# Layout Optimization of Sewer Network Using Minimum Cumulative Flow in the Sewer Network



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Abstract The optimal design of sewer network includes two subproblems: (1) finding the feasible sewer layouts and (2) optimal design of sewer layout components. The topic of sewer network optimization has been broadly studied since the concept was first proposed in the late-1960s. Most of the design practices have focused on sewer network component sizing with a fixed plan layout. The latter includes pipe diameters, slopes, and excavation depths, all of which is considerably influenced by the layout configuration. This works goal at introducing a method to solve the layout subproblem. A new approach to the selection of feasible sewer layouts has been developed, in which generation of all spanning tree algorithm is introduced to generate all possible sewer layouts from the base sewer network. After that, these sewer layouts are sorted in ascending order of total cumulative flow and a cost function is applied to determine the optimal sewer layout. The proposed method has been applied to solve two test examples at different scales, and the results have been discussed. The results clearly reveal the efficiency of the proposed method which can effectively solve the problem of optimal layout determination of a sewer network.

Keywords Sewer networks · Spanning tree · Layout optimization

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# 1 Introduction

Sewer network is an essential part of any urban infrastructure. The sewer network design consists of creating a suitable sewer layout that conforms to connect all buildings, street layouts, and local developing area. The alignment of sewer layouts is highly dependent on the location of sewage treatment plant or outlet, network size, and topography of the area. Finding the minimum length layouts among too many alternatives is the first step in designing a new urban sewer network. The capital cost of sewer network, installing, replacing, or modifying is very high. Reduction in the length of sewer line leads to a substantial saving in the capital cost. For this reason, significant researchers have attempted to the development of appropriate optimization techniques for sewer network design in recent years [1–6].

The sewer network design optimization problem is divided into two subproblems which are feasible layout selection and optimal component size determination. Due to the complex nature of the problem, most of the researchers have focused on the easier problem of component size optimization [7–26]. Only a few researchers have addressed the problem of feasible layout generation of networks by using different techniques of optimization [2–6, 11, 27–30].

This work describes a method for the selection of an optimal layout of a base sewer network. An algorithm generation of all spanning tree has applied to the generation of all minimum length layouts of a base sewer network. After that, all layouts are then sequenced in ascending order of total cumulative flow (Q) while the cost function of a layout based on that proposed by Walters and Smith [30] is applied to find the optimal sewer layouts. The proposed method is applied to two examples, and results are presented.

#### 2 Feasible Sewer Layout Selection Method

The sewer network is a graph with specific properties. Therefore, it is necessary to review some basic definitions and principles of the graphs [31–33].

An undirected graph G = (V, E) consists of a set of objects  $V (V = v_1, v_2, ..., v_n)$  called vertices and another set  $E (E = e_1, e_2, ..., e_n)$  called edges, such that each edge  $e_{ij}$  is identified with an unordered pair  $(v_i, v_j)$  of vertices. If in a graph G there is one path between every pair of vertices, G is a tree.

A weighted graph is a graph G in which every edge e has been allotted a real number w (e) called the weight of e. The weight of an edge in the sewerage system has been taken as its length.

A spanning tree of a graph G is a tree containing all vertices of G. A minimum spanning tree of an undirected weighted graph G is a spanning tree of which the sum of the edge weights is minimal.

There are several algorithms for finding a minimum spanning tree (MST) of a graph. Kruskal's algorithm is one of the optimized ways to generate a minimum

length spanning tree for every connected undirected graph G. But layout optimization problem needs to generate all the spanning trees (layouts) of a base network (graph). Therefore, an algorithm generation of all spanning tree is introduced to find all sewer layouts (spanning trees) of a base network. The algorithm is based on the assumption that a base graph or sewer network, including all possible edges of the network, is available. In the base sewer network, there are n vertices (manholes or nodes) and m edges (sewers or links). A simplified representation of the algorithm is given below.

