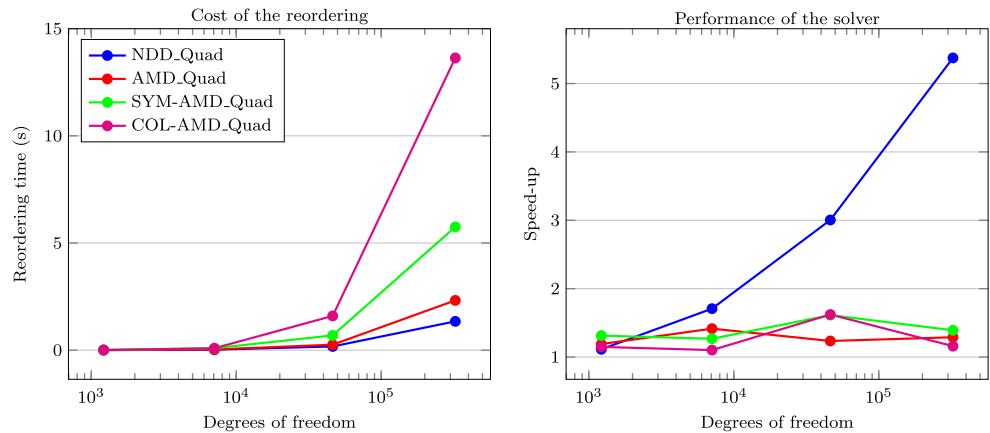


Fig. 15 Behavior of different reordering techniques. Left: reordering times. Right: speed-up in the solution of the system of equations with respect to the reference (no reordering)



and Oliver 1993; Bugada et al. 2008). As we can easily observe in Fig. 11b, this kind of projection yields in a discretization that is not compatible with the hierarchical Cartesian structure of cgFEM, thus losing most of the advantages related to its use.

In this paper we propose a projection strategy that will allow to generate an h -adapted analysis mesh of the new design $j + 1$ keeping the Cartesian structure intact.

This strategy simply requires to project the element size, evaluated using (36) for the elements of $\Omega_{j+1,def}$ (Fig. 11b), to the embedding domain Ω . To do this we assign this element size to the Gauss points of each element and project all the integration points of $\Omega_{j+1,def}$ to Ω . These projected integration points containing element size information can be trivially located into the elements of a uniform Cartesian grid of the prescribed level. Then these Cartesian elements are recursively refined until the size of each element is smaller than the minimum element sizes defined by the Gauss points contained in the element, leading to an h -adapted Cartesian grid (see Fig. 12b).

From this perspective, projection, through sensitivity analysis, can transform a posteriori error estimation into a preprocess tool able to generate an h -adapted mesh for the new design, recycling calculations obtained on previous stages of the optimization process.

5 Algorithm for constrained optimization

For the numerical examples in this contribution, we consider general problems of shape optimization as outlined in (1). We will use a sequential quadratic programming

Table 1 Thick-wall infinite cylinder defined by 1 design variable. Design variable data

Design variable	Initial value	Data range
R_{ext}	17	[9 – 20]

(SQP) approach (Powell 1983), considered an active-set method (Gill et al. 1984), given by the MATLAB implementation (MATLAB version 8.3.0.532 (R2014a) 2014). SQP approaches are one of the most effective methods for non-linearly constrained optimization and generates steps by solving quadratic subproblems (Nocedal and Wright 2006).

The formulation of a quadratic programming subproblem for the problem description in (1) is based on a quadratic approximation of the Lagrangian function

$$\mathcal{L}(\mathbf{a}, \lambda) = f(\mathbf{a}) + \sum_{n=1}^r \lambda_j \cdot g_n(\mathbf{a}) \tag{41}$$

To model this problem we now linearize the inequality constraints of (1) to obtain

$$\begin{aligned} &\text{minimize} && f(\mathbf{a}_t) + \nabla f(\mathbf{a}_t)^T \mathbf{p} \nabla_{aa}^2 \mathcal{L}_t \mathbf{p} \\ &\text{verifying} && \nabla g_n(\mathbf{a}_t) \mathbf{p} + g_n(\mathbf{a}_t) \leq 0 \quad n = 1, \dots, r \end{aligned} \tag{42}$$

The new iterate is given by $(\mathbf{a}_t + \mathbf{p}_t, \lambda_{t+1})$ where \mathbf{p}_t and λ_{t+1} are the solution and the corresponding Lagrange multiplier.

