

# Design and Analysis of Diagrid Structural Systems for High-Rise Buildings



Sneha Mole Jacob, N. Phani Charan, and Anju Raju

**Abstract** The diagrid is a framework of diagonally intersecting members that are used in the construction of buildings and roofs. It requires a lower percentage of steel than a standard design. The need for columns can be obviated by the use of diagrids. Diagonal members in the system carry gravity loads as well as lateral forces. Due to the triangulated configuration of members, internal axial forces arise in the members, in turn minimizing shear racking effects. Diagrid structures are generally used in the construction of high-rise buildings as lateral forces get minimized. The primary goal of the research is the design and analysis of diagrid structural systems for the high-rise buildings. The modeling was done by using PATRAN software and was analyzed using NASTRAN software. The study includes analysis of the representative models of various geometric forms for optimal construction in terms of strength, stiffness, aesthetic appearance, material requirement, and low cost. The investigations also include the study of the optimal cross section of diagrid members. A quasi-static environment and geometric nonlinear analysis are considered in the analysis of diagrid structures.

**Keywords** Diagrid · Optimal construction · Nonlinear analysis · PATRAN · NASTRAN

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## 1 Introduction

The advancements in the construction of high-rise buildings have led to the implementation of diagrid structures. In the olden times, high-rise buildings were of typical portal frame systems with horizontal beams and vertical columns. Later on, additional bracings were introduced in order to take up the lateral loads more efficiently than that in the conventional building frame systems. The major difference of a diagrid building in comparison with a braced tube building is the absence of vertical columns in the system. With a lower requirement and maximum exploitation of materials, diagrid structures exhibit their effectiveness in contrast to conventional and braced frames.

Diagonal members in diagrid structures act both as inclined columns and as bracing elements, thus carrying both gravity loads as well as lateral forces. With the axial action of diagonal members, it takes up the shear acting on it, thus reducing the shear racking effects. The unique geometric configuration of the system enhances structural stability along with the aesthetic appeal. It provides for a sustainable structure with greater stiffness and structural efficiency. Redundancy in the diagrid design helps in the transfer of load from a failed portion of the structure to another. A diagrid has better ability to redistribute load than a moment frame skyscraper, creating a deserved appeal for the system.

The most commonly used material for the diagrid construction is steel and the system can be applied for various geometric forms. Geometric nonlinearities have led to the evolution of diagrid projects with hyperboloid, aerodynamic, cylindrical, irregular, twisted, tapered, tilted, and free forms, the most challenging among them being “twisted”. Examples for diagrid structures with the common geometric forms are the steel diagrids that can be created using modern 3D modeling software, as the mesh conforming to almost any shape is possible. With the redundancy factor, unusual structures and complex geometries have become possible for the system.

The goal of this paper is to analyze and compare the geometric nonlinearity on diagrid structural systems. The different geometrical shapes considered are building plan sections of square, triangular, and circular. Each shape of the building is analyzed for lateral displacements by varying the cross sections of the members. The different cross-sectional geometries considered are rectangular, circular, hollow circular, and I-section. The study is carried out to obtain the optimal geometries for the building and cross section.

## 2 Objectives of the Study

The objectives of the study include the following:

- To analyze the representative models of various geometric forms for optimal construction in terms of strength, stiffness, aesthetic appearance, material requirement, and low cost

- To study the optimal cross section of diagrid members.