# 2.1 Algorithm—Generation of All Spanning Trees

Input—n, m, E, weights and nodal flow contribution;

*n* is the number of vertices, *m* is the number of edges, and *E* is the set of edges of graph *G*. Weight is the length of edges (sewers);

e\_old is an edge of the current minimum spanning tree (MST), and e\_new is a new edge from the remaining graph (base network);

Current\_MST\_weight is the total weight of current MST, and new\_MST\_weight is the total weight of newly generated MST;

1 1 -	_file (all spanning trees of G); MST from a base network by using Kruskal's algorithm;
1	T edges and weight:
1	edge e old in current MST
Step 4:	Remove selected edge e_old
Step 5:	For each edge e_new in base graph (network) except current_MST edges
Step 6:	New_MST_weight = current_MST_weight - e_old edge weight + e_new edge weight;
Step 7:	If new_MST_weight = current_MST_weight
Step 8:	Store the edges along with total weight in the final output for MST
Step 9:	Else if new_MST_weight > current_MST_weight;
Step 10:	Store temporary new edges, and corresponding weight;
Step 11: Repeat s	step 5;
Step 12:	Find minimum from all temporarily stored edges and their corresponding weight to get a next
	Spanning tree;
Step 13: Print Sp	anning trees (new edges and weight);
Step 14: Repeat s	step 3 for next Spanning tree;
Step 15: Calculat	e discharges (flows) in each spanning tree;
Step 16: End;	

#### 2.2 Optimal Sewer Layout Selection

As described above, by using the generation of all spanning tree algorithm determined all sewer layouts of a base network. As a result, a large number of alternative layouts are available, and it is very difficult to identify directly the true optimal layout. Therefore, a strategy to sequence these alternatives needs to be introduced. Discharge  $q_{ij}$  in *i*th link of the *j*th layout is calculated. After that, the sum of the cumulative discharges  $Q_j$  in all links of the *j*th layout is calculated as shown in Eq. (1).

$$Q_j = \sum_{i=1}^{i=n} q_{ij} \tag{1}$$

where n is the total number of links of the *j*th layout.

These layouts are then sequenced in ascending order by the sum of the cumulative discharges  $Q_j$ . The layouts are then investigated for optimality in this sequence. Walters and Smith [30] proposed a cost function for the cost of network layout in terms of the length and concave function of flow rate of each link as given in Eq. (2):

$$C_j = \sum_{i=1}^n L_{ij} \sqrt{q_{ij}} \tag{2}$$

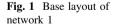
where  $C_j$  = layout cost of the *j*th alternative,  $L_{ij}$  = link length, and  $q_{ij}$  = sewer discharge in the *i*th link of the *j*th layout.

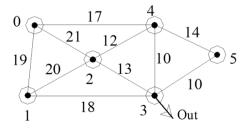
The cost of each layout is calculated by using Eq. (2), and the optimal layout of a base network is selected where the cost of the layout is minimal.

## 3 Application

For the sewer network problem, manhole represents vertices and sewer pipes represent edges. The length of the sewer is taken as the weight of an edge.

The applicability of the procedures described in the previous section is tested in this section against two examples. The first example (network 1) that has been considered is a simple network, which is shown in Fig. 1. The network 1 consists of 6 nodes (vertices) and 10 links (edges), and the outlet is located at the node number 3. The wastewater contribution at each node of the network 1 is shown in Table 1.





<b>Table 1</b> Nodal wastewater           contribution for network 1	Node No.	0	1	2	3	4	5
contribution for network 1	Flow contribution (l/s)	20	15	18	0	17	14

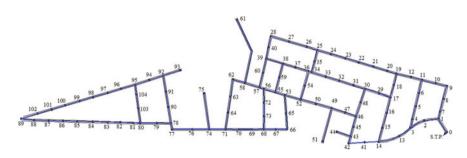


Fig. 2 Sudarshanpura base sewer network (network 2)

The second test example (sewer network of Sudarshanpura, Jaipur) is shown in Fig. 2. The Sudarshanpura base sewer network (network 2) consists of 105 nodes (vertices), 116 links (edges), and the outlet located at node number 0. Base graph data (link number, set of links, and its length) of network 2 are shown in Table 2. The wastewater contribution at each node of the network 2 is shown in Table 3.