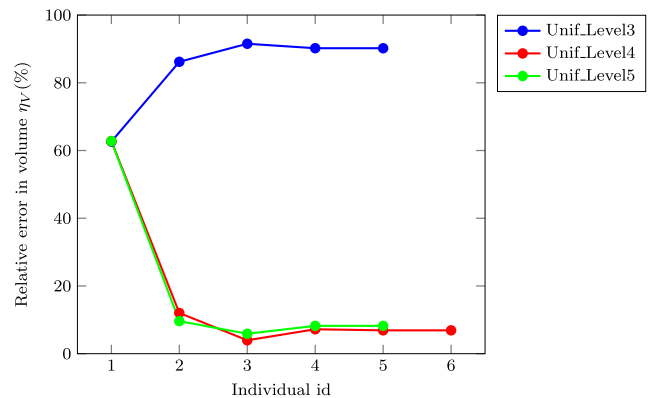


Fig. 16 Evolution of the error in the objective function (volume) with respect to the analytical solution. Uniform meshes

Table 2 Computational results for uniform meshes. Average values of computational cost and estimated discretization error in energy norm

Type of mesh	Computational cost (s)	Estimated discretization error
Unif_Level3	9.05	7.99%
Unif_Level4	25.13	2.41%
Unif_Level5	229.81	0.67%

In this approach the set of active constraints $\mathcal{A}_r = \{g_n(\mathbf{a}_r) = 0\}$ at the solution of (42) constitutes our guess of the active set at the solution of the nonlinear program. If the SQP method is able to correctly identify this optimal active set (and not change its guess at a subsequent iteration) then it will act like a Newton method for equality-constrained optimization and will converge rapidly (Nocedal and Wright 2006). For details on the MATLAB implementation we recommend (MATLAB version 8.3.0.532 (R2014a) 2014).

6 Numerical examples

In this Section we will show three numerical analyzes. The first one will be used to show the performance of the direct solver used to evaluate solution of the systems of equations when applying different reorderings to the matrices. The remaining two problems will be devoted to assess the optimization methodology presented in this contribution. The last two optimization analyzes will test the accuracy of the cgFEM implementation coupled with the optimization algorithm using an academical problem with different number of design variables.

The model proposed for this study is a thick-wall infinite cylinder loaded with internal pressure. The geometrical model for this problem is represented in Fig. 13. A linear-elasticity analysis is performed on a domain given by a CAD model that uses NURBS to represent the boundary. Only 1/4 of the section is modeled together with the appropriate symmetries. The internal and external surfaces are of radius r and R , with $R_{int} = 5$ and $R_{ext} = 20$. Young's modulus is

Table 3 Thick-wall cylinder defined by 1 design variable. Computational results for h -adapted and projected meshes

Type of mesh	Computational cost (s)	Estimated discretization error
hAdapMeshing	52.23	0.87%
ProjMeshing	39.53	0.99%

$E = 1000$, Poisson's ratio is $\nu = 0.3$ and the applied load is $P = 1$.

The exact solution for displacements and stresses is given by:

$$u_r = \frac{P(1 + \nu)}{E(k^2 - 1)} \left(r(1 - 2\nu) + \frac{R_{ext}^2}{r} \right), \quad u_y = 0 \quad (43)$$

$$\sigma_r = \frac{P}{k^2 - 1} \left(1 - \frac{R_{ext}^2}{r^2} \right), \quad \sigma_\phi = \frac{P}{k^2 - 1} \left(1 + \frac{R_{ext}^2}{r^2} \right), \quad \sigma_y = \nu(\sigma_r + \sigma_\phi) \quad (44)$$

where $k = R_{ext}/R_{int}$, $r = \sqrt{x^2 + z^2}$.

For the optimization analyzes we will substitute the constant R_{ext} for a unique design variable or we will define a set of design variables to define arbitrary external surfaces.

6.1 Performance of the direct solver

As explained in Section 2.2, the solution of the system of equations with direct solvers is a time consuming task that can be lightened using a proper reordering of the matrices involved. To solve the linear system of equations in (13), we have run the tests in MATLAB 2014a, using the standard *backslash* solver provided in this compilation. In this example, we will compare four different reordering strategies:

- *Nested Domain Decomposition (NDD)*: in this case, we use the NDD reordering presented in Section 2.2.
- *Reference*: this strategy consists in solving the system without any previous reordering.