### 3 Analysis of Diagrid Buildings

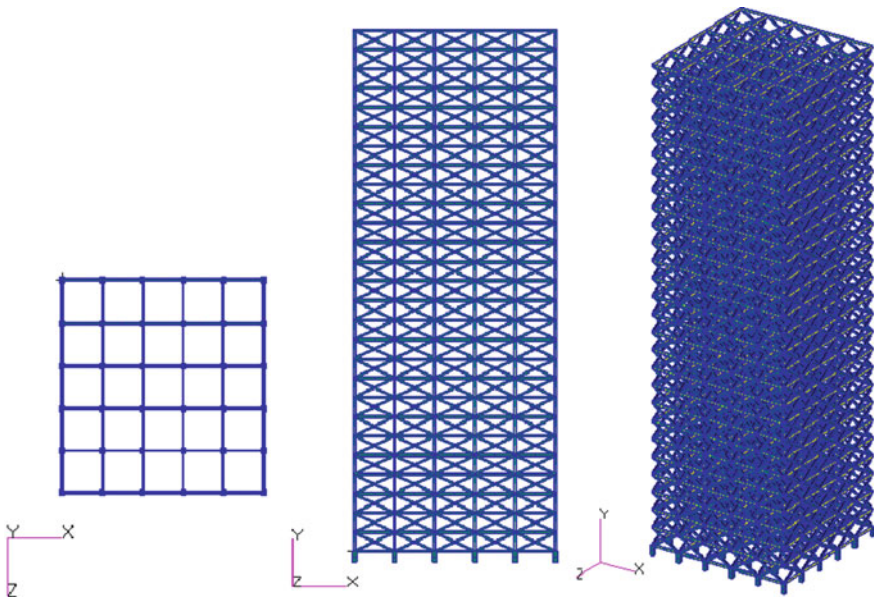
Geometrical nonlinear analysis of G+26 story buildings with a plan area of 625 m<sup>2</sup> is modeled using PATRAN software and analyzed using NASTRAN software considering the seismic behavior of the structure. Buildings of square, triangular, and circular plans are considered with varying member cross sections of rectangle, circle, hollow, and I-sections.

#### 3.1 Structural Models

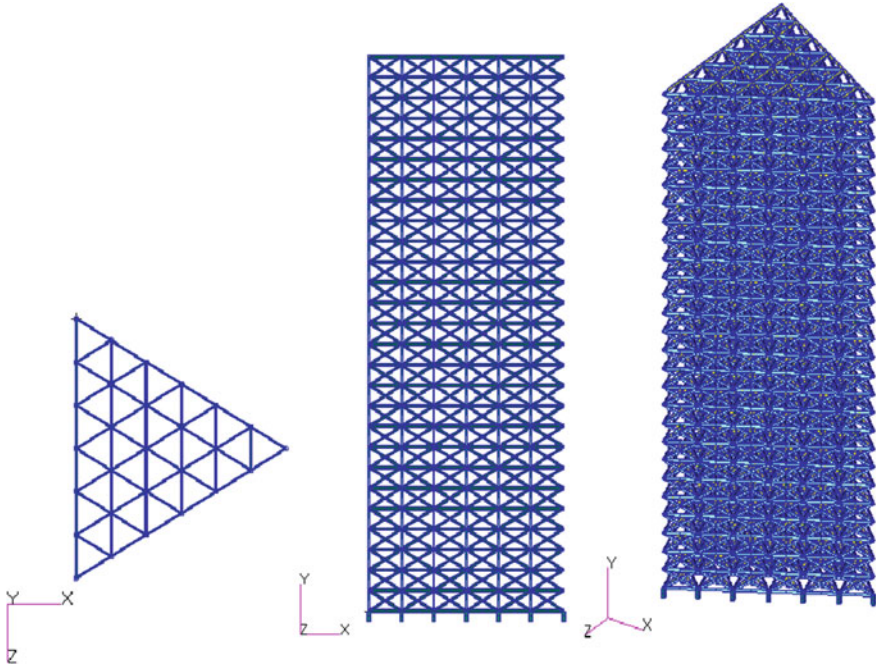
Regular structures with the same cross-sectional areas are considered for the analysis. G+26 buildings of 75.75 m height, with a story height of 2.75 m and a footing height of 1.5 m is modeled in PATRAN software.

Plan dimensions of buildings with different geometrical forms are as follows:

- Square: 25 m × 25 m, shown in Fig. 1
- Triangle: Side, a = 38 m, shown in Fig. 2