The total number of nodes and links, set of links or edges, link length (weight), and nodal wastewater contributions are entered as an input detail for both the network.

Both problems network 1 and network 2 are solved by using above proposed procedure. The generation of all spanning tree algorithm is applied to find all layouts (spanning tree) of a base network; then, these layouts to be sequenced in ascending order of total cumulative flow Q (Eq. 1). After that, the cost function (Eq. 2) is applied to determine layout cost of the sequenced layouts. The top 4 optimal layouts of a network 1 are shown in Fig. 3.

The results of network 2 are illustrated in Table 4. It is clearly seen that the layout cost (*C*) generally increases with the total cumulative flow (*Q*). The minimum cost of 13111.46 was obtained corresponding to the layout with cumulative flow value (*Q*) of 3642.78 l/s (Fig. 4). The layout with minimum cumulative flow value of 3639.13 l/s has the cost of 13170.85 (Fig. 5) which is 0.45% above the minimum cost layout. The proposed method for selection of global optimal layout is very convenient to implement on the sewer network problem. It may be concluded that minimum cumulative flow layout is near global optimal layout.

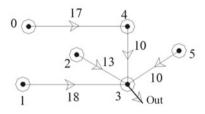
Link No.	Nodes	s	Length	Link No.	Nodes	s	Length	Link No.	Nodes	\$	Length	Link No.	Nodes		Length
	0	-	30	30	27	28	30	59	54	36	24	88	77	78	13
		7	23	31	17	29	30	60	52	53	30	89	78	79	31
	7	e	23	32	29	30	22	61	53	55	20	90	79	80	31
	e	4	10	33	30	31	30	62	55	59	30	91	80	81	10
	4	5	30	34	31	32	30	63	59	38	30	92	81	82	30
	5	9	30	35	32	33	30	64	55	56	25	93	82	83	30
	6	Ξ	30	36	33	34	18	65	56	57	8	94	83	84	30
	-	7	6	37	34	35	30	66	57	60	32	95	84	85	30
	7	~	30	38	35	25	12	67	60	39	32	96	85	86	30
10	×	6	30	39	34	36	7	68	57	58	33	76	86	87	30
	6	10	20	40	36	37	16	69	58	61	143	98	87	88	30
12	10	11	30	41	37	38	30	70	58	62	24	66	88	89	30
13	Ξ	12	20	42	38	39	30	71	62	63	33	100	78	90	33
14	12	19	30	43	39	40	14	72	63	64	33	101	90	91	33
15	3	13	30	44	40	28	30	73	64	71	33	102	91	92	33
16	13	14	30	45	14	41	30	74	53	65	30	103	92	93	36
17	14	15	30	46	41	42	30	75	65	99	30	104	92	94	30
18	15	16	30	47	42	43	11	76	99	67	22	105	94	95	26
19	16	17	30	48	43	4	30	LT	67	68	22	106	95	96	30
20	17	18	30	49	43	45	20	78	56	72	21	107	96	97	30
21	18	19	12	50	45	46	20	79	72	73	30	108	97	98	30

(continued)
e 2
Tabl

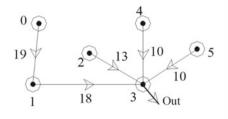
Link No.	Nodes	s	Length	Link No.	Nodes	s	Length	Link No.	Nodes	s	Length	Link No.	Nodes		Length
22	19	20	18	51	46	48	30	80	73	68	30	109	98	66	30
23	20	21	30	52	48	30	24	81	68	69	26	110	66	100	30
24	21	22	30	53	46	47	26	82	69	70	26	111	100	101	30
25	22	23	30	54	47	49	26	83	70	71	26	112	101	102	30
26	23	24	30	55	49	51	72	84	71	74	34	113	102	89	30
27	24	25	30	56	49	50	30	85	74	75	76	114	80	103	27
28	25	26	27	57	50	52	30	86	74	76	38	115	103	104	27
29	26	27	30	58	52	54	30	87	76	77	38	116	104	95	27