Fig. 17 Evolution of magnitudes for h -adapted and projected meshes. Left: error in the objective function (volume) with respect to the analytical solution. Right: von Mises stress

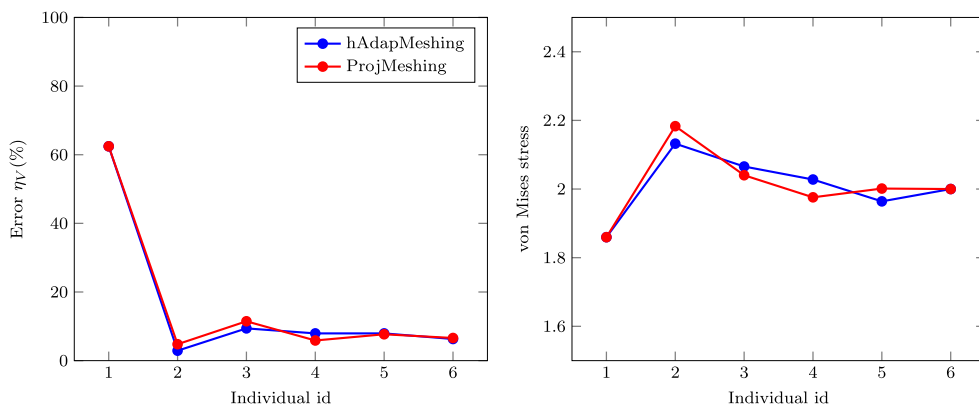
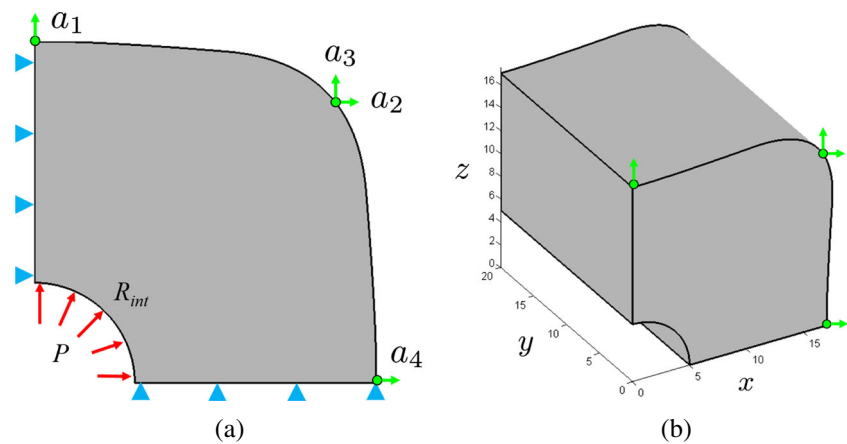


Fig. 18 Model of a cylinder under internal pressure defined by 4 design variables. **a** Front view with boundary conditions. **b** 3D model representation



- *Approximate Minimum Degree (AMD) permutation*: if the degree of a node in a graph is the number of connections to that node, the AMD algorithm (Amestoy et al. 1996) generates an ordering based on how these degrees are altered during Cholesky factorization.
- *Symmetric AMD permutation (SYM-AMD)* (MATLAB version 8.3.0.532 (R2014a) 2014): this algorithm performs an AMD reordering taking into account the symmetry of the matrix.
- *Column AMD permutation (COL-AMD)* (Davis et al. 2004): this algorithm returns the column approximate minimum degree permutation vector of the matrix. This is the default algorithm used by MATLAB.

For the analysis we will study a set of uniformly refined meshes of 20-node tri-quadratic elements. The meshes used in this simulation can be seen in Fig. 14.

On the left plot of Fig. 15, we can observe the computational cost related with the reordering of the degrees of freedom present in the meshes. This computational cost takes into account both, finding the reordered indexes and the reordering process. The right plot shows the computational cost related to the solution of the system of equations in terms of the speed-up achieved with respect to the reference, i.e., with no reordering. This means that a value larger than 1 represents the reduction of cost with respect with the reference calculation with no reordering.

From Fig. 15 we can extract the several conclusions. We can notice how, for small problems (two first meshes), the differences in computational cost between the different alternatives are not significant. However, for larger

problems we can clearly observe how the computational cost related to NDD reordering is clearly superior to the alternatives studied.