**Fig. 1** Plan, elevation, and 3D model of square building

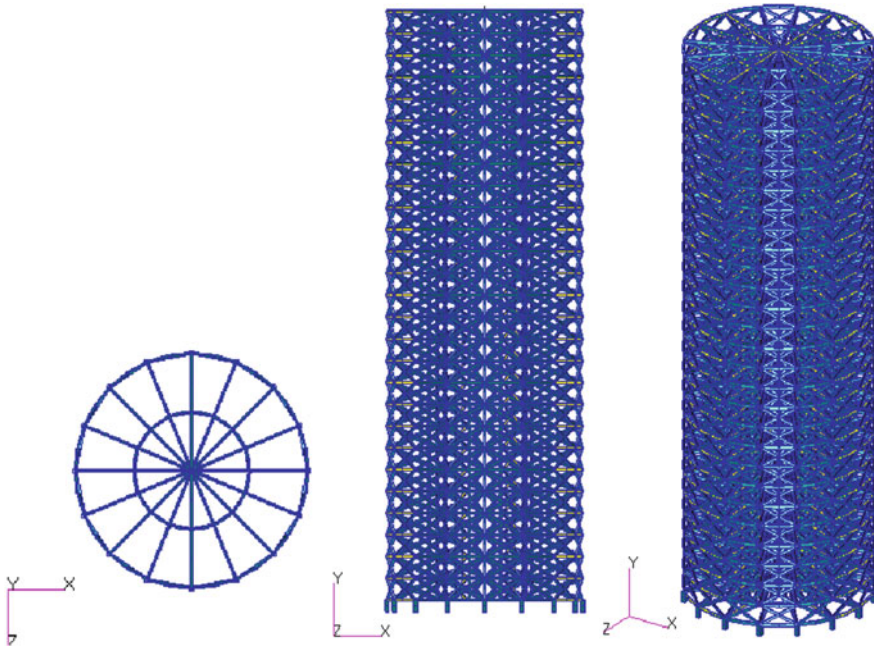


**Fig. 2** Plan, elevation, and 3D model of triangular building

- Circle: Diameter,  $\Phi = 28$  m, shown in Fig. 3.

Member cross sections are modeled for beam cross section to be  $80,000 \text{ mm}^2$ , diagrid cross section to be  $90,000 \text{ mm}^2$ , and footing cross section to be  $36,000 \text{ mm}^2$ , and are as follows:

- Rectangle: For beams— $200 \text{ mm} \times 400 \text{ mm}$   
For diagrid— $300 \text{ mm} \times 300 \text{ mm}$   
For footings— $600 \text{ mm} \times 600 \text{ mm}$
- Circle: For beams— $319.154 \text{ mm } \Phi$   
For diagrid— $338.514 \text{ mm } \Phi$   
For footings— $677.028 \text{ mm } \Phi$
- Hollow circle: For beams— $\Phi_1 = 382 \text{ mm}$ ,  $\Phi_2 = 210 \text{ mm}$   
For diagrid— $\Phi_1 = 380 \text{ mm}$ ,  $\Phi_2 = 172 \text{ mm}$   
For footings— $\Phi_1 = 720 \text{ mm}$ ,  $\Phi_2 = 245 \text{ mm}$
- I-section: For beams—Total height,  $H = 500 \text{ mm}$   
Flange width,  $W = 300 \text{ mm}$   
Web thickness,  $t_w = 78 \text{ mm}$   
Flange thickness,  $t_f = 92 \text{ mm}$   
  
For diagrid—Total height,  $H = 500 \text{ mm}$   
Flange width,  $W = 300 \text{ mm}$



**Fig. 3** Plan, elevation, and 3D model of circular building

Web thickness,  $t_w = 78$  mm  
 Flange thickness,  $t_f = 92$  mm  
 For footings—Total height,  $H = 500$  mm  
 Flange width,  $W = 300$  mm  
 Web thickness,  $t_w = 78$  mm  
 Flange thickness,  $t_f = 92$  mm

### 3.2 Material Properties

Material properties of steel are given as follows:

- Density of steel =  $7800 \text{ kg/m}^3$
- Young's modulus =  $2.1 \times 10^5 \text{ N/mm}^2$
- Poisson's ratio = 0.3

### 3.3 Loads and Boundary Conditions

Boundary conditions are provided as fixed at the base.

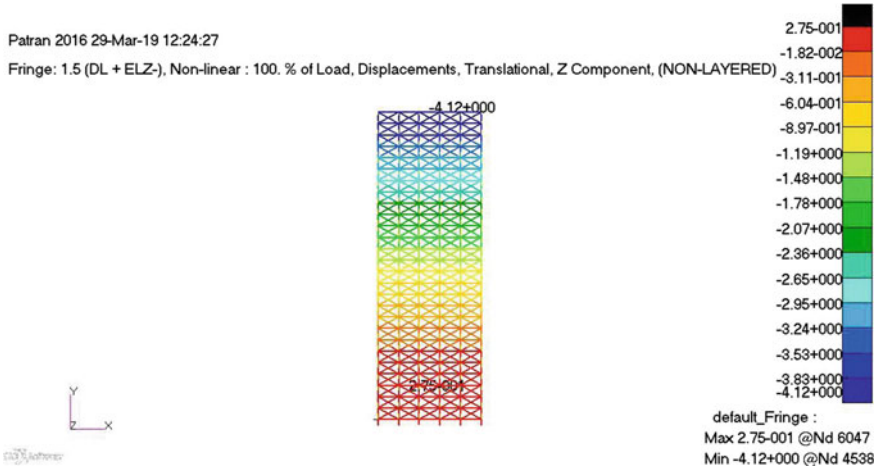


Fig. 4 Maximum lateral displacement obtained for square building with rectangular cross section