Node No.	Flow (l/s)								
0	0.000	21	0.380	42	0.380	63	0.418	84	0.380
1	0.380	22	0.380	43	0.139	64	0.672	85	0.380
2	0.292	23	0.380	44	0.380	65	0.380	86	0.380
3	0.292	24	0.380	45	0.254	66	0.380	87	0.380
4	0.127	25	0.380	46	0.444	67	0.279	88	0.380
5	0.380	26	0.342	47	0.330	68	0.279	89	50.380
6	0.380	27	0.380	48	0.507	69	0.355	90	0.418
7	0.114	28	0.380	49	0.330	70	0.330	91	0.837
8	0.380	29	0.380	50	0.380	71	0.507	92	0.380
9	0.254	30	0.279	51	0.913	72	0.570	93	0.456
10	0.444	31	0.380	52	0.570	73	0.570	94	0.330
11	0.570	32	0.380	53	0.380	74	0.431	95	0.342
12	0.254	33	0.380	54	0.507	75	0.963	96	0.380
13	0.380	34	0.418	55	0.444	76	0.482	97	0.380
14	0.380	35	0.380	56	0.317	77	0.482	98	0.380
15	0.380	36	0.089	57	0.317	78	0.165	99	0.380
16	0.380	37	0.203	58	0.418	79	0.393	100	0.380
17	0.570	38	0.570	59	0.570	80	0.393	101	0.380
18	0.342	39	0.177	60	0.608	81	0.127	102	0.761
19	0.380	40	0.380	61	51.80	82	0.380	103	0.342
20	0.228	41	0.380	62	0.304	83	0.380	104	0.342

 Table 3 Nodal wastewater contribution for network 2

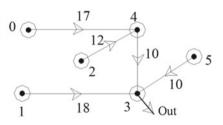


(a) Optimal layout with C= 299.1, Q = 104

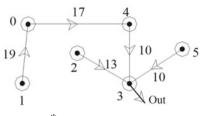


(c)  $3^{rd}$  alternative with C=325.2, Q = 104

Fig. 3 Top for optimal layout of a network 1



(b)  $2^{nd}$  alternative with C= 308.2, Q = 122



(d)  $4^{\text{th}}$  alternative with C=338.8, Q =134

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Total cumulative flow (l/s)	Layout cost	Total cumulative flow (l/s)	Layout cost
3639.13	13170.85	4375.98	14411.83
3641.96	13114.59	4426.58	14543.76
3642.78	13111.46	4521.92	14993.24
3718.5	13183.71	4616.77	14816.62
3896.6	13915.56	4676.69	14619.86
4027.95	14022.27	4720.37	15102.32
4077.09	14135.68	4772.37	14946.37
4178.5	14562.22	4820.16	15163.73
4227.3	14476.68	4870.45	15113.36
4275.64	14739.47	4920.55	15321.61

Table 4 Total cumulative flow versus layout cost comparison

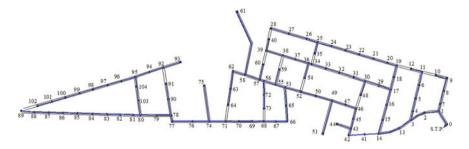


Fig. 4 Optimal sewer layout of network 2

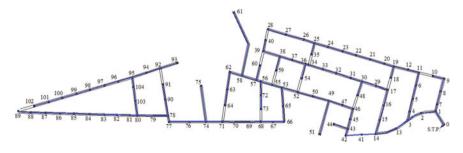


Fig. 5 Alternative sewer layout of network 2

#### 4 Conclusions

A method for the selection of an optimal layout of a base network is introduced, in which an algorithm is applied to generate all spanning trees of a network (graph). As large numbers of possible spanning trees are available, these spanning trees are sorted in ascending order of total cumulative flow (Q). Further, the cost function is

applied to determine layout cost of the sorted layouts. The optimal layout was selected where the total cost of layout found to be minimal. It may be concluded that the total cost of alternative generally increases with the total cumulative flow. The applicability and efficiency of the proposed method for layout optimization of sewer networks were tested against two examples. The result revealed that proposed method gives an optimal solution of the sewerage network.

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