So, when using NDD, the time devoted to reorder the system of equations and to solve it is reduced, allowing for the solution of larger systems of equations with the same resources. The reason behind this positive performance of the proposed reordering technique can be that the NDD reordering could represent an optimal reordering, as it takes into account the topology of the mesh.

6.2 Thick-wall infinite cylinder loaded with internal pressure defined by 1 design variable

Let us consider R_{ext} as the design variable that defines the cylinder presented in Fig. 13. Our objective in this problem is to minimize the volume of the model under internal pressure P applied on the circular internal surface, with unknown external surface, where the Von Mises stresses must be below the yield stress S_y . For the parameters defined above and for $S_y = 2$, the optimal analytical solution corresponds to $b = 13.681300358237177$ and the corresponding volume is $V = 2547.485744735241$ (Table 1).

The first analysis consist of using sets of uniform meshes of 20-node tri-quadratic elements with different element size. We will use meshes of levels 3, 4 and 5 that correspond with the three last levels of refinement represented in Fig. 14. By doing this, we will evaluate how varying the discretization affects the accuracy and the computational cost.

Table 4 Thick-wall infinite cylinder defined by 4 design variables. Design variables data

Design variable	Initial value	Data range	Constraints on the design variables
a_1	17	[10 – 20]	None
a_2	16	[8 – 17]	$a_2 \leq a_4 - 1$
a_3	16	[8 – 17]	$a_3 \leq a_1 - 1$
a_4	17	[10 – 20]	None

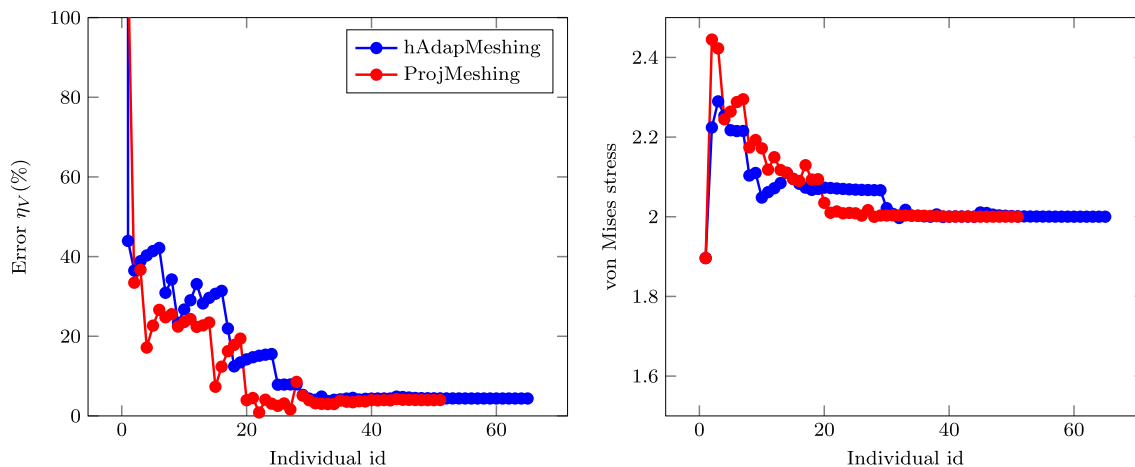


Fig. 19 Evolution of magnitudes for h -adapted and projected meshes. Left: error in the objective function (volume) with respect to the analytical solution. Right: von Mises stress

In Fig. 16 we can observe the evolution of the relative error in volume evaluated as $\eta_V(\%) = \frac{|V_h - V|}{V} \cdot 100$ where V_h is the volume integrated with the FE mesh and V is the exact volume of the model. The plot shows the convergence of the optimization process to a clearly suboptimal solution when using coarse meshes. In order to get closer to the theoretical optimal solution finer meshes have to be used, however this decision will involve an increase of the computational cost.

In Table 2 we can see the average discretization estimated error in energy norm per individual and the average computational cost per individual. We observe how, in order to reduce the discretization error, the computational cost of each individual increases significantly. This conclusion justifies the use of h -adaptive meshes.

We repeat the analysis but using h -adapted meshes and the projection technique presented in Section 4. In Fig. 17 we can observe the behavior of h -adapted meshes with tri-quadratic elements (hAdapMeshing) and projected h -adapted meshes with the same elements (ProjMeshing).

Table 3 shows the details of the analyzes in terms of average computational cost and estimated discretization error of the meshes. In this case, the h -adapted meshes achieve a level of accuracy similar to the accuracy obtained with the level 5 uniform mesh, but in a fraction of the time. In addition, the projected meshes cut the computational cost of the h -adaptive process in around 25%.