Dead load value for the roof is given as 1.5 kN/m<sup>2</sup> and for all other floors as 4.5 kN/m<sup>2</sup>.

Live load values for all floors are given as 4 kN/m<sup>2</sup>.

Earthquake loads are calculated as per IS 1893(Part1):2002.

6 load cases and 13 basic load combinations were considered for the study.

### 3.4 Analysis Results

Analysis results were taken based on lateral displacement as shown in Figs. 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14 and 15.

## 4 Conclusions

The conclusions obtained are the following:

- The minimum lateral displacement is obtained for square building and is maximum for triangular building.
- The maximum displacements are obtained at load case 1.5 (DL+ELZ<sup>-</sup>) for square and circular buildings and is 1.5 (DL+ELZ) for triangular building.
- Buildings of members with circular cross section show the minimum displacement and with rectangular cross section shows the maximum displacement.

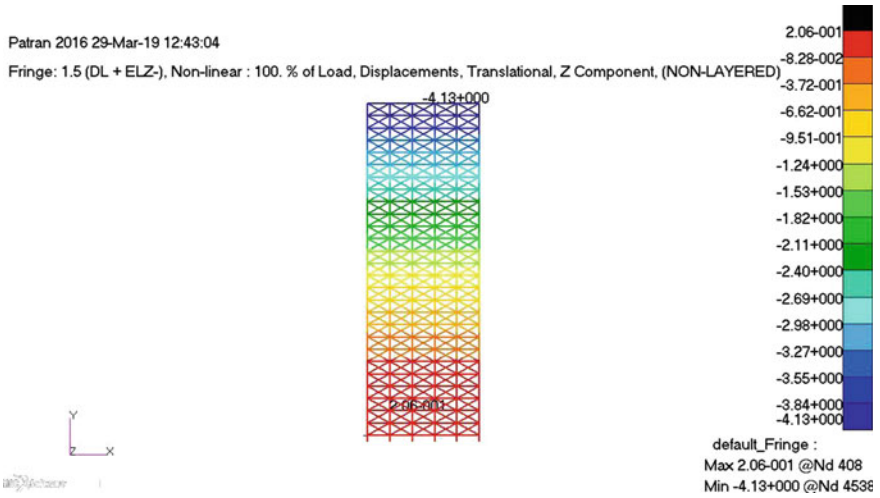


Fig. 5 Maximum lateral displacement obtained for square building with circular cross section

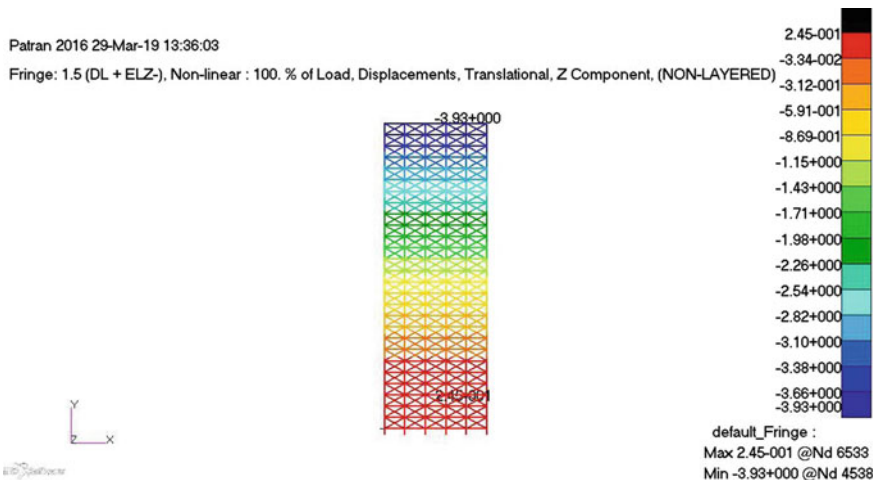


Fig. 6 Maximum lateral displacement obtained for square building with hollow circular cross section



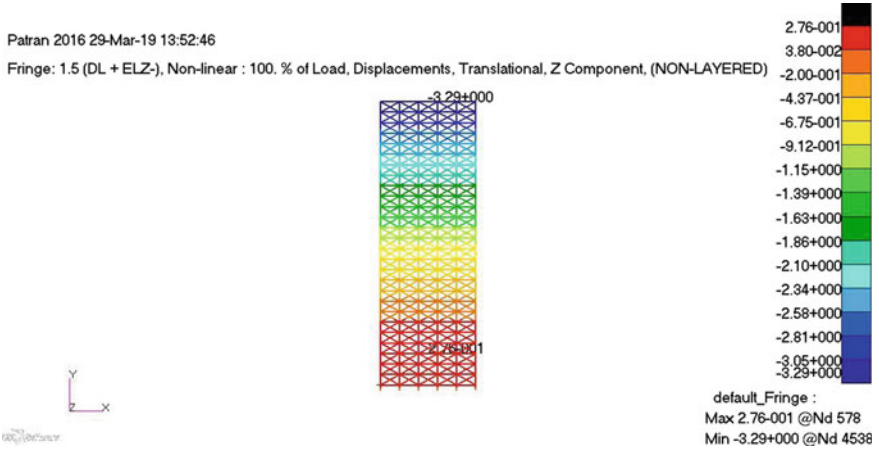


Fig. 7 Maximum lateral displacement obtained for square building with I-section

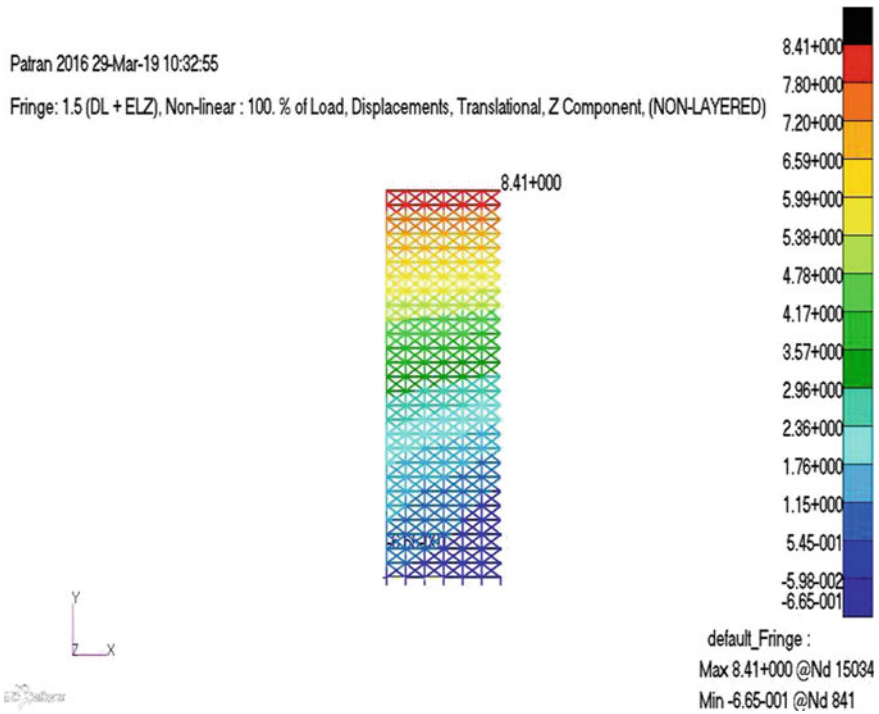


Fig. 8 Maximum lateral displacement obtained for triangular building with rectangular cross section