6.3 Thick-wall infinite cylinder loaded with internal pressure defined by 4 design variables

In this example we modify the previous model introducing several design variables. The initial shape is shown in Fig. 18. The shape optimization problem consists of finding the best shape for the external boundary defined by four

design variables, corresponding to coordinates of the points used to define the external boundary.

The mechanical properties for this problem correspond to those exposed at the beginning of the section. The initial values of the design variables and their allowed data range and constraints are shown in Table 4.

Figure 19 shows the evolution of the relative error in volume for an optimization process performed using standard h -refined meshes and another carried out using projected meshes. We can observe a common convergence path regardless of the different discretizations used.

In Table 5 we can see the average discretization estimated error in energy norm per individual and the computational cost per individual. The computational costs include the simulations performed to evaluate the sensitivities. We observe that for a comparable level of discretization error we save close to 20% of time when using mesh projection.

Figure 20 shows several of the individuals analyzed during the process including the first and the last one (51). In addition, the theoretical optimal solution has been drawn to clarify the evolution of the procedure.

6.4 Connecting rod defined by 8 design variables

The objective of this problem is to minimize the volume of a connecting rod without violating the given maximum

Table 5 Thick-wall cylinder defined by 4 design variables. Computational results for h -adapted and projected meshes

Type of mesh	Computational cost (s)	Estimated discretization error
hAdapMeshing	151.81	1.22%
ProjMeshing	124.90	1.46%

Fig. 20 Samples of individuals from the optimization procedure. The index indicate the number of model during the process