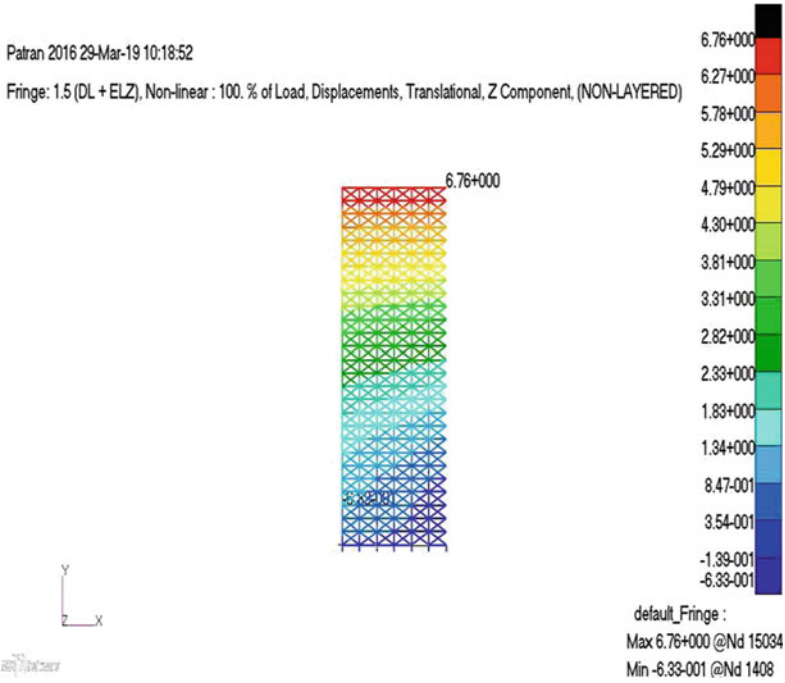


Fig. 11 Maximum lateral displacement obtained for triangular building with I-section

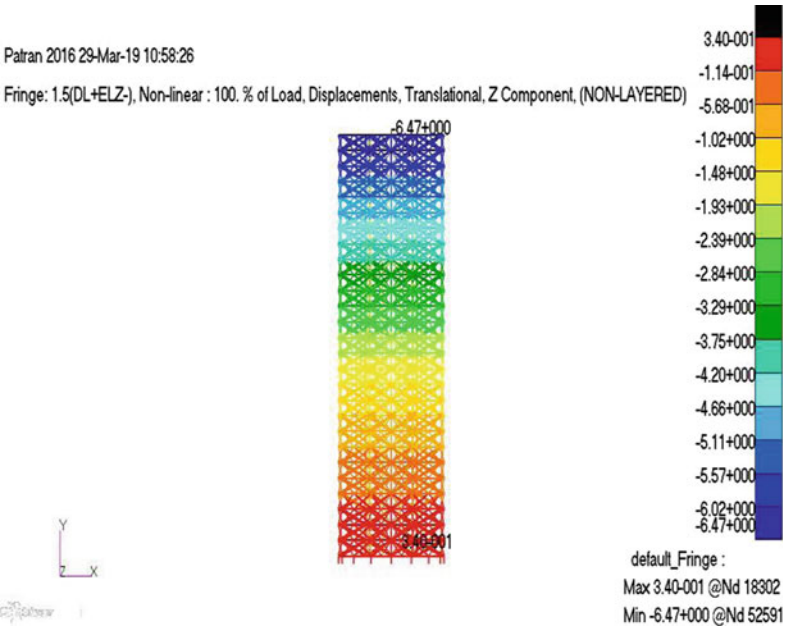


Fig. 12 Maximum lateral displacement obtained for cylindrical building with rectangular cross section

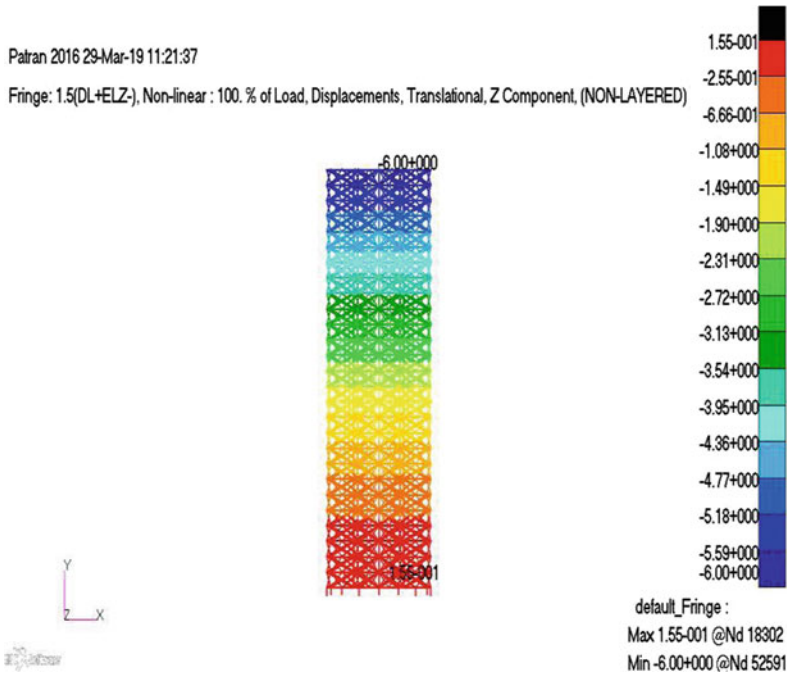


Fig. 13 Maximum lateral displacement obtained for cylindrical building with circular cross section

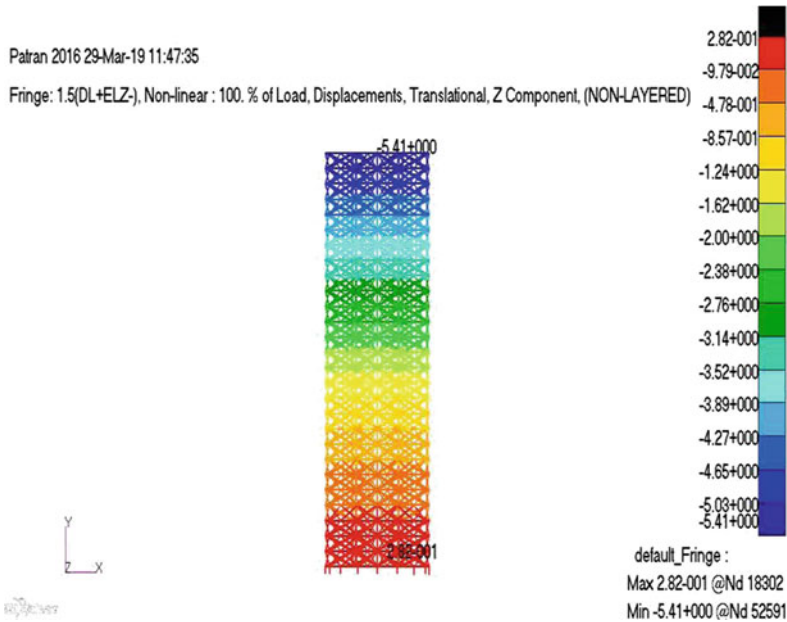
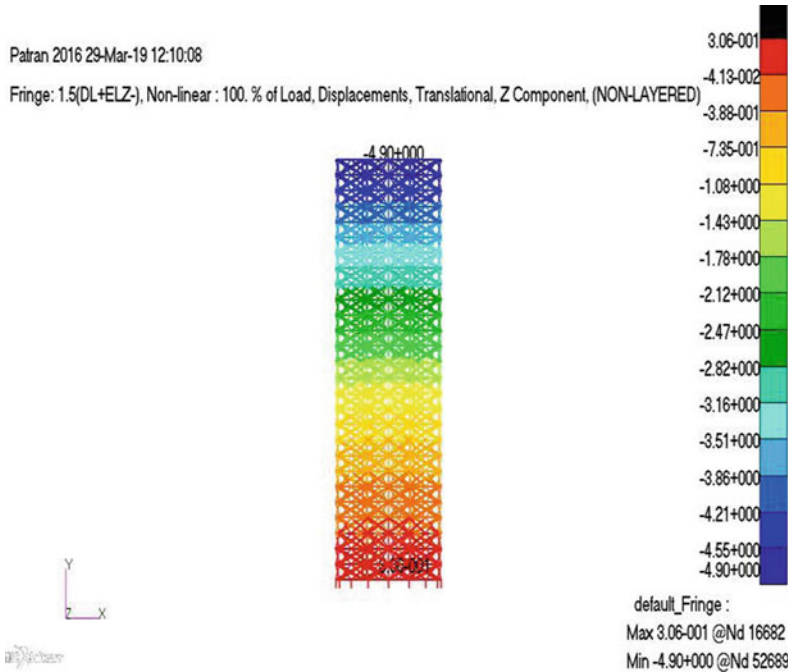


Fig. 14 Maximum lateral displacement obtained for cylindrical building with hollow circular cross section



**Fig. 15** Maximum lateral displacement obtained for cylindrical building with I-section

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