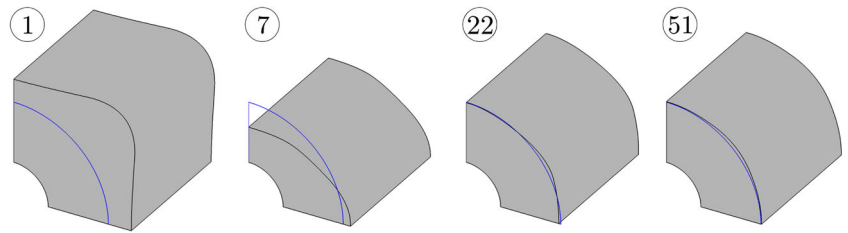


Fig. 21 Front view of the connecting rod problem with boundary conditions

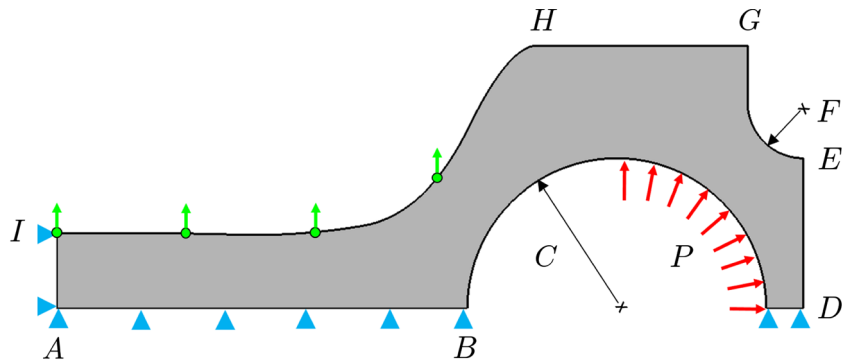


Fig. 22 3D model representation showing the 8 design variables

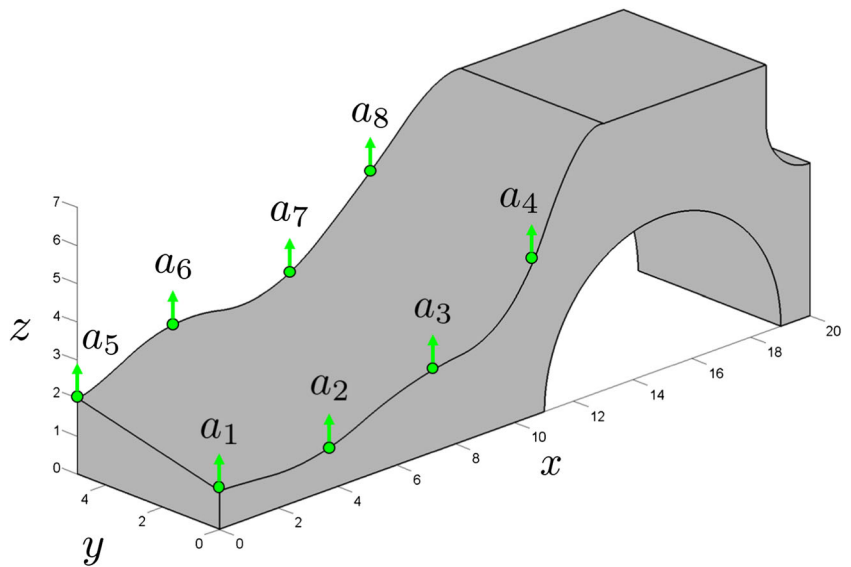


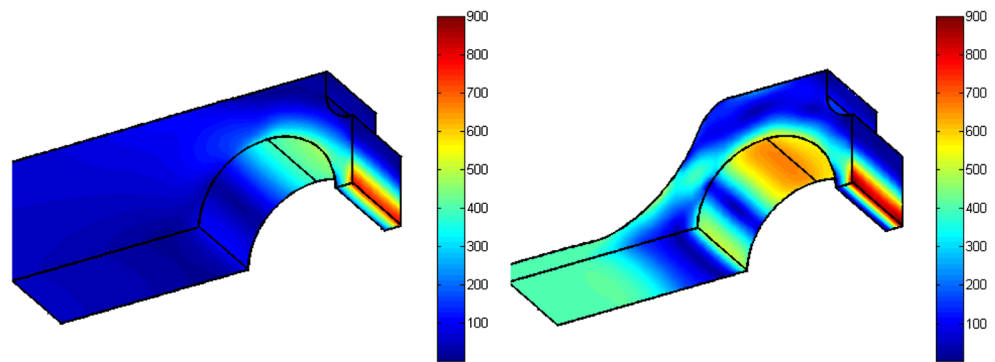
Table 6 Connecting rod defined by 8 design variable. Design variables data

Design variable	Initial value	Data range
a_1, a_5	7	[1 – 7]
a_2, a_6	7	[1 – 7]
a_3, a_7	7	[1.2 – 7]
a_4, a_8	7	[2 – 7]

Table 7 Connecting rod defined by 8 design variable. Computational results for *h*-adapted and projected meshes

Type of mesh	Computational cost (s)	Estimated discretization error
hAdapMeshing	607.84	2.83%
ProjMeshing	471.02	2.62%

Fig. 23 Von Mises stress fields: (left) initial configuration results and (right) configuration obtained using projected meshes



von Mises stress. Because of the symmetry, only a fourth of the component is modeled. The geometry of the initial design and the boundary conditions are shown in Fig. 21. The geometry parameters are $AB = 11$, $C = 4$, $AD = 20$, $DE = 4$, $F = 1.5$, $DG = 7$, $HG = 5.5$. The Young's modulus is $E = 10^5$, and Poisson's ratio $\nu = 0.333$. The pressure is $P = 100$ in the normal direction of the half arc as shown in Fig. 21.

The design boundary is the surface HG . The end point H is fixed while eight points are used to interpolate HG . The vertical positions of the eight interpolation points on the design surface are set as design variables (see Fig. 22). The allowable von Mises stress is $\sigma_{VM} = 900$.

The initial values of the design variables and their allowed data range are shown in Table 6.

Table 7 shows the average discretization estimated error in energy norm per individual and the computational cost per individual. We observe for this problem how the optimization procedure based in mesh projection cuts slightly more of a 20% of the time per individual.

Figure 23 shows the von Mises stress fields for the initial configuration of the model opposed to the field obtained for the optimal solution provided by the shape optimization algorithm.

7 Conclusions

Several tools to make gradient-based optimization procedures have been proposed. First, information sharing procedures that can be easily applied reducing the number of calculations needed. Also, the Nested Domain Decomposition reordering technique has been developed and tested for a 3D code. The NDD provides an optimal reordering of the global system of equations with minimum computational cost in comparison with other techniques. In addition, the speed-up shown during the resolution of the systems of equations is significant, allowing the efficient usage of the computational resources. Finally, an h -adaptive mesh projection strategy has been adapted to the immersed boundary environment. The projection avoids the need to generate a

suitable discretization after following a full refinement process. The discretizations generated with this procedure has been demonstrated as effective, in terms of convergence, than the standard h -refined meshes, but with an important reduction of the computational cost per individual.

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